

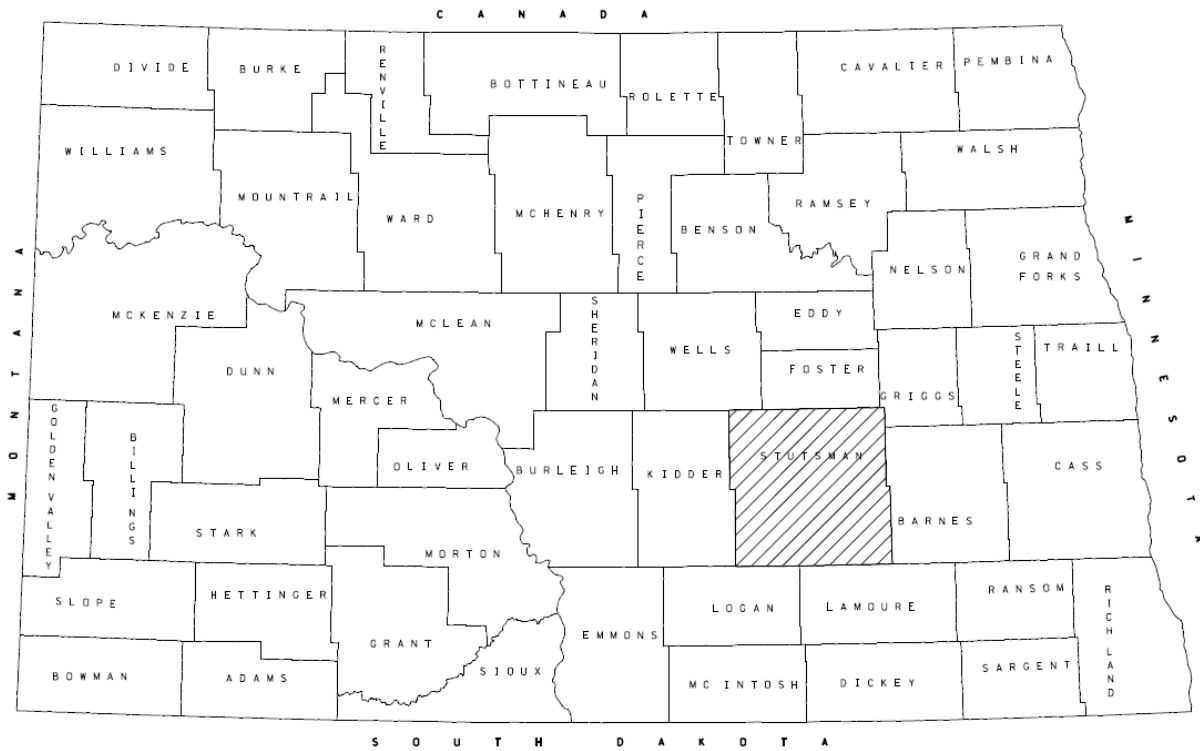
# Deep Foundation Report and Analysis

PROJECT NO. IM-2-094(194)260

PCN 23577

COUNTY Stutsman

Bridge #: 0094-260.125 R



PREPARED BY: Riley McAdoo-Roesler

NORTH DAKOTA DEPARTMENT OF TRANSPORTATION  
MATERIALS AND RESEARCH DIVISION

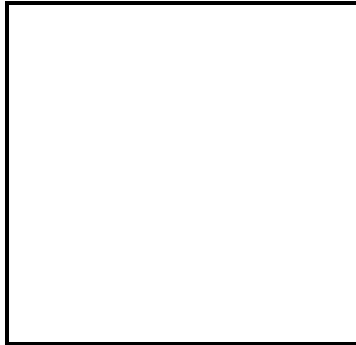
July 2023

**IM-2-094(194)260**

SE Jamestown Interchange  
Bridge #: 0094-260.125 R

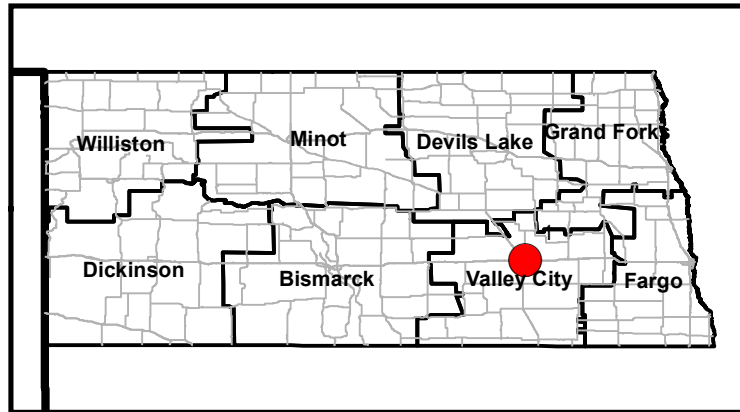
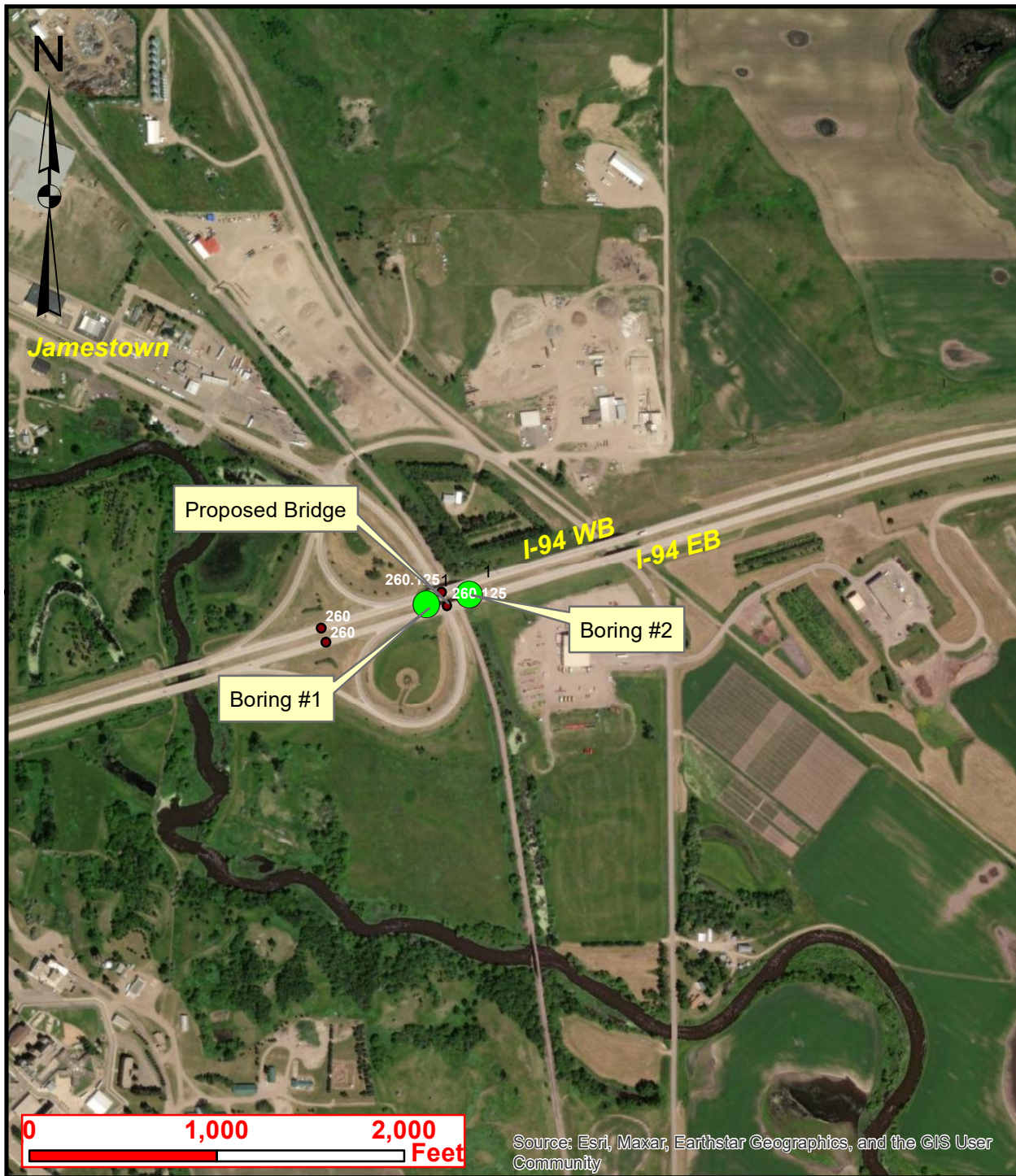
## ***CERTIFICATION***

I hereby certify that this report was prepared by me or under my direct supervision and that I am a duly registered professional engineer under the laws of the State of North Dakota. This document was originally issued and sealed by Colter Schwagler, Registration number PE-27747 on 07/19/2023 and the original document is stored at the North Dakota Department of Transportation.



# SE Jamestown Interchange Deep Foundation Borings

Project: IM-2-094(194)260  
PCN: 23577  
Bridges: 0094-260.125 R  
Scope: Structure Replacement  
Location: SE Jamestown Int. - EB



Source: Esri, Maxar, Earthstar Geographics, and the GIS User Community

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## **Appendix**

- Appendix A - Boring Logs
- Appendix B - Lab Results
- Appendix C - Stability Analysis Outputs

## **Introduction**

The purpose of the project is to address the existing deficiencies of the current structure by removing and replacing the bridge on the same alignment along the I-94 corridor.

This report will provide foundation, settlement, and bridge end slope recommendations for the construction of the new structure.

## **Existing Structure Information**

There is an existing bridge on I-94 that will be removed as part of the project. The information for this structure is below.

### **Bridge #0094-260.125 R**

Main Structure Type: Prestressed Concrete Stringer

Length: 221'

Foundation Type: Treated Timber Piling

Year Constructed: 1958

## **Soil Borings**

A total of 2 borings were completed near the proposed structure. See the project location map for the boring locations.

### **Boring #1**

Boring #1 has an elevation of approximately 1423 feet at the top of boring and is located near the proposed west abutment. This boring extends to a depth of 119.5 feet.

### **Boring #2**

Boring #2 has an elevation of approximately 1426 feet at the top of boring and is located near the proposed east abutment. This boring extends to a depth of 119.6 feet.

## **Sampling and Testing Procedures:**

Shelby tube sampling and split spoon sampling were used to extract the samples from a hollow stem auger.

Shelby tube sampling provides an “undisturbed” sample of fine-grained soils for laboratory testing via a thin wall tube that is slowly pushed into the soils to be sampled. Densities were calculated according to AASHTO test method T-296.

Split spoon samplers are utilized during advancement of the boring to perform the Standard Penetration Test (SPT). The samples are considered “disturbed”, due to the driving nature in which they are obtained. The SPT results in an N-value, or number of

blows required to drive the split spoon sampler 1 foot. This N-value is used to estimate the friction angle of non-cohesive soils and define the consistency of cohesive soils.

For cohesive soils, the shear strength values were determined from the Unconsolidated Undrained and Consolidated Undrained tests utilizing Shelby tubes samples.

The samples from the split spoon and Shelby tubes are submitted to the laboratory for determination of AASHTO classification, moisture content, dry density, sieve analysis, and Atterberg limits.

### **Test Results**

A summary of the lab analysis has been included in the Appendix B.

### **Proposed Structures**

Main Structure Type: 4 Span  
Foundation Type: Steel H-Piles

### **Foundation Recommendation**

#### **Steel Piling**

Pile recommendations are given as termination elevations. The pile sizes that have been analyzed are HP10x42, HP12X53, HP14x73 and HP14x102. See table 3 below for the recommended termination elevations.

The software "APile" was used in conjunction with engineering judgment and past experience in pile driving in these types of soils to estimate the pile lengths. The output from this analysis is available upon request from the NDDOT Geotechnical Section.

Below are simplified soil profiles for each boring that was used to predict the unfactored nominal geotechnical resistance in APile. A graphical representation of the unfactored nominal geotechnical resistance is also shown below in figures 1-3. The nominal geotechnical resistance is used to help predict the pile termination elevations which can be found in table 3 below.

Table 1 - Boring #1 Simplified Soil Profile-West Abutment

Layer	Depth (feet)	Elevation (feet)	Cohesion (lb/ft <sup>2</sup> )	Friction Angle	Effective Unit Weight (lb/ft <sup>3</sup> )
Loose Moist Brn Clayey Sand	0.0-2.0	1423.4-1421.4	-	$\phi=28^\circ$	-
Medium Stiff to Stiff Moist Brn/Gry Lean Clay	2.0-32.0	1421.4-1389.4	c=1200 psf	$\phi=35^\circ$	129.1
Medium Stiff to Stiff Moist Brn/Gry Lean Clay	32.0-43.0	1389.4-1380.4	c=1200 psf	$\phi=35^\circ$	66.7
Loose to Medium Dense Wet Brn/Gry Silty Sand	43.0-68.0	1380.4-1355.4	-	$\phi=32^\circ$	-
Dense Wet Gry Silty Sand	68.0-96.0	1355.4-1327.4	-	$\phi=37^\circ$	-
Hard Moist Gry Silt	96.0-106.0	1327.4-1317.4	c=4625 psf	-	-
Hard Moist Gry Fat Clay	106.0-119.5	1317.4-1303.9	c=12500 psf	-	-

Table 2 - Boring #2 Simplified Soil Profile-East Abutment

Layer	Depth (feet)	Elevation (feet)	Cohesion (lb/ft <sup>2</sup> )	Friction Angle	Effective Unit Weight (lb/ft <sup>3</sup> )
Stiff Moist Brn Lean Clay	0.0-32.0	1426.0-1394.0	c=1500 psf	$\phi=32.3^\circ$	129.4
Stiff Moist Brn Lean Clay	32.0-36.0	1394.0-1390.0	c=1500 psf	$\phi=32.3^\circ$	67
Stiff to Very Stiff Moist Brn/Gry/Blk Clayey Sand	36.0-53.0	1390.0-1373.0	-	$\phi=33^\circ$	-
Medium Dense to Dense Wet Gry Silty Sand	53.0-68.0	1373.0-1358.0	-	$\phi=33^\circ$	-
Water Bearing Coal	68.0-86.0	1358.0-1340.0	-	$\phi=34^\circ$	-
Medium Dense to Dense Wet Gry Silty Sand	86.0-101.0	1340.0-1325.0	-	$\phi=34^\circ$	-
Medium Dense to Dense Moist Gry Clayey Sand	101.0-119.6	1325.0-1306.4	c=12500 psf	-	-

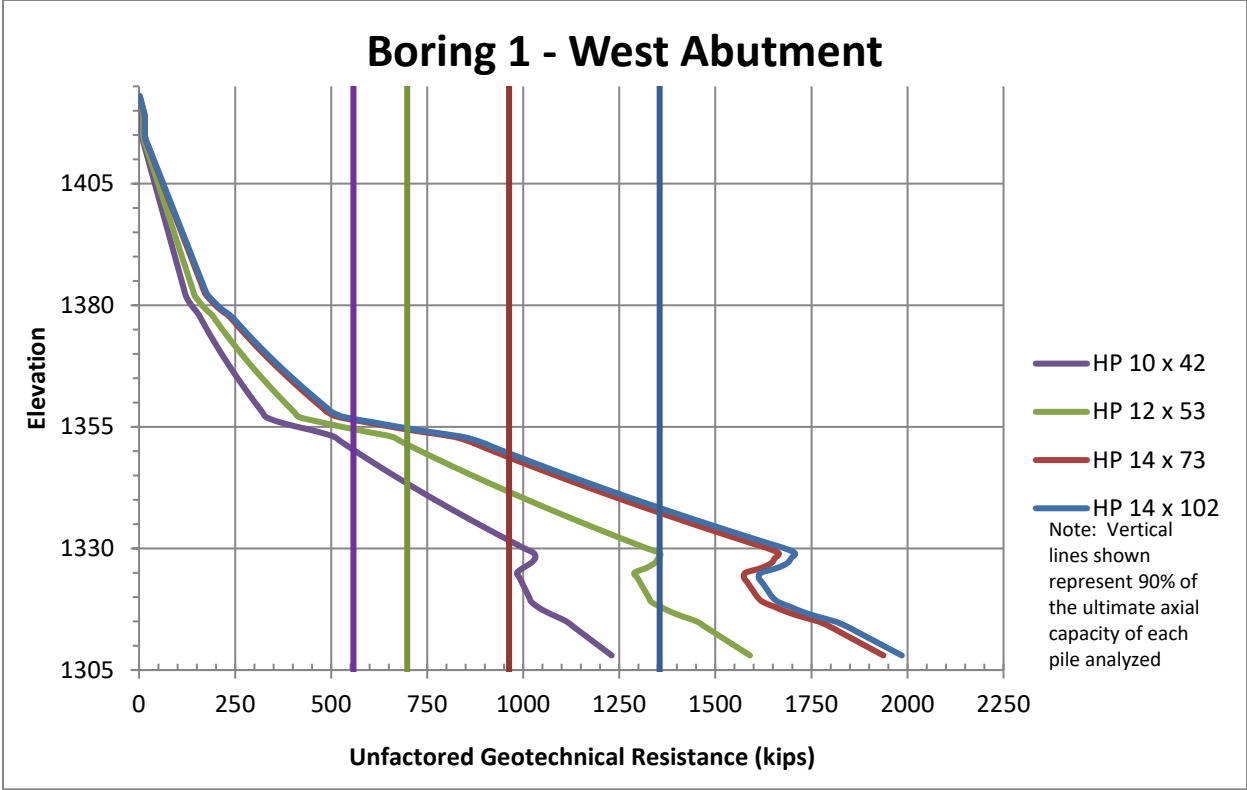


Figure 1: Nominal Resistance for Pile at West Abutment

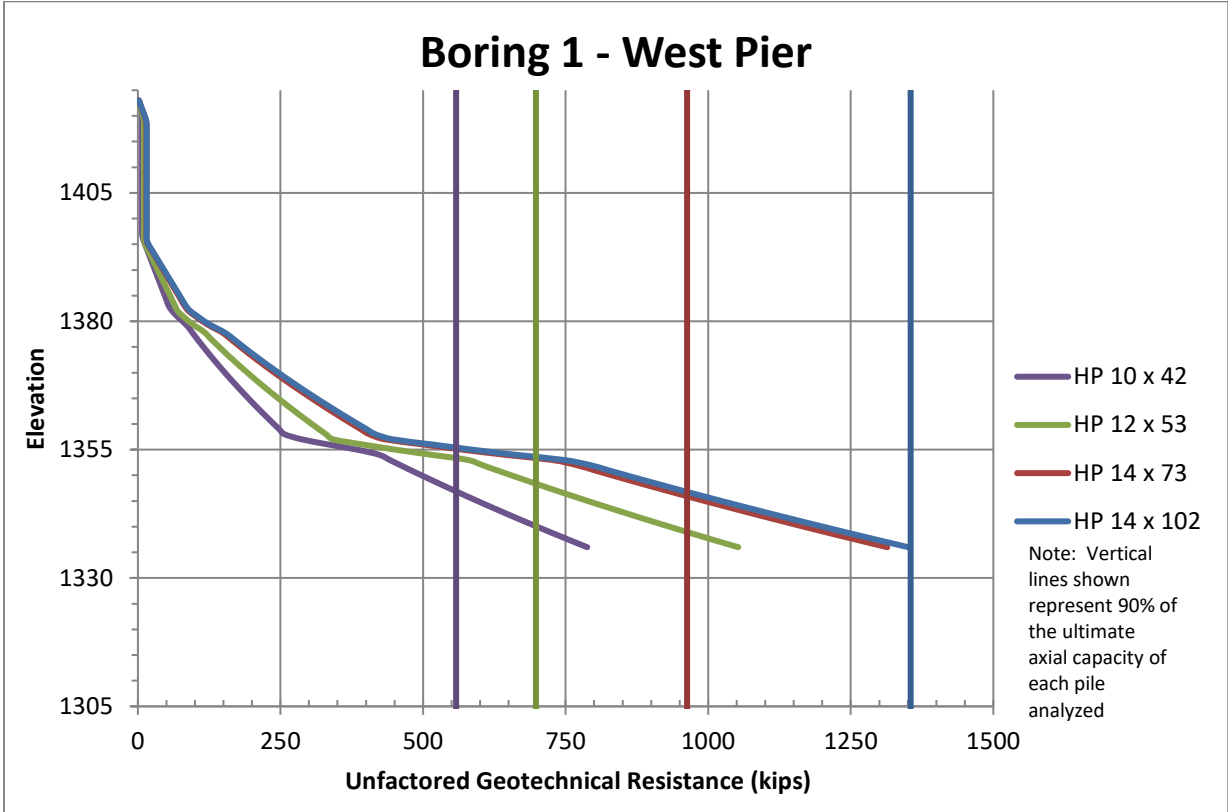


Figure 2: Nominal Resistance for Pile at West Pier



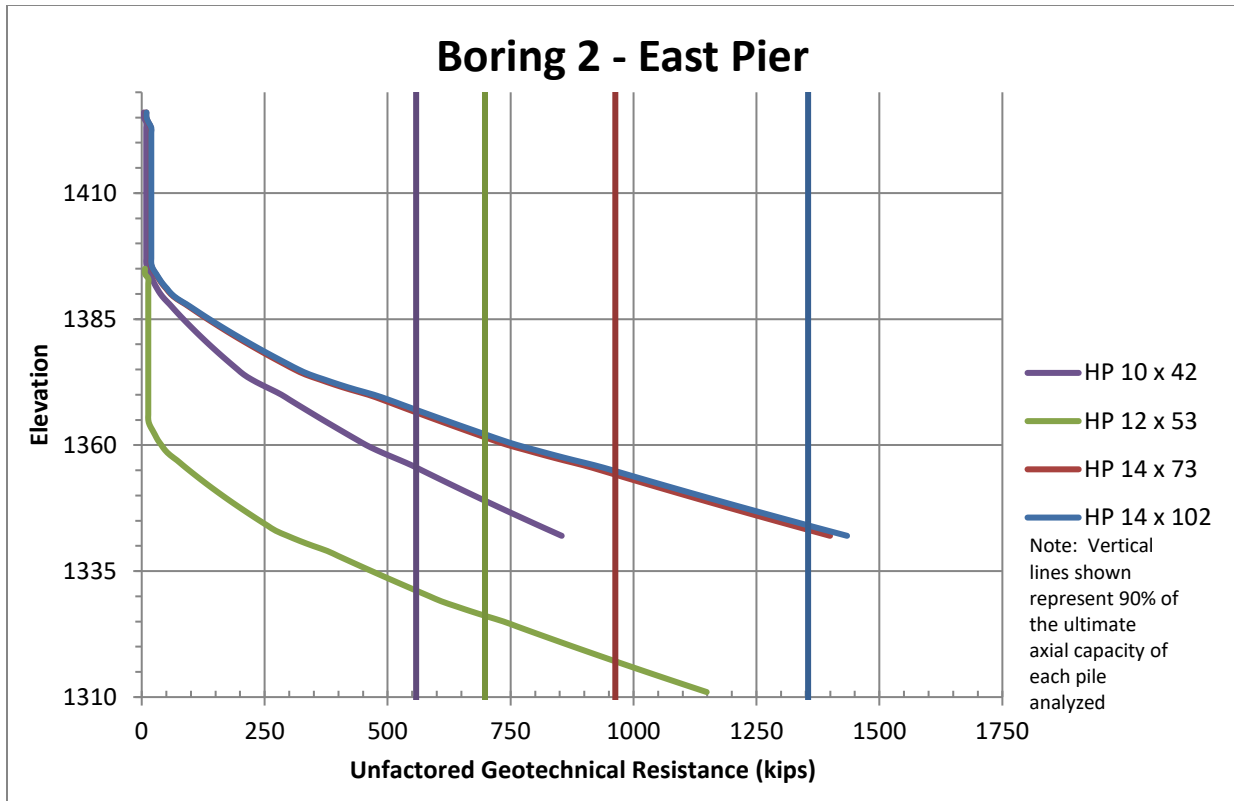


Figure 3: Nominal Resistance for Pile at East Pier

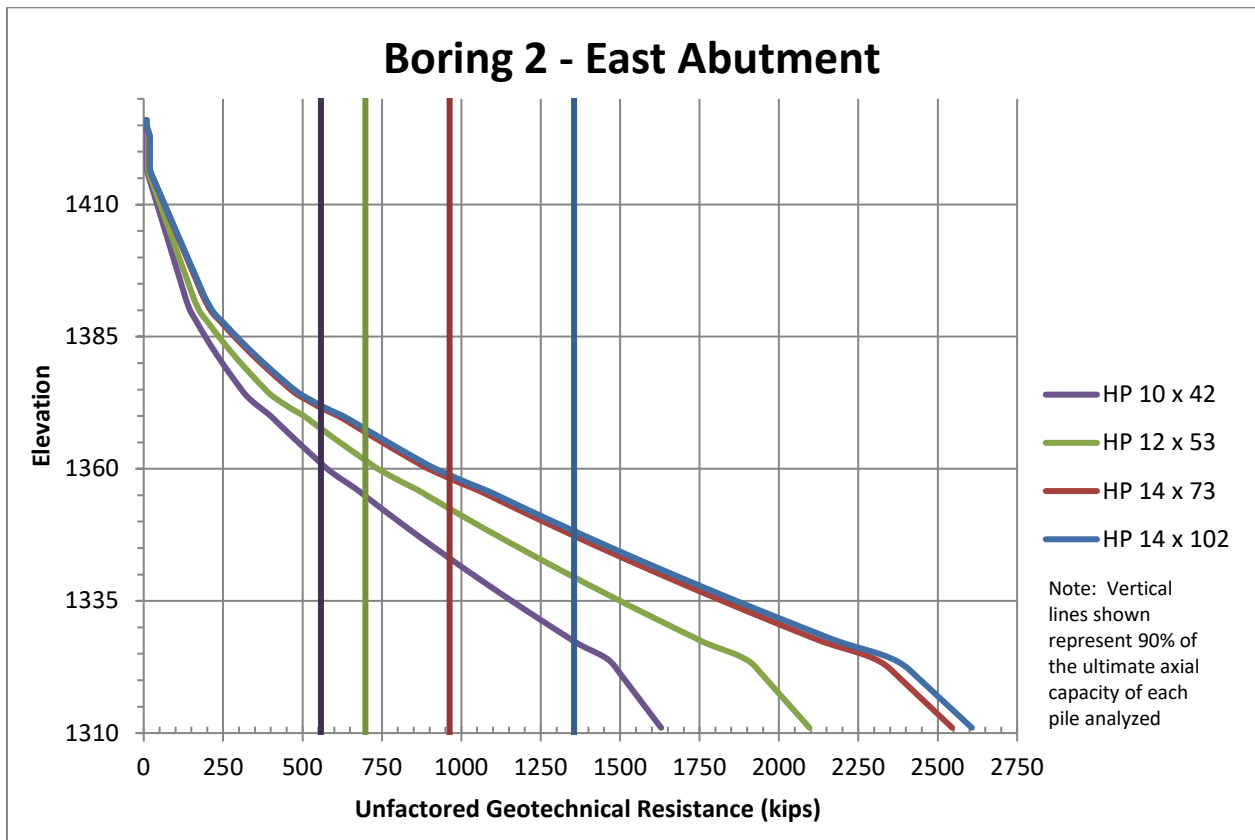


Figure 4: Nominal Resistance for Pile at East Abutment

Based on the Apile information, past experience, and engineering judgement the pile termination elevations were selected as shown in the table below.

Table 3 - Pile Length Information

Pile Type	90% Ultimate Axial Capacity	West Abutment	West & Middle Pier	East Pier	East Abutment
	kips	Termination Elevation	Termination Elevation	Termination Elevation	Termination Elevation
HP10X42	558	1350	1346	1355	1361
HP12X53	698	1351	1348	1357	1361
HP14x73	963	1347	1345	1354	1358
HP14x102	1355	1338	1335	1344	1348

Based on the sandy nature of the soil where some of the piles are estimated to reach bearing, the piles may go deeper than necessary. If the pile has not reached bearing 10 feet beyond the estimated depth, stop driving the pile and wait 24 hours to allow pile setup to occur. After 24 hours warm the hammer with a minimum of 20 blows by striking the ground or timber mats. Restrike the pile with 10 blows to determine if bearing has been achieved. If bearing was not achieved during restrike, continue to drive the pile until bearing is achieved.

### **Pile Tips**

It is not anticipated that pile tips will be required for this bridge.

### **Downdrag**

Due to the minimal amount of fill being placed at the structures downdrag is not applicable.

### **Scour**

This bridge is not over a water crossing therefore there is no scour.

### **Compaction Recommendation**

Compact roadway embankment material to at least 90 percent of the maximum dry density with moisture content no less than the optimum moisture and no more than 5.0 percentage points above the optimum moisture. The Engineer will determine the maximum dry density and optimum moisture content as specified in ND T 180.

Compact the aggregate transitions at the bridge ends according to section 714.04 A.10.

### **Slope Recommendation**

A slope stability analysis was conducted with Slope/W developed by Geo-Slope International. The analysis was a two-dimensional limit equilibrium method. Design division requested analysis of 2.5:1 and 2:1 side slopes for the bridge. Slope stability analysis shows that 2:1 slopes do not meet the required factor of safety. Based on our analysis it is recommended to construct slopes no steeper than 2.5:1. The bridge end slopes were also analyzed. It was determined that a 2.5:1 end slope will be sufficient for this structure. The stability analysis outputs can be found in Appendix C.

### **Drainage Recommendation**

The drainage off the bridge should be controlled so that water does not drain directly off the bridge ends and cause erosion around the wing-wall. A combination of inlets, downdrains, and/or deckdrains should be used to control water runoff. Water should not be allowed to drain off the bridge as to allow the in slopes and end slopes to become saturated, which is a common cause of slope failure.

### **Settlement Analysis/Recommendation**

Minimal fill is proposed to be placed with this project, therefore settlement mitigation is not recommended.

### **Pre-Boring**

Minimal fill is proposed to be placed with this project, therefore pre-boring is not recommended.

# APPENDIX A

## Boring Logs

PROJECT NUMBER IM-2-094(194)260 DATE STARTED \_\_\_\_\_ COMPLETED \_\_\_\_\_  
 PCN 23577 ELEVATION 1426 ft  
 LOCATION Stutsman County  
 DRILLED BY Dallan LOGGED BY Jamie DRILLING METHOD \_\_\_\_\_  
 ENGINEER \_\_\_\_\_  
 NOTES East abutment

ELEVATION (ft)	DEPTH (ft)	MATERIAL DESCRIPTION	GRAPHIC LOG	AASHTO	USCS	SAMPLE TYPE & NUMBER	RECOVERY (%)	SPT N VALUE	MC		TESTS & REMARKS	
									PL	LL		
1420	0	Stiff Moist Brn Lean Clay		A-6(5) SC	CL	378	75	10	19	37		
				A-6(10) CL	CL	379	60	6	17	38	Y=126.5 pcf, UC=4136 psf, c=2068 psf	
	10			A-7-6(10) CL	CL	380	35	6	20	41		
				A-6(10) CL	CL	381	60	6	17	40	Y=127.1 pcf, UC=1456 psf, c=728psf	
				A-6(8) CL	CL	382	50	6	18	38		
1410	20			A-6(11) CL	CL	383	75	11	16	40	Y=129.9 pcf, UU=1532 psf	
				A-6(9) CL	CL	384	85	9	18	38		
				A-6(9) CL	CL	385	75	9	16	37	Y=131.9 pcf, UU=2618psf	
				A-6(9) CL	CL	386	50	9	19	40		
				A-7-6(7) SC	SC	387	75	10	19	42	Y=129.2 pcf, UC=3653 psf, c=1826 psf	
				A-7-6(1) CL	CL	388	75	10	18	41		
	30			A-6(10) CL	CL	389	60	11	17	38	Y=131.3 pcf, UC=3243 psf, c=1621 psf	
				A-6(10) CL	CL	390	85	11	17	39		
1390	36.0	1390.0 ft	36.0 ft	A-6(11) CL	CL	391	85	14	16	39	Y=129.6 pcf, CU=32.3°, c=155 psf	
		Stiff to Very Stiff Moist Brn/Gry/Blk Clayey Sand		A-2-6(0) SC	SC	392	85	14	17	28		
	40			A-2-6(0) SC	SC	393	100	31	19	31		
				A-2-4(0) SC	SC	394	100	31	2	31		
				A-6(3) SC	SC	395	90	23	21	36		
				A-6(3) SC	SC	396	100	27	12	27		
	50			A-4(0) SC	SC	397	100	14	2	29		
				A-4(2) CL	CL	398	85	15	2	31		
		1373.0 ft	53.0 ft	A-2-4(0) SC	SC	399	85	7	18	33		
		Medium Dense to Dense Wet Gry Silty Sand		A-6(9) CL	CL	400	100	16	0	1		
	60			A-1-b(SW-SM)	SM	401	85	42	0	1		
				A-1-a(SW-SM)	SM	402	85	36	0	1		
				A-1-b(SW-SM)	SM	403	85	37	2	23		
1360	68.0	1358.0 ft	68.0 ft	A-2-6(0) SC	SC	404	90	27	2	30		
				A-1-b(SW-SM)	SM	405	90	49	2	30		
	70	Water Bearing Coal				406	25	16	42	29		
						407	85	16	46	46		
						408	65	26	46	46		
						409	65	26	30	30		
						410	90	24	24	24		
						411	90	24	24	24		
1340	86.0	1340.0 ft	86.0 ft			412	100	22	0	0		
		Medium Dense to Dense Wet Gry Silty Sand		A-3(1)SP-SM	SM	413	75	46	0	0		
				A-3(1)SP-SM	SM	414	90	21	0	0		
1330	101.0	1325.0 ft	101.0 ft	A-2-4(0) SM	SM	415	95	47	1	27		
		Medium Dense to Dense Moist Gry Clayey Sand		A-4(1) SC	SC	416	95	47	100	17	31	
	110			A-6(2) SC	SC	417	100	100	27	50		
				A-7-6(8) CH	CH	418	100	100	34	66		
1310	119.6	1306.4 ft	119.6 ft	A-7-5(1)MH	MH	419	67	100	34	66		
Bottom of borehole at 119.6 ft												

NDDOT LOG - NDDOT\_DATA\TEMP\_20180208\GDT - 7/13/23 11:26 - R:\PROJECT\20094260\_194\MATERIAL\GEOTECH\DEEP FOUNDATION\VOID\BORING 2.GPJ



PROJECT NUMBER IM-2-094(194)260 DATE STARTED \_\_\_\_\_ COMPLETED \_\_\_\_\_  
 PCN 23577 ELEVATION 1423.4 ft  
 LOCATION Stutsman County  
 DRILLED BY Dallan LOGGED BY Jamie DRILLING METHOD \_\_\_\_\_  
 ENGINEER \_\_\_\_\_  
 NOTES West

NDDOT LOG - NDDOT\_TEMP\_20180208\_GDT - 7/13/23 08:32 - R:\PROJECT\20094260\_194\MATERIAL\GEOTECH\DEEP FOUNDATION\VOID\BORING 1.GPJ

ELEVATION (ft)	DEPTH (ft)	MATERIAL DESCRIPTION	GRAPHIC LOG	AASHTO	USCS	SAMPLE TYPE & NUMBER	RECOVERY (%)	SPT N VALUE	CLAY FRACTION (%)		TESTS & REMARKS	
									PL	LL		
1420	0	Loose Moist Brn Clayey Sand		A-2-4(0)	SC	337	50	7	20	37		
1420	2.0	1421.4 ft		A-6(8)	CL	338	50	7	20	37		
1410	10	Medium stiff to stiff Moist Brn/Gry Lean Clay		A-7-6(10)	CL	339	75	6	20	42	Y=129.2 pcf, UC= 4912 psf, c=2457 psf	
1410	10			A-7-6(1)	CL	340	75	6	20	42		
1410	10			A-6(9)	CL	341	75	5	20	40		
1410	10			A-6(10)	CL	342	85	5	20	40		
1410	10			A-7-6(10)	CL	343	75	5	20	42	Y=128.6 pcf, CU= 35° c=180 psf	
1410	10			A-6(9)	CL	344	75	5	20	42		
1410	10			A-7-6(1)	CL	345	60	9	20	42	Y=129.3 pcf, UC= 3336 psf, c=1669 psf	
1410	10			A-7-6(1)	CL	346	65	9	20	42		
1410	10			A-6(9)	CL	347	75	15	20	38	Y=129.9 pcf, UC=2416 psf, c=1208psf	
1410	10			A-6(9)	CL	348	75	15	20	42		
1410	10			A-7-6(10)	CL	349	75	14	20	38	Y=128.4 pcf, c=1977 psf	
1410	10			A-6(10)	CL	350	75	14	20	38		
1410	10			A-7-6(1)	CL	351	100	27	20	41	Y=129.0 pcf, c=3210 psf	
1410	10			A-6(9)	CL	352	85	27	20	38		
1410	10			A-6(6)	CL	353	100	7	20	34		
1410	10			A-6(10)	CL	354	100	8	20	37		
1410	10	1380.4 ft	43.0 ft	A-1-b(1)	SP-SM	355	15	17	0	30		
1410	10	Loose to medium dense Wet Brn/Gry Silty Sand		A-1-b(1)	SP-SM	356	85	7	0	30		
1410	10			A-1-b(1)	SP	357	85	27	0	30		
1410	10			A-3(1)	SP-SM	358	85	21	0	30		
1410	10			A-1-b(1)	SP-SM	359	100	21	0	30		
1410	10			A-1-a(1)	SP-SM	360	100	66	0	30		
1410	10			A-2-4(1)	SW-SM	362	5	7	0	32		
1410	10	1355.4 ft	68.0 ft	A-2-4(1)	SW-SM	363	65	14	0	32		
1410	10	Dense Wet Gry Silty Sand		A-1-b(0)	SM	364	80	32	0	30		
1410	10			A-1-b(0)	SM	365	75	25	0	30		
1410	10			A-2-4(0)	SP-SM	366	75	33	0	30		
1410	10			A-2-4(0)	SM	367	75	46	0	30		
1410	10			A-2-4(0)	SM	368	50	48	0	30		
1410	10			A-2-4(0)	SM	369	75	54	0	30		
1410	10					370	5	52	0	30		
1410	10			A-2-4(0)	SM	371	100	52	0	30		
1410	10	1327.4 ft	96.0 ft	A-2-4(0)	SM	372	100	38	0	30		
1410	10	Hard Moist Gry Silt		A-6(5)	SC	373	100	37	0	34		
1410	10	1317.4 ft	106.0 ft	A-4(0)	CL-ML	374	100	63	0	25		
1410	10	Hard Moist Gry Fat Clay		A-6(4)	SC	375	77	100	22	37		
1410	10			A-7-6(16)	CH	376	100	100	27	60		
1410	10	1303.9 ft	119.5 ft	A-7-6(2)	CH	377	100	100	26	69		
		Bottom of borehole at 119.5 ft										

# APPENDIX B

## Lab Results



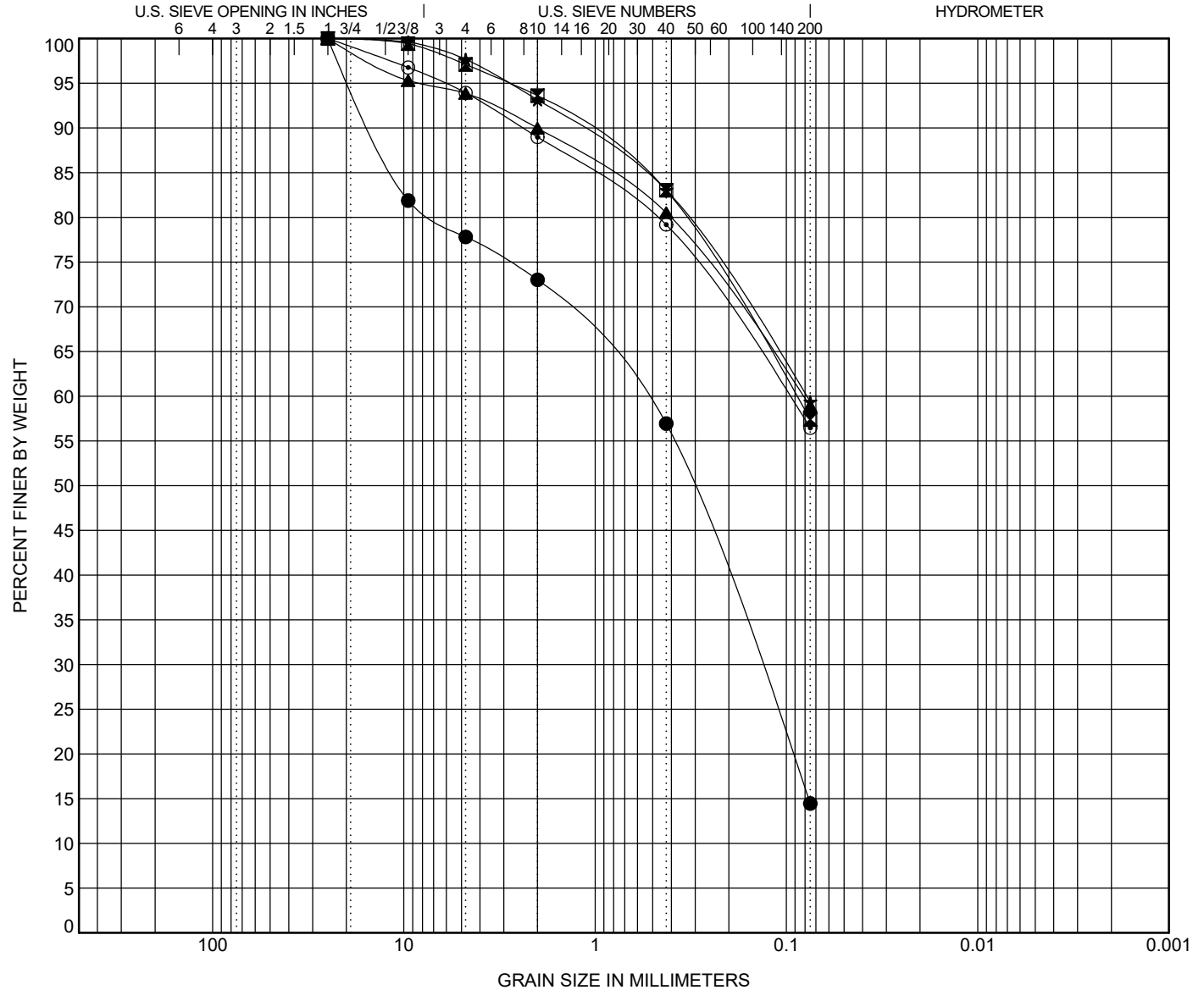
NORTH DAKOTA DEPARTMENT OF TRANSPORTATION  
300 AIRPORT ROAD  
BISMARCK, ND 58504

# GRAIN SIZE DISTRIBUTION

PROJECT NUMBER IM-2-094(194)260

LOCATION Stutsman County

PCN 23577



COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

BOREHOLE	DEPTH	AASHTO Classification	USCS Classification	LL	PL	PI	Cc	Cu
● B1	0.0	A-2-6 (0)	SC	37	20	17		
☒ B1	2.0	A-6 (8)	CL	37	18	19		
▲ B1	4.0	A-7-6 (11)	CL	42	19	23		
★ B1	6.0	A-7-6 (11)	CL	42	18	24		
◎ B1	9.0	A-6 (9)	CL	40	18	22		

BOREHOLE	DEPTH	D100	D50	D30	D15	%Gravel	%Sand	%Silt	%Clay
● B1	0.0	25	0.32	0.141	0.077	22.2	63.4	14.5	
☒ B1	2.0	25				2.9	39.7	57.4	
▲ B1	4.0	25				6.1	35.0	58.9	
★ B1	6.0	25				2.3	38.3	59.4	
◎ B1	9.0	25				6.1	37.5	56.5	

GRAIN SIZE D ADJUSTED - 20171219.GDT - 7/13/23 08:33 - R:\PROJECT\20094260-194\MATERIAL\GEOTECH\DEEP FOUNDATION\VOIDIBORING 1.GPJ





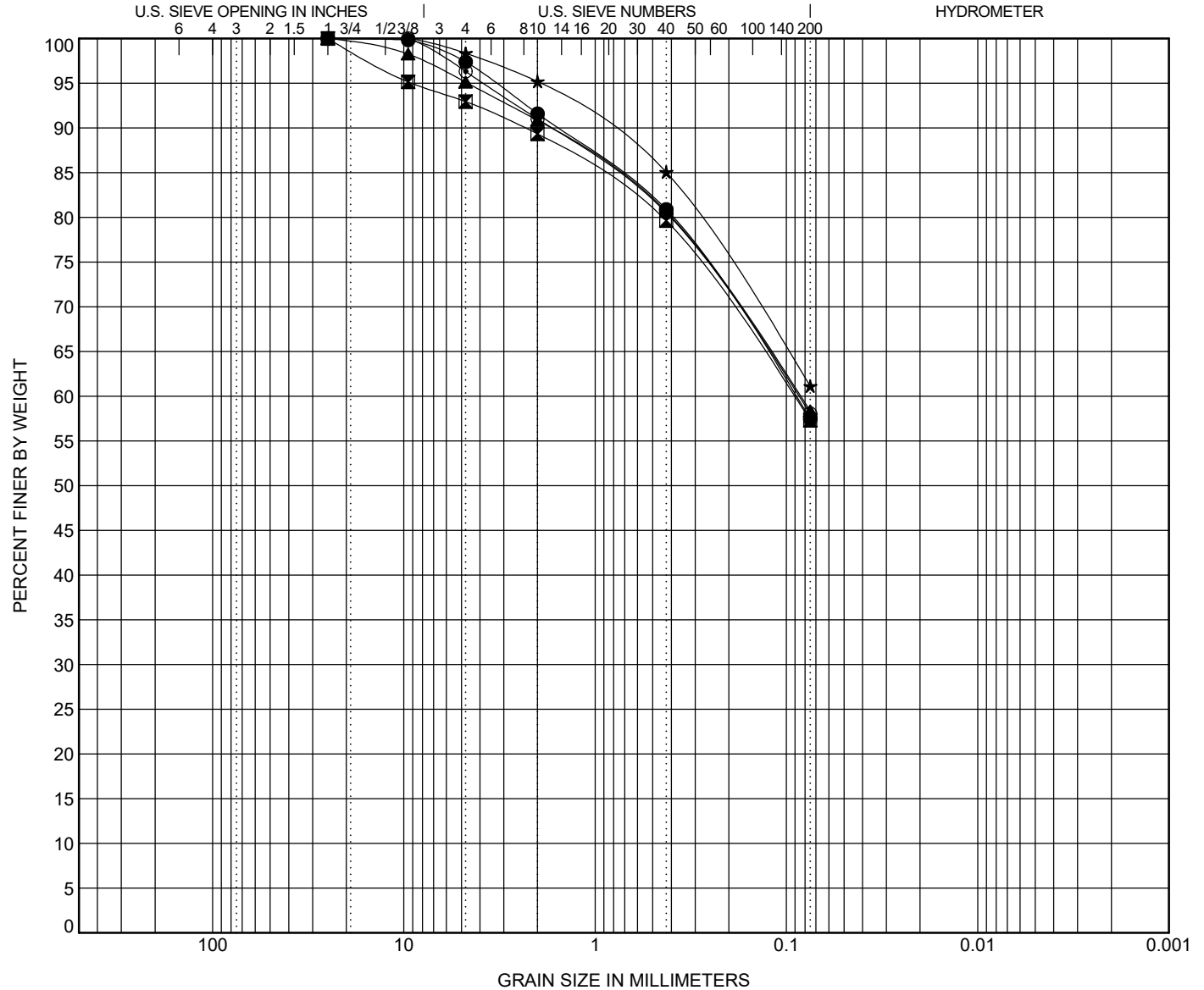
NORTH DAKOTA DEPARTMENT OF TRANSPORTATION  
300 AIRPORT ROAD  
BISMARCK, ND 58504

# GRAIN SIZE DISTRIBUTION

PROJECT NUMBER IM-2-094(194)260

LOCATION Stutsman County

PCN 23577



COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

BOREHOLE	DEPTH	AASHTO Classification	USCS Classification	LL	PL	PI	Cc	Cu
● B1	11.0	A-6 (10)	CL	40	17	23		
☒ B1	14.0	A-7-6 (10)	CL	42	20	22		
▲ B1	16.0	A-6 (9)	CL	40	19	21		
★ B1	19.0	A-7-6 (11)	CL	42	20	22		
◎ B1	21.0	A-7-6 (11)	CL	42	18	24		

BOREHOLE	DEPTH	D100	D50	D30	D15	%Gravel	%Sand	%Silt	%Clay
● B1	11.0	25				2.6	39.9	57.5	
☒ B1	14.0	25				7.0	35.7	57.3	
▲ B1	16.0	25				4.9	36.8	58.4	
★ B1	19.0	25				1.6	37.2	61.1	
◎ B1	21.0	25				3.7	38.3	58.1	

GRAIN SIZE D ADJUSTED - 20171219.GDT - 7/13/23 08:33 - R:\PROJECT\20094260-194\MATERIAL\GEOTECH\DEEP FOUNDATION\VOIDIBORING 1.GPJ

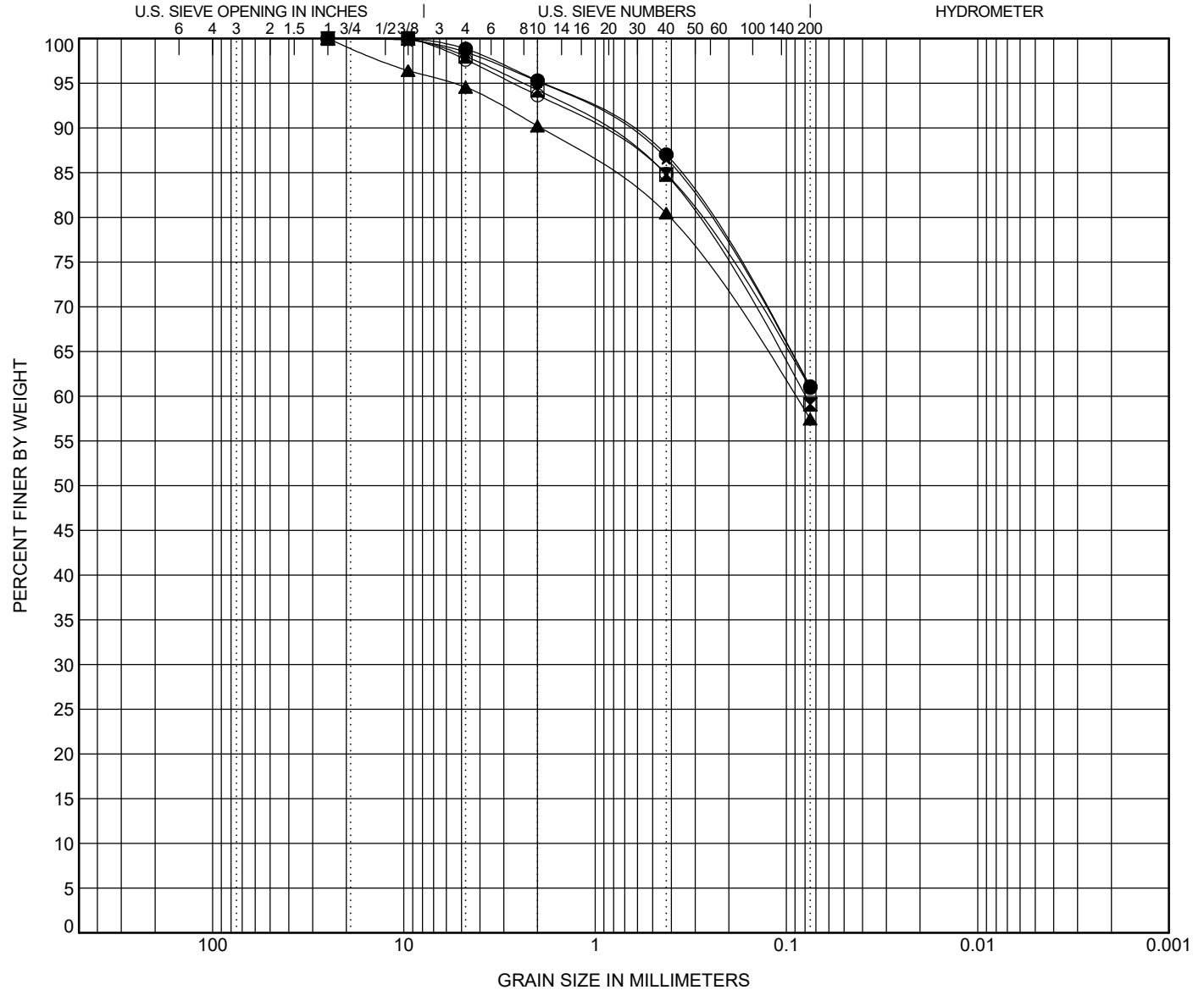


# GRAIN SIZE DISTRIBUTION

PROJECT NUMBER IM-2-094(194)260

LOCATION Stutsman County

PCN 23577



COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

BOREHOLE	DEPTH	AASHTO Classification	USCS Classification	LL	PL	PI	Cc	Cu
● B1	24.0	A-6 (10)	CL	38	18	20		
☒ B1	26.0	A-6 (9)	CL	37	17	20		
▲ B1	29.0	A-7-6 (10)	CL	42	19	23		
★ B1	31.0	A-6 (10)	CL	38	17	21		
◎ B1	34.0	A-7-6 (11)	CL	41	19	22		

BOREHOLE	DEPTH	D100	D50	D30	D15	%Gravel	%Sand	%Silt	%Clay
● B1	24.0	25				1.2	37.8	61.1	
☒ B1	26.0	25				2.0	38.9	59.1	
▲ B1	29.0	25				5.4	37.1	57.4	
★ B1	31.0	25				1.5	37.4	61.1	
◎ B1	34.0	25				2.4	36.7	60.9	

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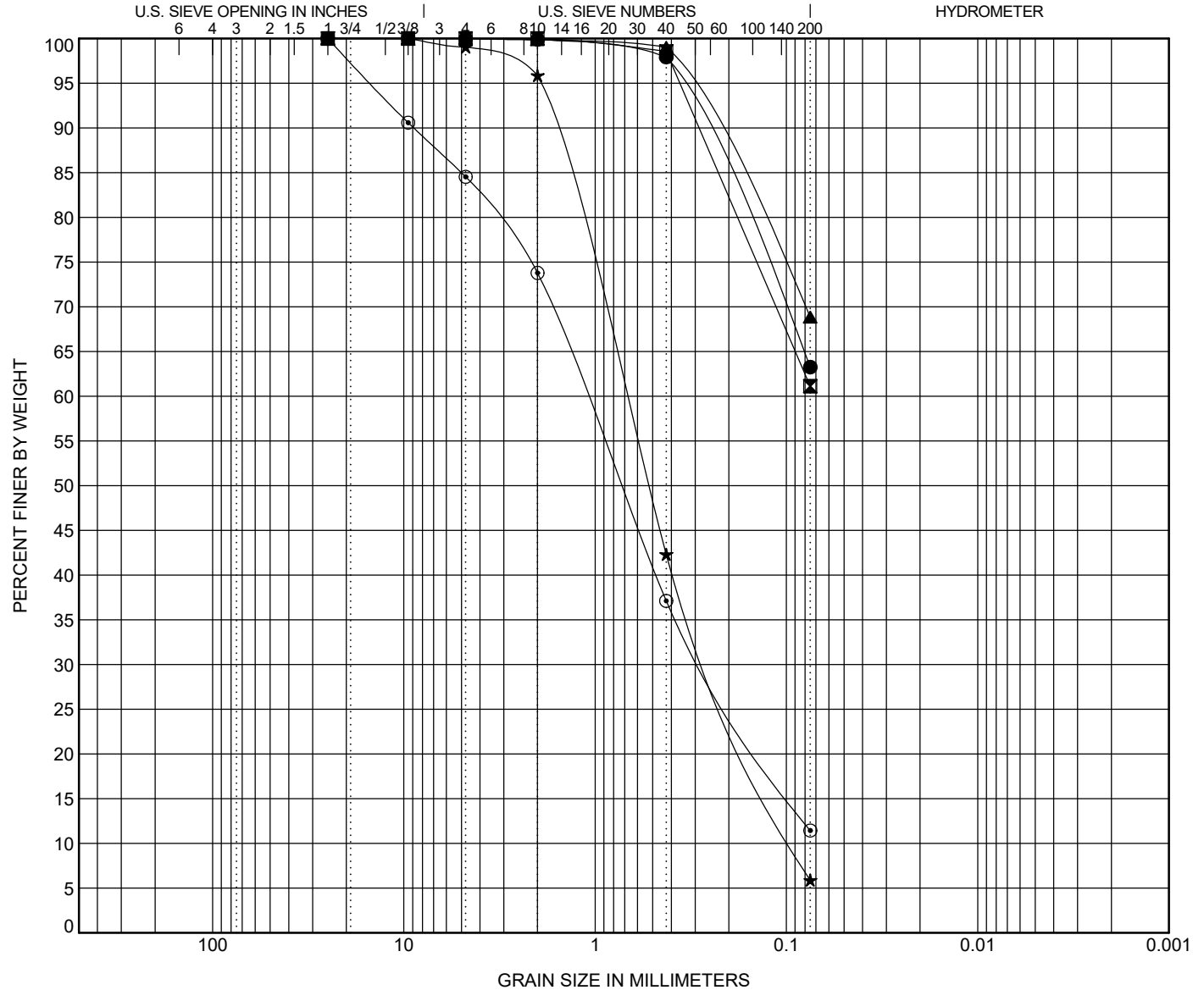
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BISMARCK, ND 58504

# GRAIN SIZE DISTRIBUTION

PROJECT NUMBER IM-2-094(194)260

LOCATION Stutsman County

PCN 23577



COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

BOREHOLE	DEPTH	AASHTO Classification	USCS Classification			LL	PL	PI	Cc	Cu
● B1	36.0	A-6 (9)	CL			38	21	17		
☒ B1	39.0	A-6 (6)	CL			34	20	14		
▲ B1	41.0	A-6 (11)	CL			37	19	18		
★ B1	44.0	A-1-b (0)	SP-SM			NP	NP	NP	0.86	7.77
⊙ B1	49.0	A-1-b (0)	SP-SM			NP	NP	NP	0.91	16.41

BOREHOLE	DEPTH	D100	D50	D30	D15	%Gravel	%Sand	%Silt	%Clay
● B1	36.0	25				0.0	36.7	63.3	
☒ B1	39.0	25				0.0	38.9	61.1	
▲ B1	41.0	25				0.0	31.2	68.8	
★ B1	44.0	25	0.531	0.236	0.116	0.9	93.2	5.9	
⊙ B1	49.0	25	0.732	0.263	0.095	15.5	73.1	11.4	

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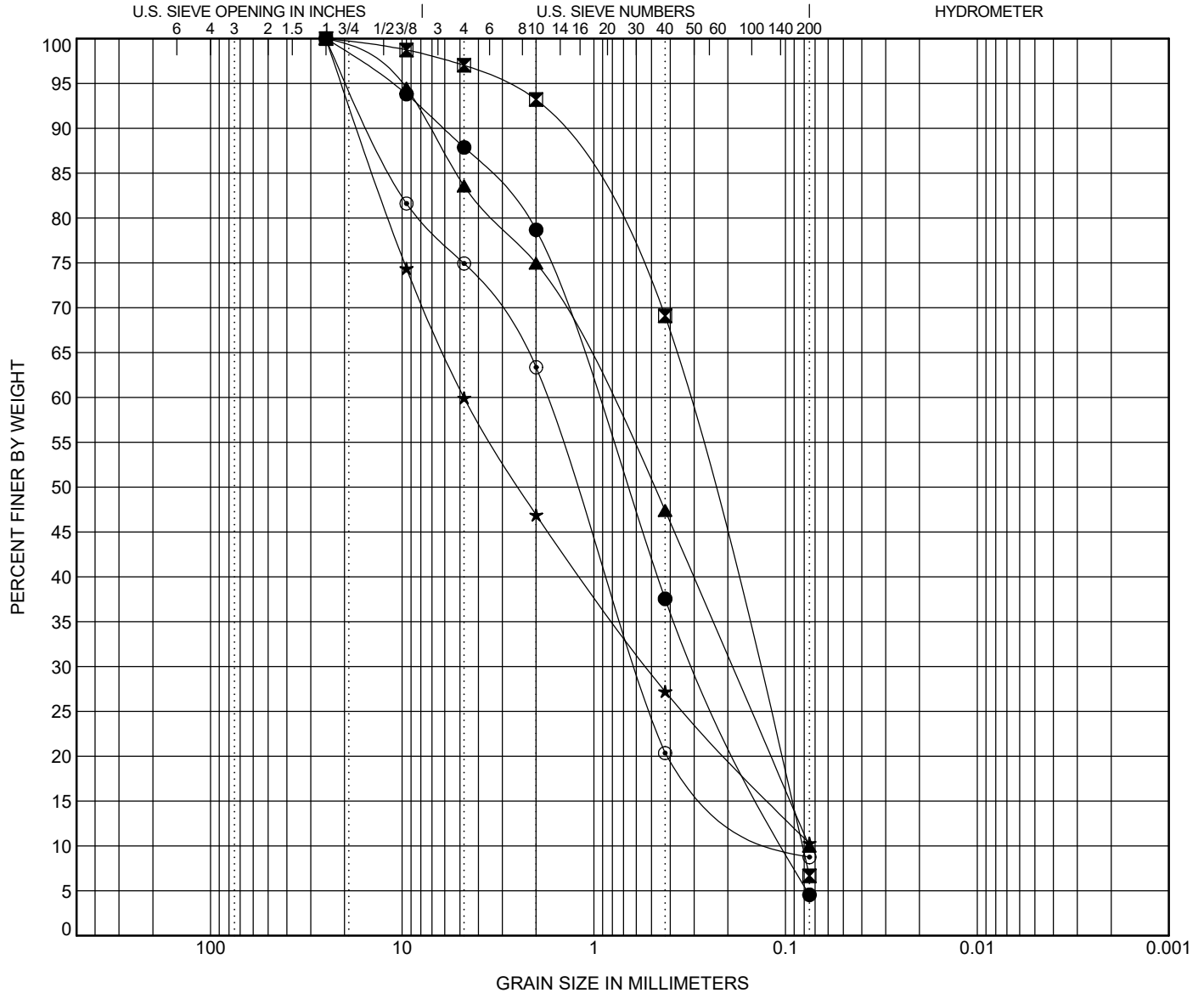
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# GRAIN SIZE DISTRIBUTION

PROJECT NUMBER IM-2-094(194)260

LOCATION Stutsman County

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COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

BOREHOLE	DEPTH	AASHTO Classification	USCS Classification			LL	PL	PI	Cc	Cu
● B1	51.0	A-2-4 (0)	SP			30	21	9	0.83	9.91
■ B1	54.0	A-3 (0)	SP-SM			NP	NP	NP	0.76	4.01
▲ B1	56.0	A-1-b (0)	SP-SM			NP	NP	NP	0.56	11.48
★ B1	59.0	A-1-a (0)	SP-SM			NP	NP	NP	0.81	65.53
⊙ B1	64.0	A-2-4 (0)	SW-SM			32	24	8	2.26	19.62

BOREHOLE	DEPTH	D100	D50	D30	D15	%Gravel	%Sand	%Silt	%Clay
● B1	51.0	25	0.679	0.286	0.13	12.1	83.3	4.5	
■ B1	54.0	25	0.25	0.143	0.095	3.0	90.4	6.7	
▲ B1	56.0	25	0.492	0.19	0.095	16.4	73.6	10.0	
★ B1	59.0	25	2.457	0.529	0.121	40.1	49.6	10.3	
⊙ B1	64.0	25	1.236	0.602	0.191	25.1	66.2	8.8	

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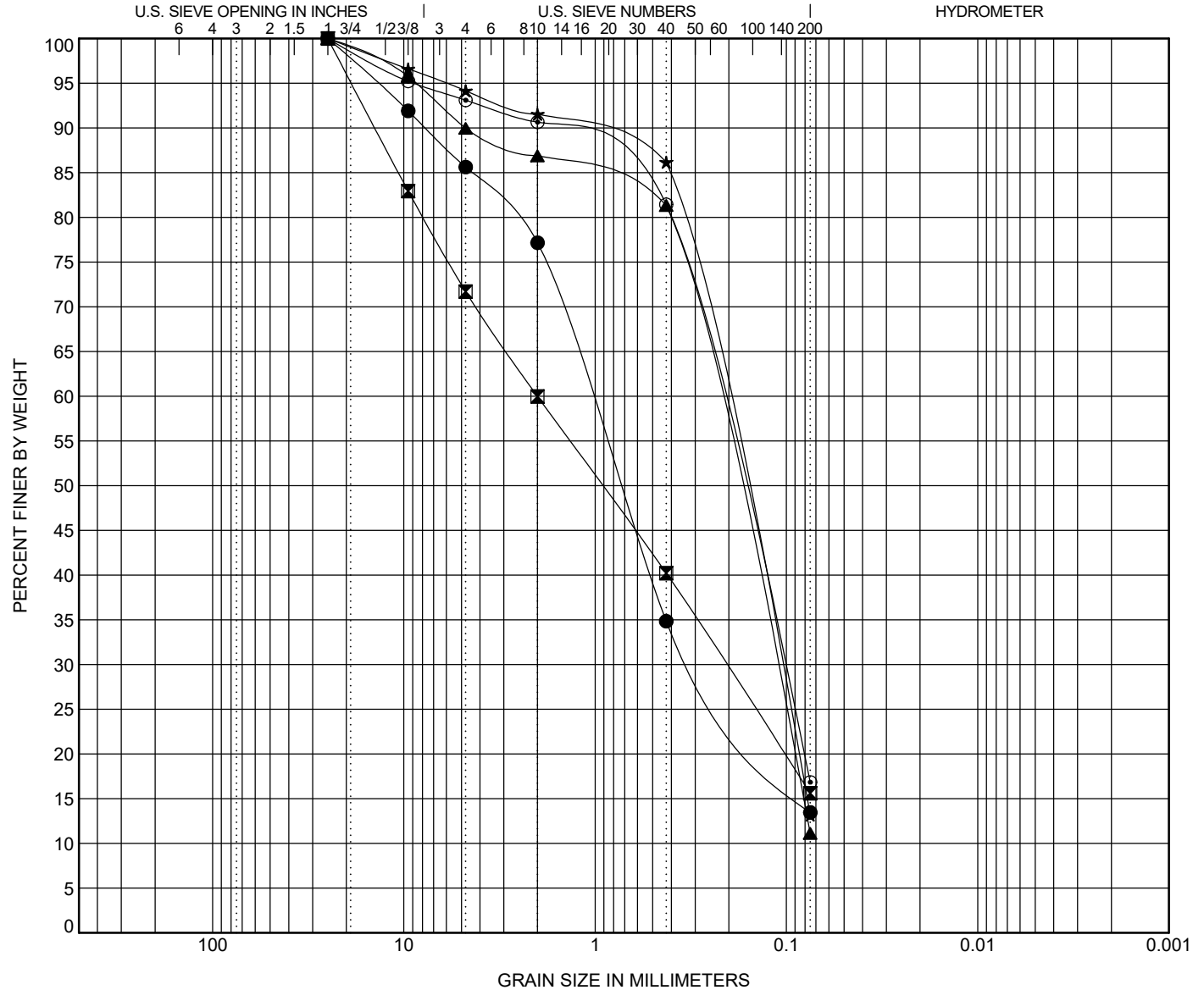


# GRAIN SIZE DISTRIBUTION

PROJECT NUMBER IM-2-094(194)260

LOCATION Stutsman County

PCN 23577



COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

BOREHOLE	DEPTH	AASHTO Classification	USCS Classification			LL	PL	PI	Cc	Cu
● B1	69.0	A-1-b (0)	SM			30	24	6		
☒ B1	71.0	A-1-b (0)	SM			NP	NP	NP		
▲ B1	74.0	A-2-4 (0)	SP-SM			NP	NP	NP	0.78	3.44
★ B1	76.0	A-2-4 (0)	SM			NP	NP	NP		
⊙ B1	79.0	A-2-4 (0)	SM			NP	NP	NP		

BOREHOLE	DEPTH	D100	D50	D30	D15	%Gravel	%Sand	%Silt	%Clay
● B1	69.0	25	0.74	0.287	0.085	14.4	72.2	13.5	
☒ B1	71.0	25	0.915	0.207		28.3	56.1	15.6	
▲ B1	74.0	25	0.196	0.12	0.083	10.0	78.8	11.1	
★ B1	76.0	25	0.18	0.112	0.078	5.8	81.0	13.2	
⊙ B1	79.0	25	0.183	0.107		6.9	76.3	16.8	

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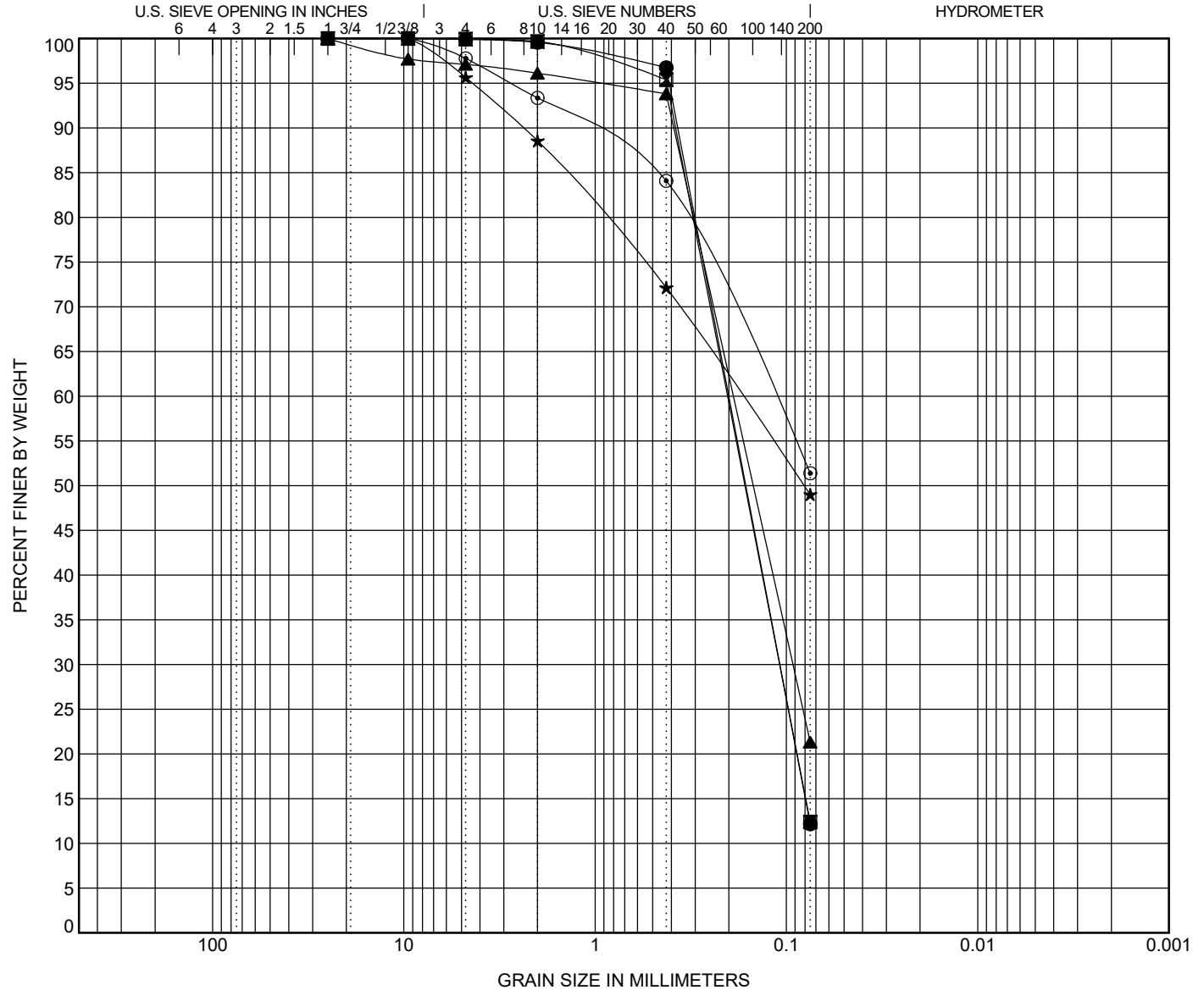
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BISMARCK, ND 58504

# GRAIN SIZE DISTRIBUTION

PROJECT NUMBER IM-2-094(194)260

LOCATION Stutsman County

PCN 23577



COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

BOREHOLE	DEPTH	AASHTO Classification	USCS Classification			LL	PL	PI	Cc	Cu
● B1	81.0	A-2-4 (0)	SM			NP	NP	NP	0.81	2.79
☒ B1	89.0	A-2-4 (0)	SM			NP	NP	NP	0.81	2.84
▲ B1	94.0	A-2-4 (0)	SM			NP	NP	NP		
★ B1	99.0	A-6 (4)	SC			34	18	16		
◎ B1	104.0	A-4 (0)	CL-ML			25	21	4		

BOREHOLE	DEPTH	D100	D50	D30	D15	%Gravel	%Sand	%Silt	%Clay
● B1	81.0	25	0.163	0.108	0.079	0.1	87.7	12.2	
☒ B1	89.0	25	0.165	0.108	0.079	0.1	87.5	12.4	
▲ B1	94.0	25	0.149	0.092		2.9	75.8	21.3	
★ B1	99.0	25	0.081			4.4	46.6	49.0	
◎ B1	104.0	25				2.2	46.4	51.4	

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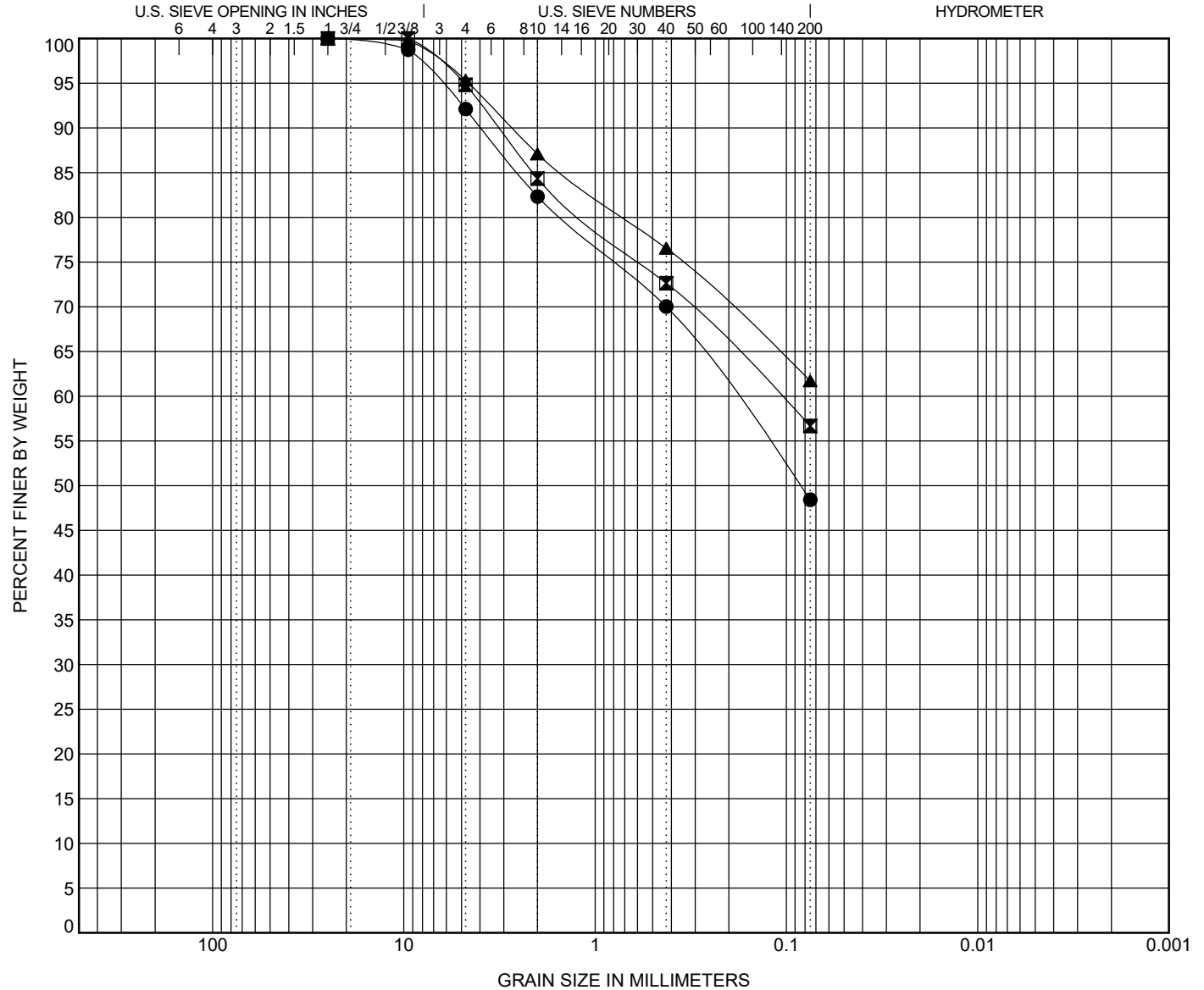
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 BISMARCK, ND 58504

# GRAIN SIZE DISTRIBUTION

PROJECT NUMBER IM-2-094(194)260

LOCATION Stutsman County

PCN 23577



COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

BOREHOLE	DEPTH	AASHTO Classification	USCS Classification	LL	PL	PI	Cc	Cu
● B1	109.0	A-6 (4)	SC	37	22	15		
■ B1	114.0	A-7-6 (16)	CH	60	27	33		
▲ B1	119.0	A-7-6 (25)	CH	69	26	43		

BOREHOLE	DEPTH	D100	D50	D30	D15	%Gravel	%Sand	%Silt	%Clay
● B1	109.0	25	0.085			7.9	43.7	48.4	
■ B1	114.0	25				5.2	38.2	56.7	
▲ B1	119.0	25				4.7	33.6	61.7	

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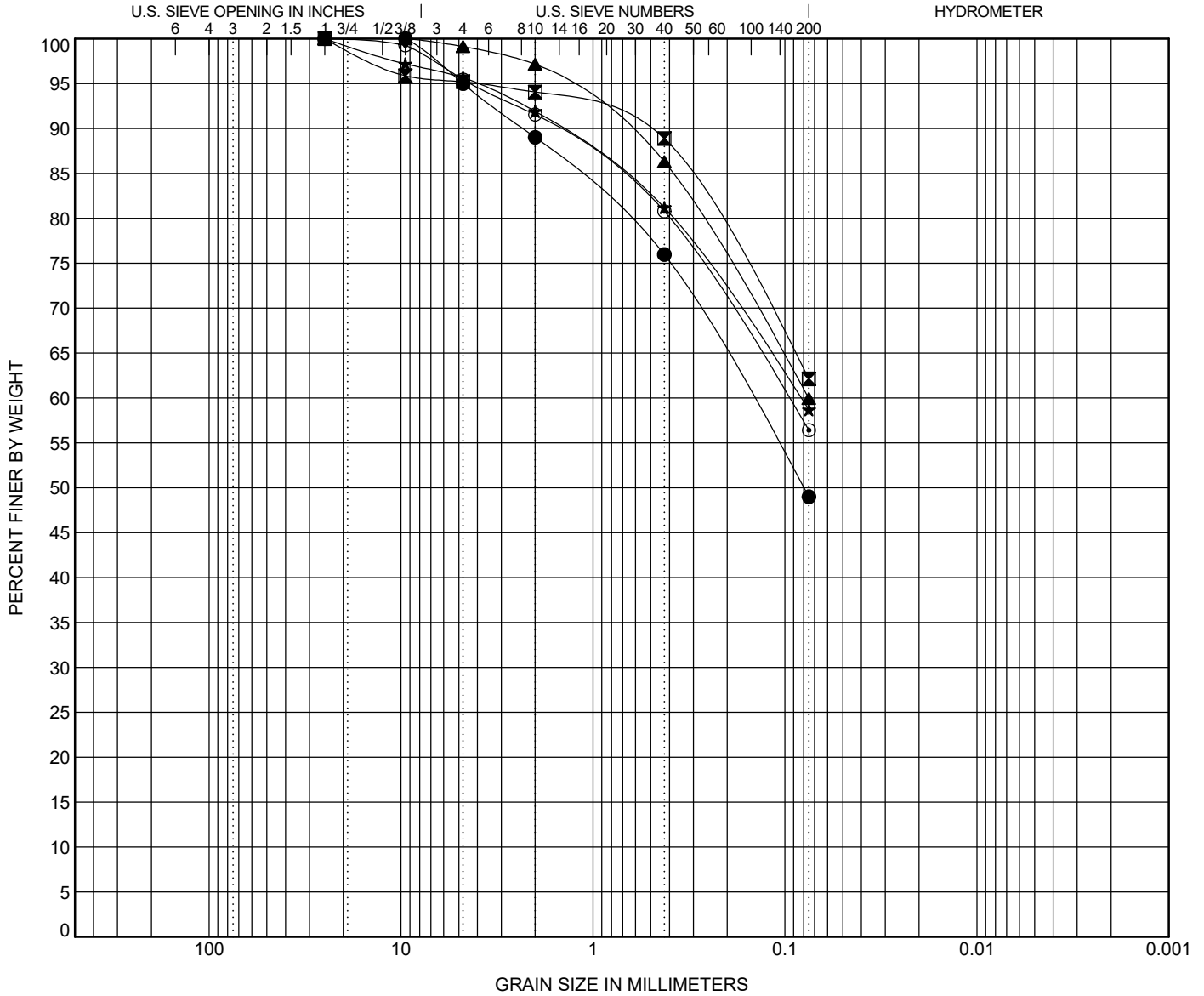
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# GRAIN SIZE DISTRIBUTION

PROJECT NUMBER IM-2-094(194)260

LOCATION Stutsman County

PCN 23577



COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

BOREHOLE	DEPTH	AASHTO Classification	USCS Classification		LL	PL	PI	Cc	Cu
● B2	0.0	A-6 (5)	SC		37	19	18		
☒ B2	4.0	A-6 (10)	CL		38	17	21		
▲ B2	6.0	A-7-6 (10)	CL		41	20	21		
★ B2	9.0	A-6 (11)	CL		40	17	23		
◎ B2	11.0	A-6 (8)	CL		38	18	20		

BOREHOLE	DEPTH	D100	D50	D30	D15	%Gravel	%Sand	%Silt	%Clay
● B2	0.0	25	0.08			5.0	46.0	49.0	
☒ B2	4.0	25				4.8	33.1	62.1	
▲ B2	6.0	25				0.9	39.2	59.9	
★ B2	9.0	25				4.4	36.9	58.7	
◎ B2	11.0	25				4.6	39.0	56.4	

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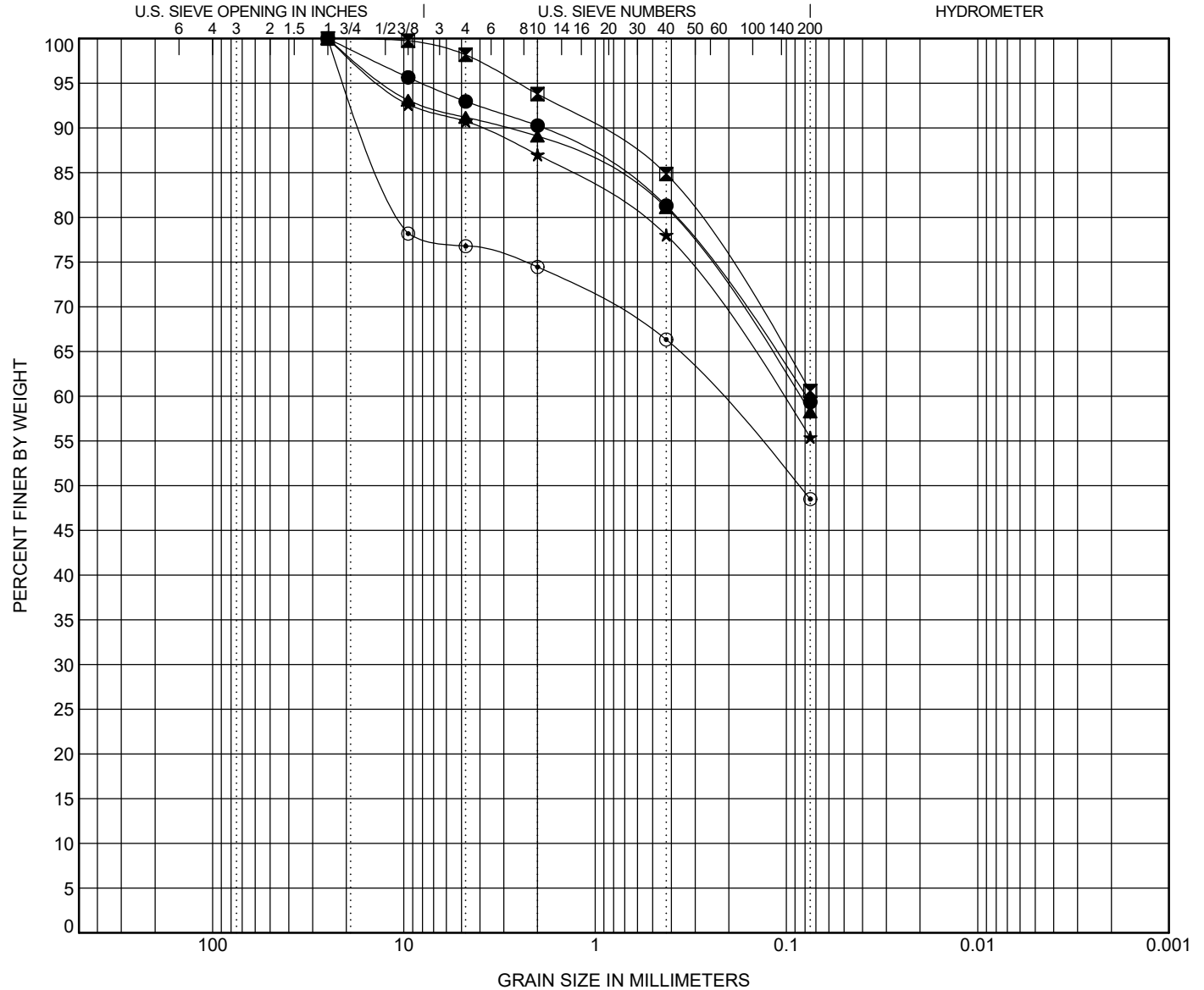
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# GRAIN SIZE DISTRIBUTION

PROJECT NUMBER IM-2-094(194)260

LOCATION Stutsman County

PCN 23577



COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

BOREHOLE	DEPTH	AASHTO Classification	USCS Classification	LL	PL	PI	Cc	Cu
● B2	14.0	A-6 (11)	CL	40	16	24		
■ B2	16.0	A-6 (10)	CL	38	18	20		
▲ B2	19.0	A-6 (9)	CL	37	16	21		
★ B2	21.0	A-6 (8)	CL	40	19	21		
○ B2	24.0	A-7-6 (7)	SC	42	19	23		

BOREHOLE	DEPTH	D100	D50	D30	D15	%Gravel	%Sand	%Silt	%Clay
● B2	14.0	25				7.0	33.6	59.4	
■ B2	16.0	25				1.8	37.6	60.6	
▲ B2	19.0	25				8.8	32.9	58.2	
★ B2	21.0	25				9.3	35.3	55.4	
○ B2	24.0	25	0.087			23.2	28.3	48.5	

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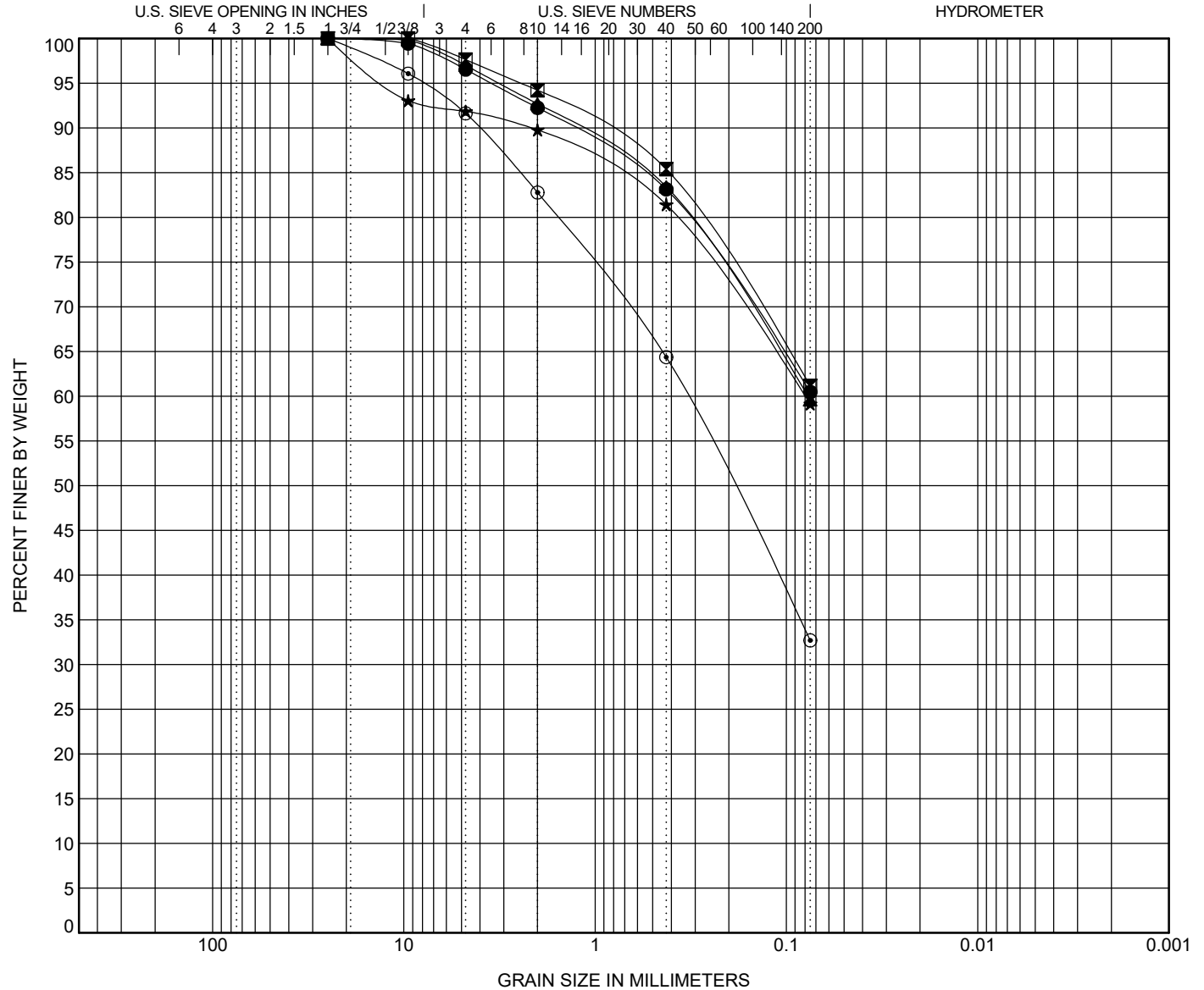


# GRAIN SIZE DISTRIBUTION

PROJECT NUMBER IM-2-094(194)260

LOCATION Stutsman County

PCN 23577



COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

BOREHOLE	DEPTH	AASHTO Classification	USCS Classification	LL	PL	PI	Cc	Cu
● B2	26.0	A-7-6 (11)	CL	41	18	23		
■ B2	29.0	A-6 (10)	CL	38	17	21		
▲ B2	31.0	A-6 (10)	CL	39	17	22		
★ B2	34.0	A-6 (10)	CL	39	16	23		
○ B2	36.0	A-2-6 (0)	SC	28	17	11		

BOREHOLE	DEPTH	D100	D50	D30	D15	%Gravel	%Sand	%Silt	%Clay
● B2	26.0	25				3.5	36.1	60.5	
■ B2	29.0	25				2.4	36.5	61.1	
▲ B2	31.0	25				3.0	37.4	59.6	
★ B2	34.0	25				8.1	32.7	59.1	
○ B2	36.0	25	0.193			8.4	58.9	32.7	

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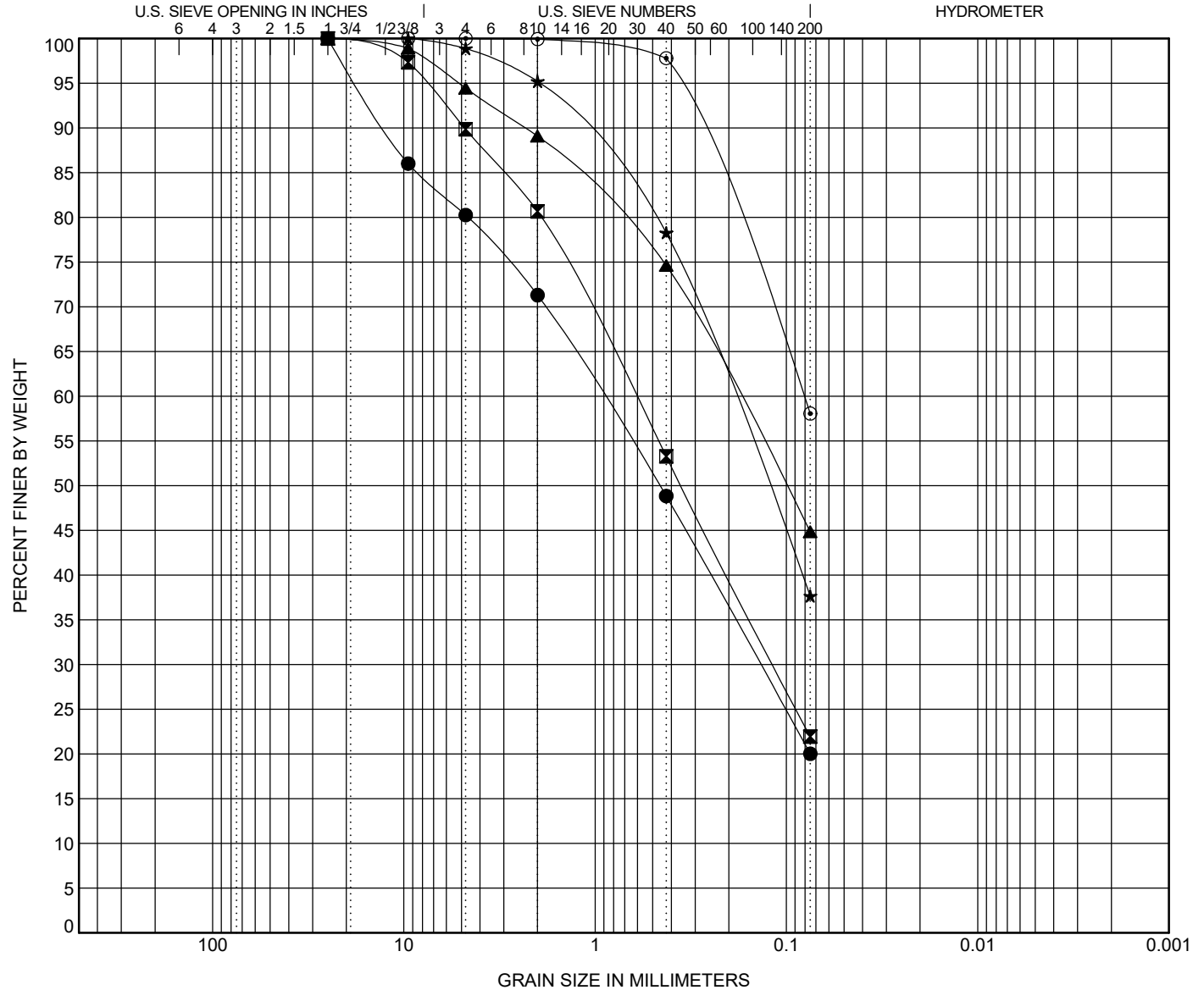


# GRAIN SIZE DISTRIBUTION

PROJECT NUMBER IM-2-094(194)260

LOCATION Stutsman County

PCN 23577



COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

BOREHOLE	DEPTH	AASHTO Classification	USCS Classification		LL	PL	PI	Cc	Cu
● B2	39.0	A-2-6 (0)	SC		31	19	12		
☒ B2	40.0	A-2-4 (0)	SC		31	21	10		
▲ B2	45.0	A-6 (3)	SC		36	21	15		
★ B2	47.0	A-4 (0)	SC		27	18	9		
⊙ B2	49.0	A-4 (2)	CL		29	21	8		

BOREHOLE	DEPTH	D100	D50	D30	D15	%Gravel	%Sand	%Silt	%Clay
● B2	39.0	25	0.46	0.137		19.7	60.2	20.0	
☒ B2	40.0	25	0.355	0.117		10.1	67.9	21.9	
▲ B2	45.0	25	0.101			5.6	49.6	44.8	
★ B2	47.0	25	0.127			1.1	61.2	37.7	
⊙ B2	49.0	25				0.0	41.9	58.1	

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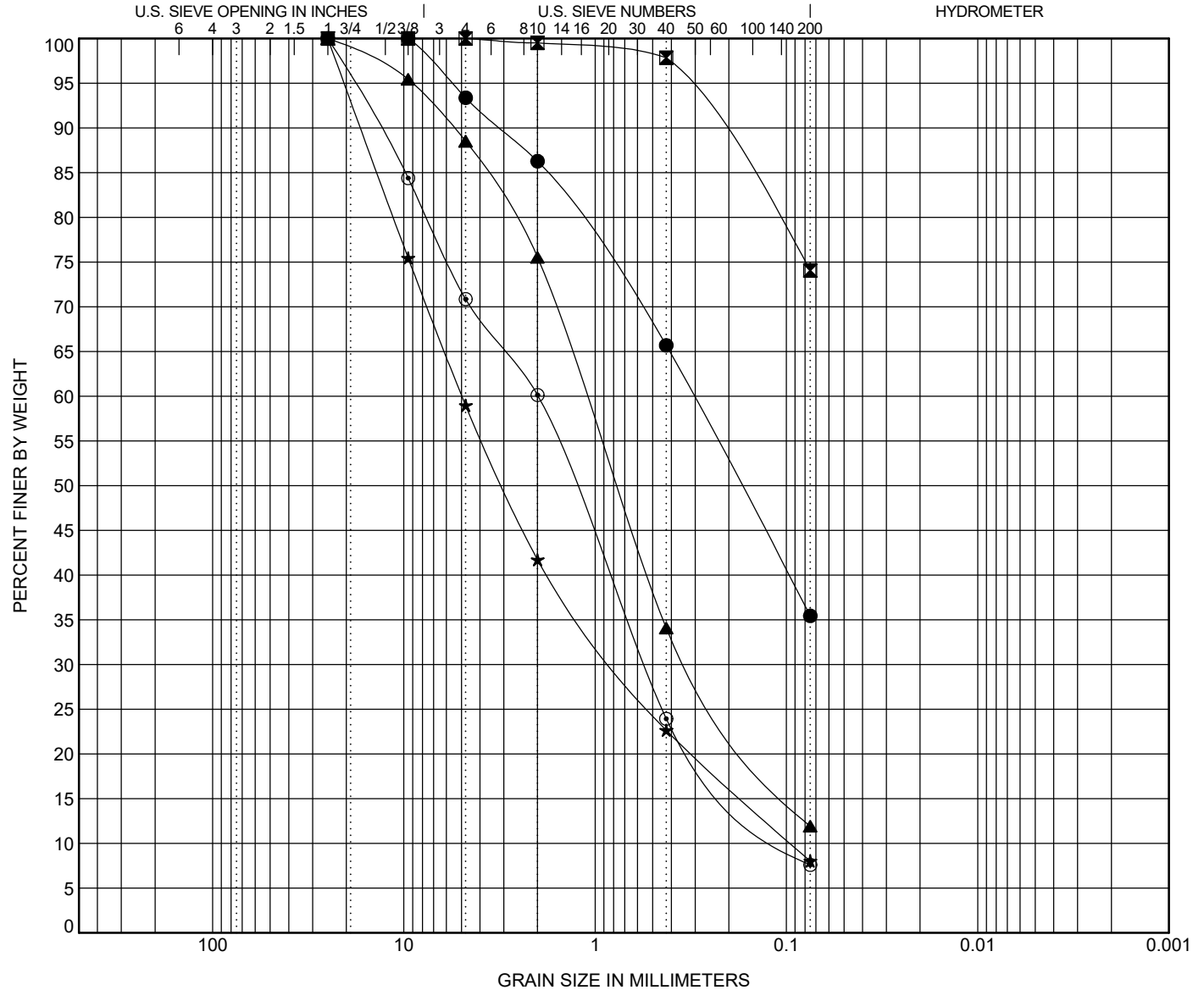


# GRAIN SIZE DISTRIBUTION

PROJECT NUMBER IM-2-094(194)260

LOCATION Stutsman County

PCN 23577



COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

BOREHOLE	DEPTH	AASHTO Classification	USCS Classification	LL	PL	PI	Cc	Cu
● B2	51.0	A-2-4 (0)	SC	31	22	9		
⊠ B2	54.0	A-6 (9)	CL	33	18	15		
▲ B2	56.0	A-1-b (0)	SW-SM	NP	NP	NP	1.32	17.34
★ B2	59.0	A-1-a (0)	SW-SM	NP	NP	NP	1.27	52.32
⊙ B2	61.0	A-1-b (0)	SW-SM	NP	NP	NP	1.58	20.55

BOREHOLE	DEPTH	D100	D50	D30	D15	%Gravel	%Sand	%Silt	%Clay
● B2	51.0	25	0.173			6.6	57.9	35.4	
⊠ B2	54.0	25				0.0	25.9	74.1	
▲ B2	56.0	25	0.77	0.309	0.095	11.5	76.6	11.9	
★ B2	59.0	25	3.03	0.772	0.172	41.0	50.9	8.0	
⊙ B2	61.0	25	1.296	0.551	0.165	29.1	63.3	7.6	

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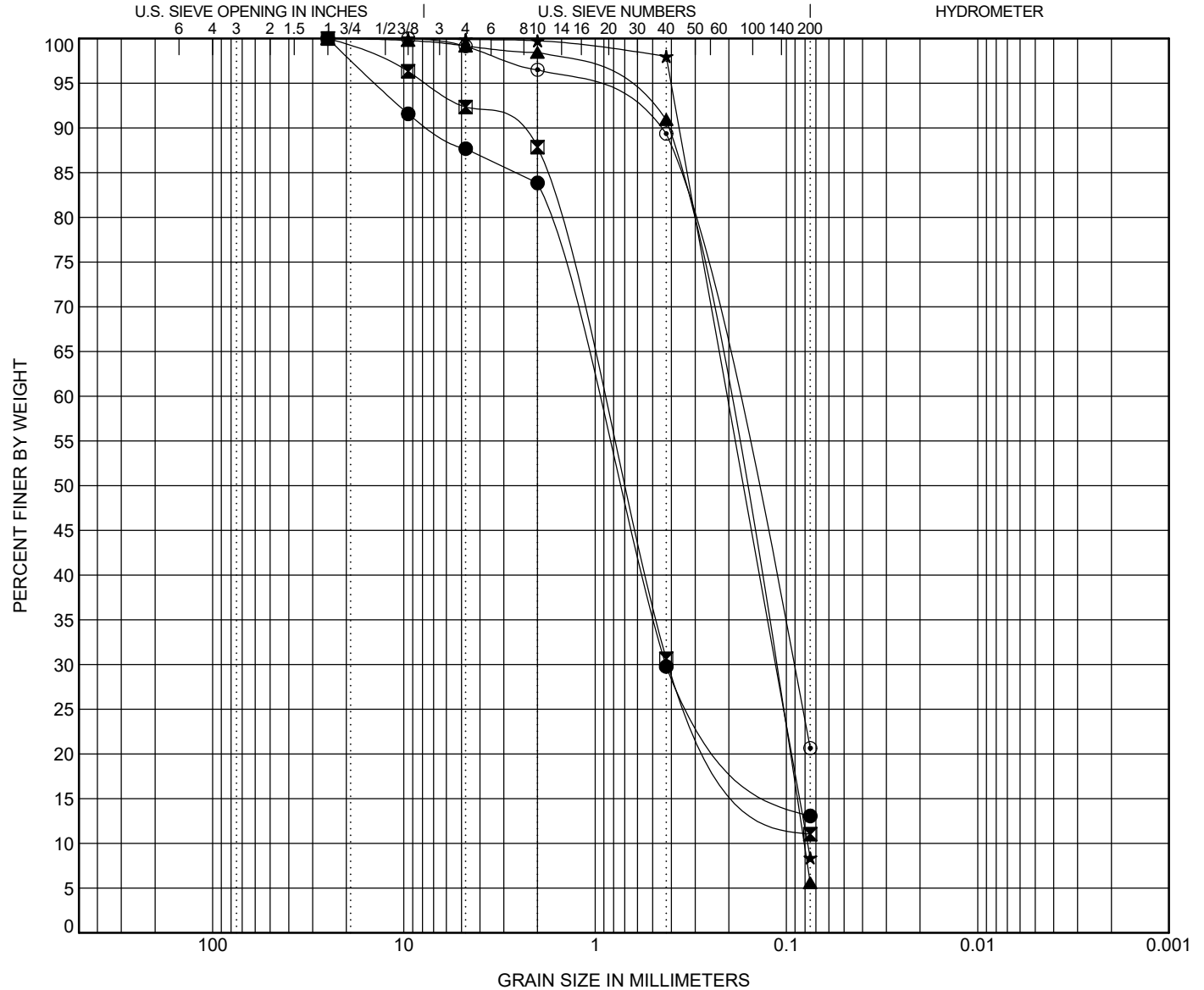
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BISMARCK, ND 58504

# GRAIN SIZE DISTRIBUTION

PROJECT NUMBER IM-2-094(194)260

LOCATION Stutsman County

PCN 23577



COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

BOREHOLE	DEPTH	AASHTO Classification	USCS Classification			LL	PL	PI	Cc	Cu
● B2	64.0	A-2-6 (0)	SC			33	22	11		
☒ B2	66.0	A-1-b (0)	SW-SM			30	25	5	2.51	13.75
▲ B2	89.0	A-3 (0)	SP-SM			NP	NP	NP	0.82	2.76
★ B2	94.0	A-3 (0)	SP-SM			NP	NP	NP	0.82	2.63
◎ B2	99.0	A-2-4 (0)	SM			NP	NP	NP		

BOREHOLE	DEPTH	D100	D50	D30	D15	%Gravel	%Sand	%Silt	%Clay
● B2	64.0	25	0.759	0.428	0.092	12.3	74.6	13.1	
☒ B2	66.0	25	0.718	0.402	0.107	7.7	81.3	11.0	
▲ B2	89.0	25	0.185	0.123	0.091	0.8	93.6	5.6	
★ B2	94.0	25	0.168	0.114	0.085	0.1	91.5	8.4	
◎ B2	99.0	25	0.157	0.095		0.9	78.5	20.7	

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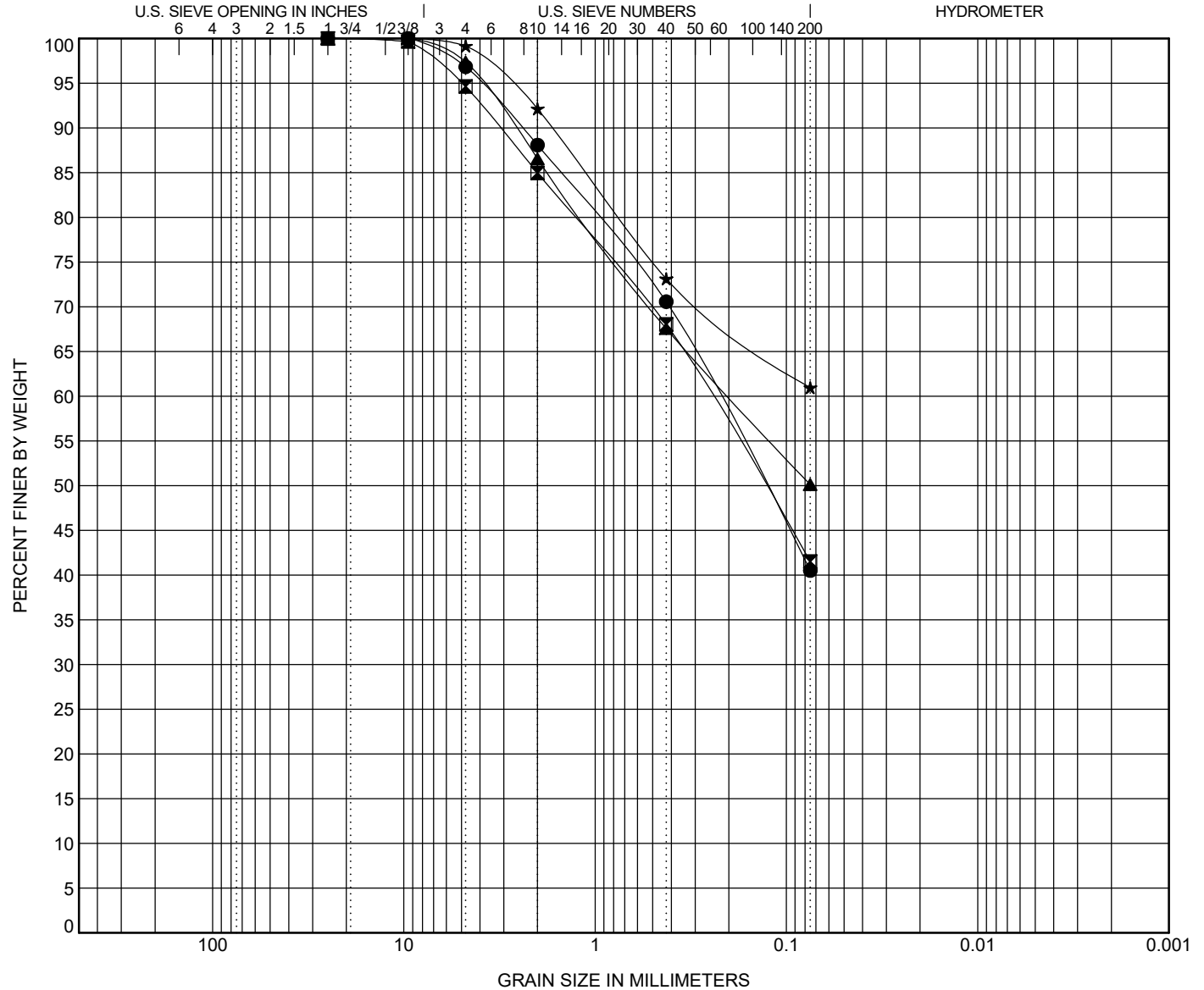
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LOCATION Stutsman County

PCN 23577



COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

BOREHOLE	DEPTH	AASHTO Classification	USCS Classification	LL	PL	PI	Cc	Cu
● B2	104.0	A-4 (1)	SC	27	17	10		
■ B2	109.0	A-6 (2)	SC	31	17	14		
▲ B2	114.0	A-7-6 (8)	CH	50	27	23		
★ B2	119.0	A-7-5 (19)	MH	66	34	32		

BOREHOLE	DEPTH	D100	D50	D30	D15	%Gravel	%Sand	%Silt	%Clay
● B2	104.0	25	0.13			3.2	56.3	40.5	
■ B2	109.0	25	0.131			5.4	53.1	41.5	
▲ B2	114.0	25				2.6	47.3	50.1	
★ B2	119.0	25				0.8	38.2	61.0	

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# SUMMARY OF LABORATORY RESULTS

PROJECT NUMBER IM-2-094(194)260

LOCATION Stutsman County

PCN 23577

Borehole	Depth	Liquid Limit	Plastic Limit	Plasticity Index	Maximum Size (mm)	%<#200 Sieve	AASHTO Classification	USCS Classification	Water Content (%)	Avg. Water Content (%)	Dry Density (pcf)	Saturation (%)	Void Ratio
B1	0.0	37	20	17	25	14	A-2-6 (0)	SC					
B1	2.0	37	18	19	25	57	A-6 (8)	CL					
B1	4.0	42	19	23	25	59	A-7-6 (11)	CL					
B1	6.0	42	18	24	25	59	A-7-6 (11)	CL					
B1	9.0	40	18	22	25	56	A-6 (9)	CL					
B1	11.0	40	17	23	25	57	A-6 (10)	CL					
B1	14.0	42	20	22	25	57	A-7-6 (10)	CL					
B1	16.0	40	19	21	25	58	A-6 (9)	CL					
B1	19.0	42	20	22	25	61	A-7-6 (11)	CL					
B1	21.0	42	18	24	25	58	A-7-6 (11)	CL					
B1	24.0	38	18	20	25	61	A-6 (10)	CL					
B1	26.0	37	17	20	25	59	A-6 (9)	CL					
B1	29.0	42	19	23	25	57	A-7-6 (10)	CL					
B1	31.0	38	17	21	25	61	A-6 (10)	CL					
B1	34.0	41	19	22	25	61	A-7-6 (11)	CL					
B1	36.0	38	21	17	25	63	A-6 (9)	CL					
B1	39.0	34	20	14	25	61	A-6 (6)	CL					
B1	41.0	37	19	18	25	69	A-6 (11)	CL					
B1	44.0	NP	NP	NP	25	6	A-1-b (0)	SP-SM					
B1	49.0	NP	NP	NP	25	11	A-1-b (0)	SP-SM					
B1	51.0	30	21	9	25	5	A-2-4 (0)	SP					
B1	54.0	NP	NP	NP	25	7	A-3 (0)	SP-SM					
B1	56.0	NP	NP	NP	25	10	A-1-b (0)	SP-SM					
B1	59.0	NP	NP	NP	25	10	A-1-a (0)	SP-SM					
B1	64.0	32	24	8	25	9	A-2-4 (0)	SW-SM					
B1	69.0	30	24	6	25	13	A-1-b (0)	SM					
B1	71.0	NP	NP	NP	25	16	A-1-b (0)	SM					
B1	74.0	NP	NP	NP	25	11	A-2-4 (0)	SP-SM					
B1	76.0	NP	NP	NP	25	13	A-2-4 (0)	SM					
B1	79.0	NP	NP	NP	25	17	A-2-4 (0)	SM					
B1	81.0	NP	NP	NP	25	12	A-2-4 (0)	SM					
B1	89.0	NP	NP	NP	25	12	A-2-4 (0)	SM					
B1	94.0	NP	NP	NP	25	21	A-2-4 (0)	SM					
B1	99.0	34	18	16	25	49	A-6 (4)	SC					
B1	104.0	25	21	4	25	51	A-4 (0)	CL-ML					
B1	109.0	37	22	15	25	48	A-6 (4)	SC					
B1	114.0	60	27	33	25	57	A-7-6 (16)	CH					
B1	119.0	69	26	43	25	62	A-7-6 (25)	CH					

LAB SUMMARY - 20171219.GDT - 7/13/23 08:34 - R:\PROJECT\20094260\_194\MATERIAL\GEOTECH\DEEP FOUNDATION\VOID\BORING 1.GPJ



# SUMMARY OF LABORATORY RESULTS

PROJECT NUMBER IM-2-094(194)260

LOCATION Stutsman County

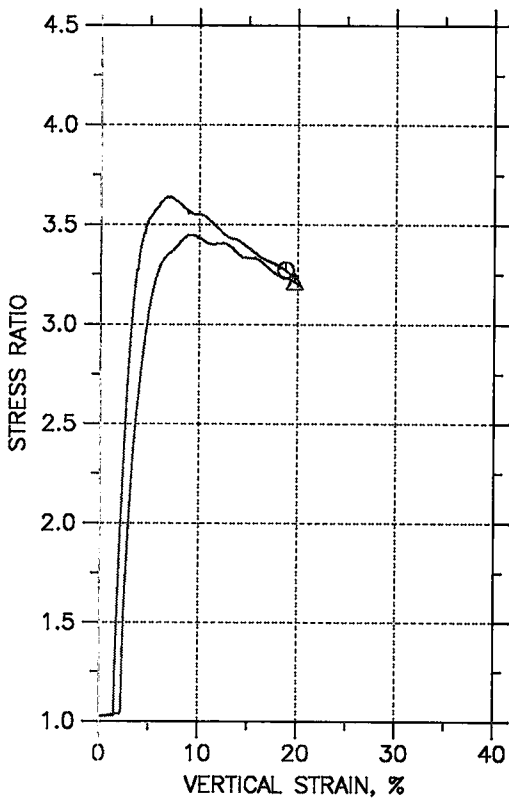
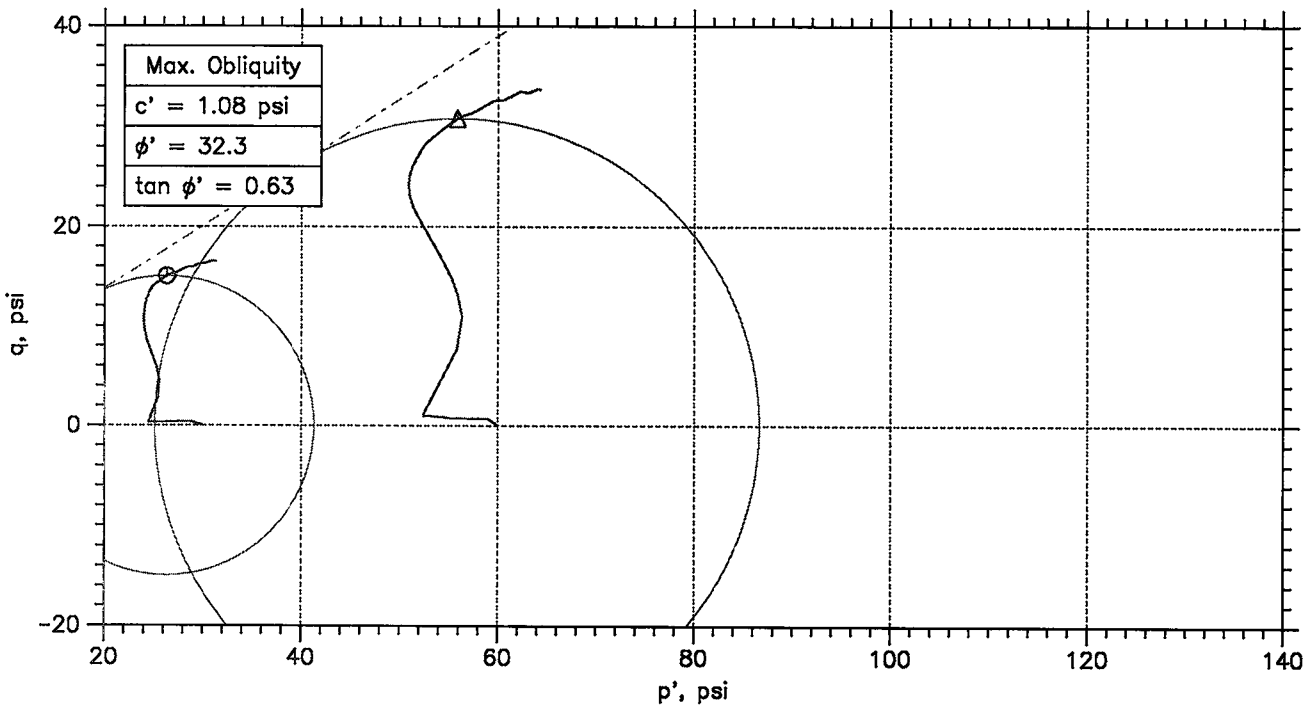
PCN 23577

Borehole	Depth	Liquid Limit	Plastic Limit	Plasticity Index	Maximum Size (mm)	%<#200 Sieve	AASHTO Classification	USCS Classification	Water Content (%)	Avg. Water Content (%)	Dry Density (pcf)	Saturation (%)	Void Ratio
B2	0.0	37	19	18	25	49	A-6 (5)	SC					
B2	4.0	38	17	21	25	62	A-6 (10)	CL					
B2	6.0	41	20	21	25	60	A-7-6 (10)	CL					
B2	9.0	40	17	23	25	59	A-6 (11)	CL					
B2	11.0	38	18	20	25	56	A-6 (8)	CL					
B2	14.0	40	16	24	25	59	A-6 (11)	CL					
B2	16.0	38	18	20	25	61	A-6 (10)	CL					
B2	19.0	37	16	21	25	58	A-6 (9)	CL					
B2	21.0	40	19	21	25	55	A-6 (8)	CL					
B2	24.0	42	19	23	25	48	A-7-6 (7)	SC					
B2	26.0	41	18	23	25	60	A-7-6 (11)	CL					
B2	29.0	38	17	21	25	61	A-6 (10)	CL					
B2	31.0	39	17	22	25	60	A-6 (10)	CL					
B2	34.0	39	16	23	25	59	A-6 (10)	CL					
B2	36.0	28	17	11	25	33	A-2-6 (0)	SC					
B2	39.0	31	19	12	25	20	A-2-6 (0)	SC					
B2	40.0	31	21	10	25	22	A-2-4 (0)	SC					
B2	45.0	36	21	15	25	45	A-6 (3)	SC					
B2	47.0	27	18	9	25	38	A-4 (0)	SC					
B2	49.0	29	21	8	25	58	A-4 (2)	CL					
B2	51.0	31	22	9	25	35	A-2-4 (0)	SC					
B2	54.0	33	18	15	25	74	A-6 (9)	CL					
B2	56.0	NP	NP	NP	25	12	A-1-b (0)	SW-SM					
B2	59.0	NP	NP	NP	25	8	A-1-a (0)	SW-SM					
B2	61.0	NP	NP	NP	25	8	A-1-b (0)	SW-SM					
B2	64.0	33	22	11	25	13	A-2-6 (0)	SC					
B2	66.0	30	25	5	25	11	A-1-b (0)	SW-SM					
B2	89.0	NP	NP	NP	25	6	A-3 (0)	SP-SM					
B2	94.0	NP	NP	NP	25	8	A-3 (0)	SP-SM					
B2	99.0	NP	NP	NP	25	21	A-2-4 (0)	SM					
B2	104.0	27	17	10	25	41	A-4 (1)	SC					
B2	109.0	31	17	14	25	42	A-6 (2)	SC					
B2	114.0	50	27	23	25	50	A-7-6 (8)	CH					
B2	119.0	66	34	32	25	61	A-7-5 (19)	MH					

LAB SUMMARY - 20171219.GDT - 7/13/23 08:44 - R:\PROJECT\20094260\_194\MATERIAL\GEOTECH\DEEP FOUNDATION\VOID\BORING 2.GPJ



### CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767



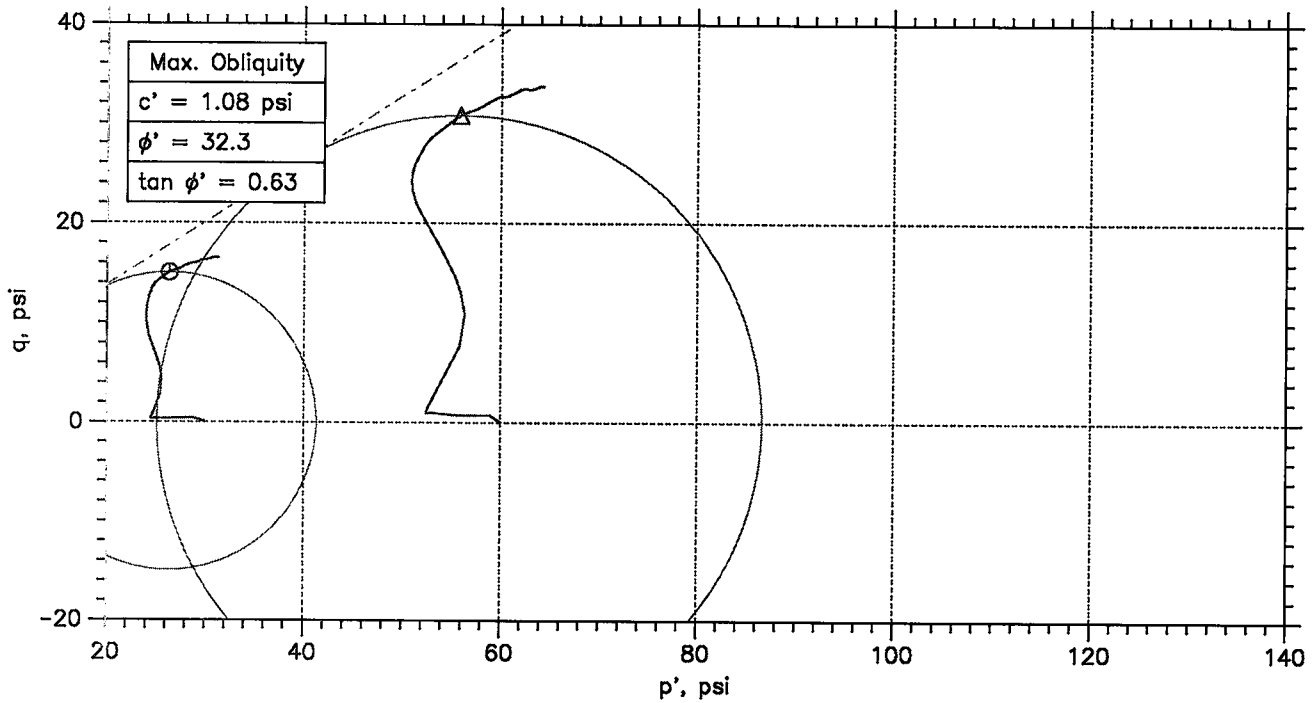
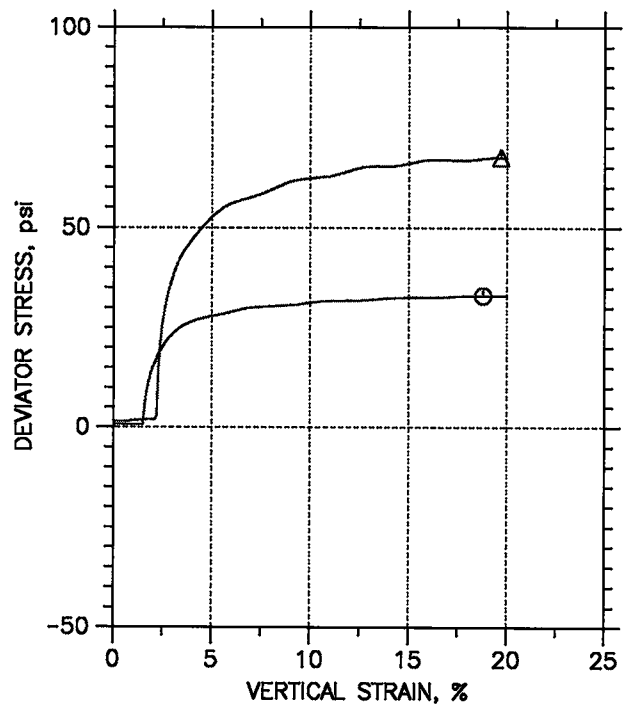
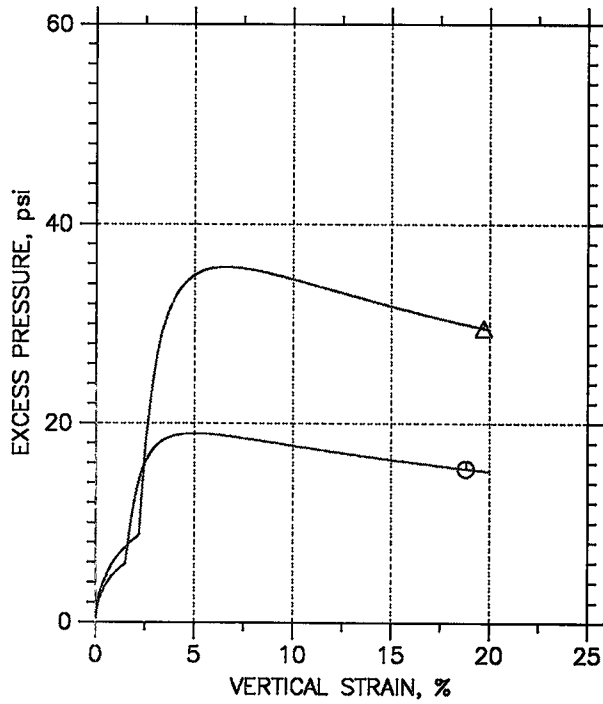
Symbol	⊙	△	
Sample No.	SS-391-16	SS-391-16	
Test No.	CU-8-16	CU-9-16	
Depth	34.3-34.8	34.8-35.3	
Initial	Diameter, in	2.85	2.863
	Height, in	5.719	5.738
	Water Content, %	19.2	18.5
	Dry Density, pcf	107.7	110.4
	Saturation, %	94.9	98.2
Before Shear	Void Ratio	0.535	0.498
	Water Content, %	20.0	18.9
	Dry Density, pcf	108.2	110.3
	Saturation*, %	100.0	100.0
	Void Ratio	0.529	0.5
	Back Press., psi	60.95	51.01
	Ver. Eff. Cons. Stress, psi	30.02	59.98
	Shear Strength, psi	16.52	33.73
	Strain at Failure, %	18.8	19.7
	Strain Rate, %/min	0.075	0.075
	B-Value	0.95	0.95
	Estimated Specific Gravity	2.65	2.65
	Liquid Limit	---	---
	Plastic Limit	---	---

	Project: IM-2-094(143)260	<div style="display: flex; justify-content: space-around;"> <div style="border: 1px solid black; width: 40px; height: 40px;"></div> <div style="border: 1px solid black; width: 40px; height: 40px;"></div> <div style="border: 1px solid black; width: 40px; height: 40px;"></div> <div style="border: 1px solid black; width: 40px; height: 40px;"></div> </div>
	Location:	
	Project No.:	
	Boring No.: 2	
	Sample Type:	
	Description: T. L. 22" Brn Sandy Clay with I.O. & coal deposits, rock upto 3/4"	
Remarks:		

Phase calculations based on start and end of test.

\* Saturation is set to 100% for phase calculations.

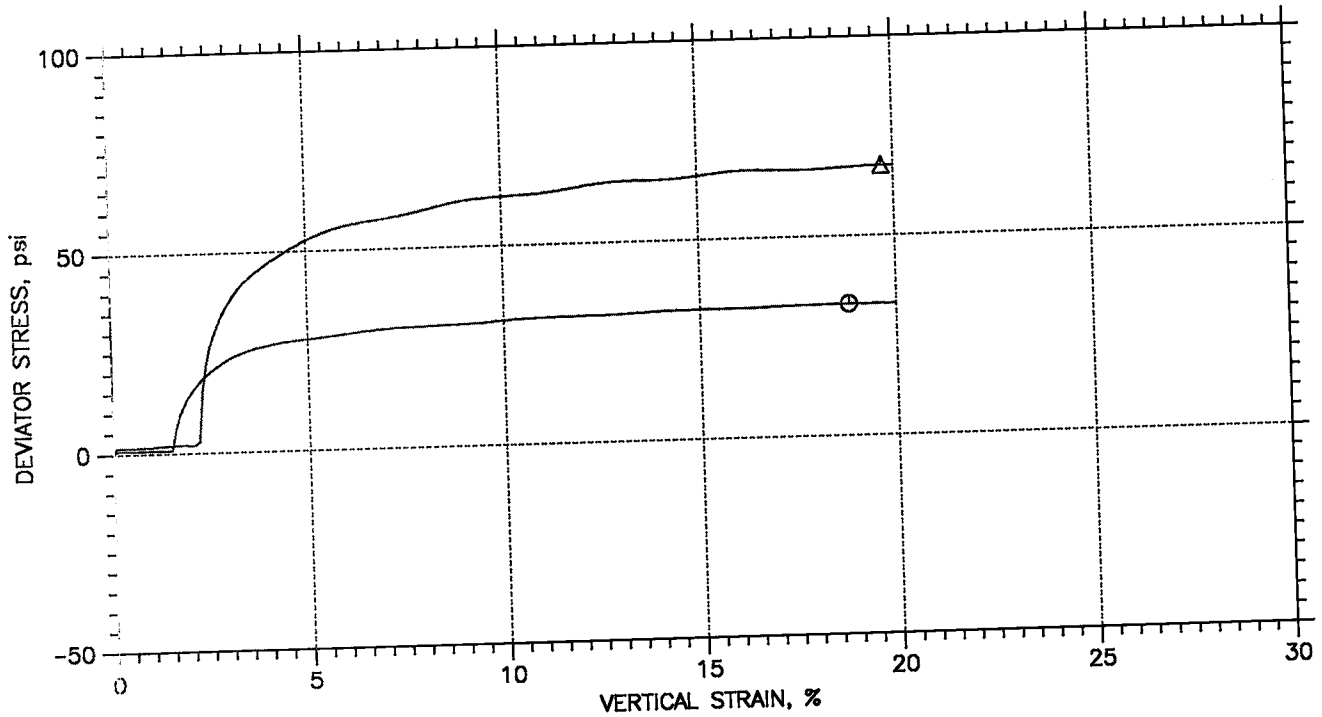
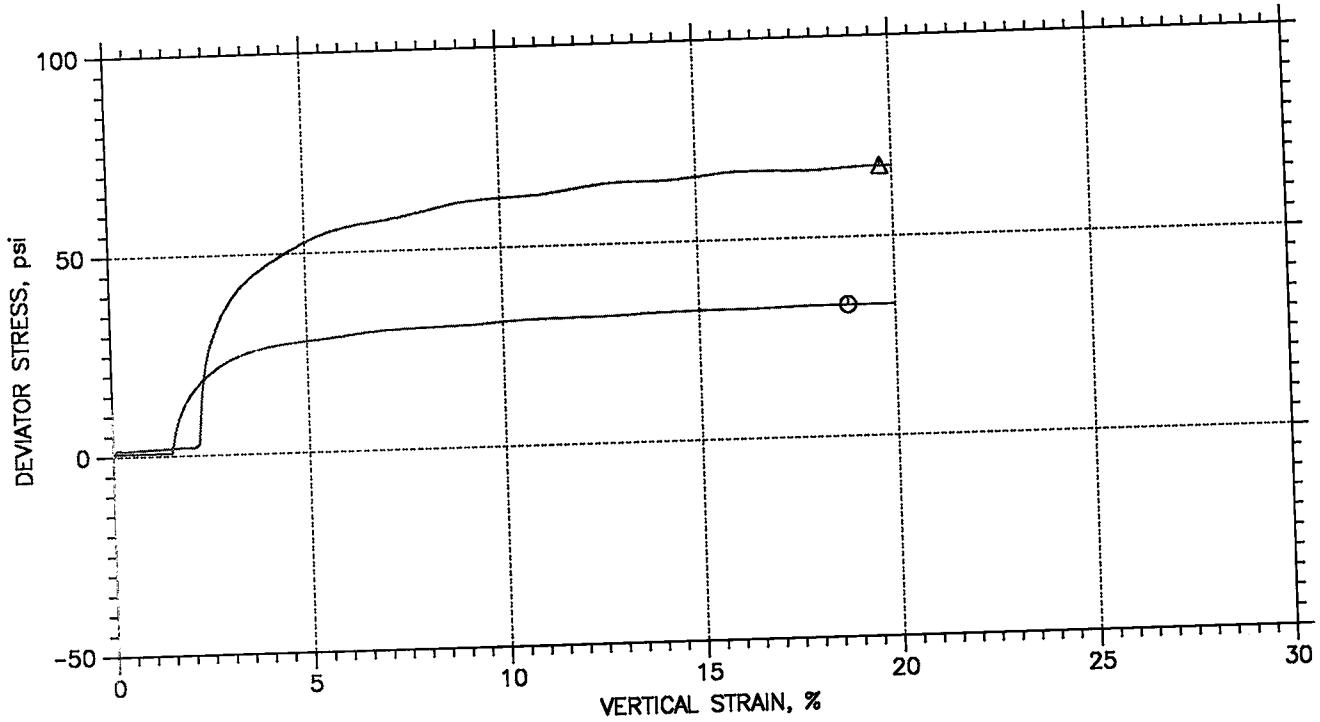
# CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767



Sample No.	Test No.	Depth	Tested By	Test Date	Checked By	Check Date	Test File
⊙	SS-391-16	CU-8-16	34.3-34.8	Dan	5/25/16	Matt	CU-8-16.dat
△	SS-391-16	CU-9-16	34.8-35.3	Dan	6/1/16	Matt	CU-9-16.dat

	Project: IM-2-094(143)260		Location:		Project No.:		
	Boring No.: 2		Sample Type:				
	Description: T. L. 22" Brn Sandy Clay with I.O. & coal deposits, rock upto 3/4"						
	Remarks:						

# CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767



Symbol	Sample No.	Test No.	Depth	Tested By	Test Date	Checked By	Check Date	Test File
⊙	SS-391-16	CU-8-16	34.3-34.8	Dan	5/25/16	Matt		CU-8-16.dat
Δ	SS-391-16	CU-9-16	34.8-35.3	Dan	6/1/16	Matt		CU-9-16.dat

	Project: IM-2-094(143)260		Location:		Project No.:	
	Boring No.: 2		Sample Type:			
	Description: T. L. 22" Brn Sandy Clay with I.O. & coal deposits, rock upto 3/4"					
	Remarks:					

**TRIAXIAL UU(Q) CCU(R) CD(S)**

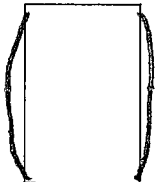
North Dakota Department of Transportation, Materials & Research  
SFN 50459 (5-2016)

56 | 29.66  
Wet Wt | 172.62  
Dry Wt | 148.93

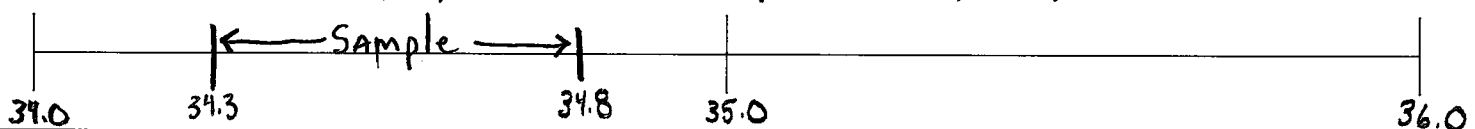
Project Number  
IM-2-094(143)260  
Boring Number  
2

After Test Wt. 1230.79

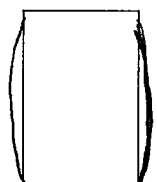
Field Sample Number 55-391-16		Lab Number CU-8-16		Depth 34.3 to 34.8	
Weight of Sample 1229.69		Confining Pressure 30		Test Number 1 of 3	
Diameter	2.856	2.846	Height	5.718	
	2.850	2.840		5.718	
	2.851	2.856		5.720	
	Average 2.850			Average 5.719	
				Moisture Can Number 535	
				Wet Wt + Can 65.46	
				Dry Wt + Can 57.69	
				Wt of Can 17.16	



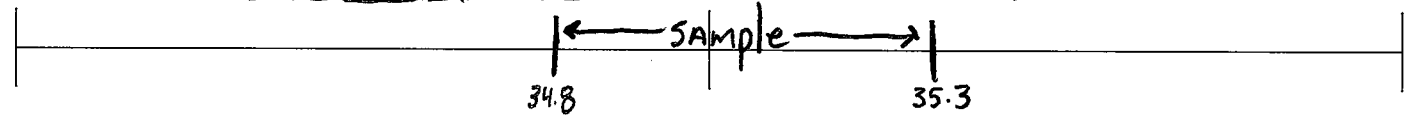
Total Length: 22" Brn Sandy Cly with I.O+ Coal Deposits, Rock up to 3/4"



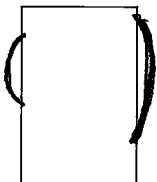
Field Sample Number 55-391-16		Lab Number CU-9-16		Depth 34.8 to 35.3	
Weight of Sample 1268.35		Confining Pressure 60		Test Number 2 of 3	
Diameter	2.856	2.878	Height	5.739	
	2.853	2.868		5.736	
	2.857	2.866		5.739	
	Average 2.863			Average 5.738	
				Moisture Can Number 536	
				Wet Wt + Can 62.35	
				Dry Wt + Can 55.34	
				Wt of Can 17.40	



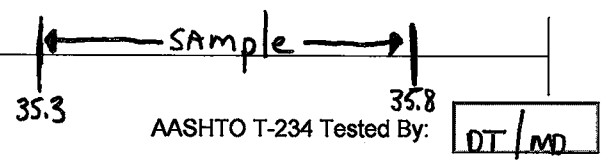
Total Length: After Test Wt. 1259.36 CAN 522 Tare Wt. 29.59 Wet Wt. 180.34 Dry Wt. 156.40



Field Sample Number 55-391-16		Lab Number CU-10-16		Depth 35.3 to 35.8	
Weight of Sample 1263.04		Confining Pressure 90		Test Number 3 of 3	
Diameter	2.847	2.866	Height	5.736	
	2.851	2.867		5.731	
	2.857	2.866		5.734	
	Average 2.859			Average 5.734	
				Moisture Can Number 566	
				Wet Wt + Can 59.80	
				Dry Wt + Can 52.74	
				Wt of Can 15.38	



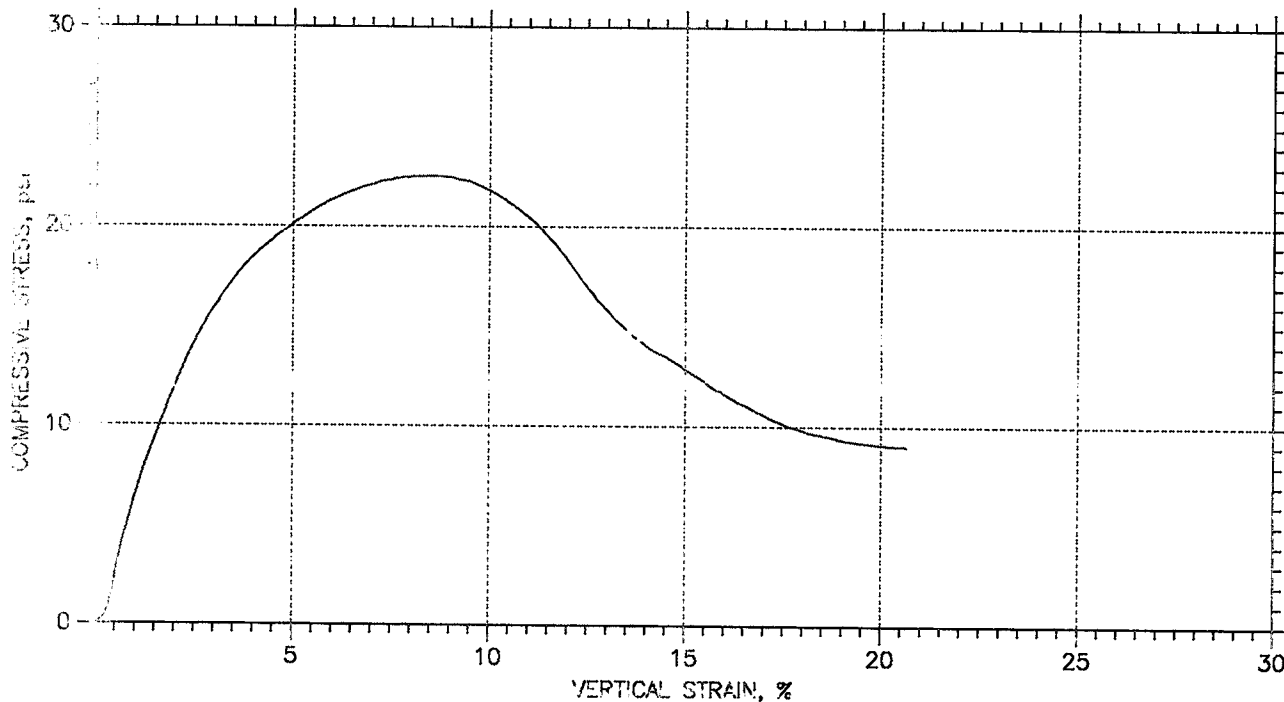
Total Length:



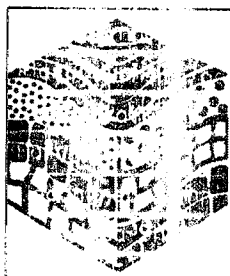
AASHTO T-234 Tested By:

DT/MD

### UNCONFINED COMPRESSION TEST REPORT



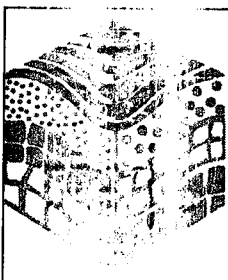
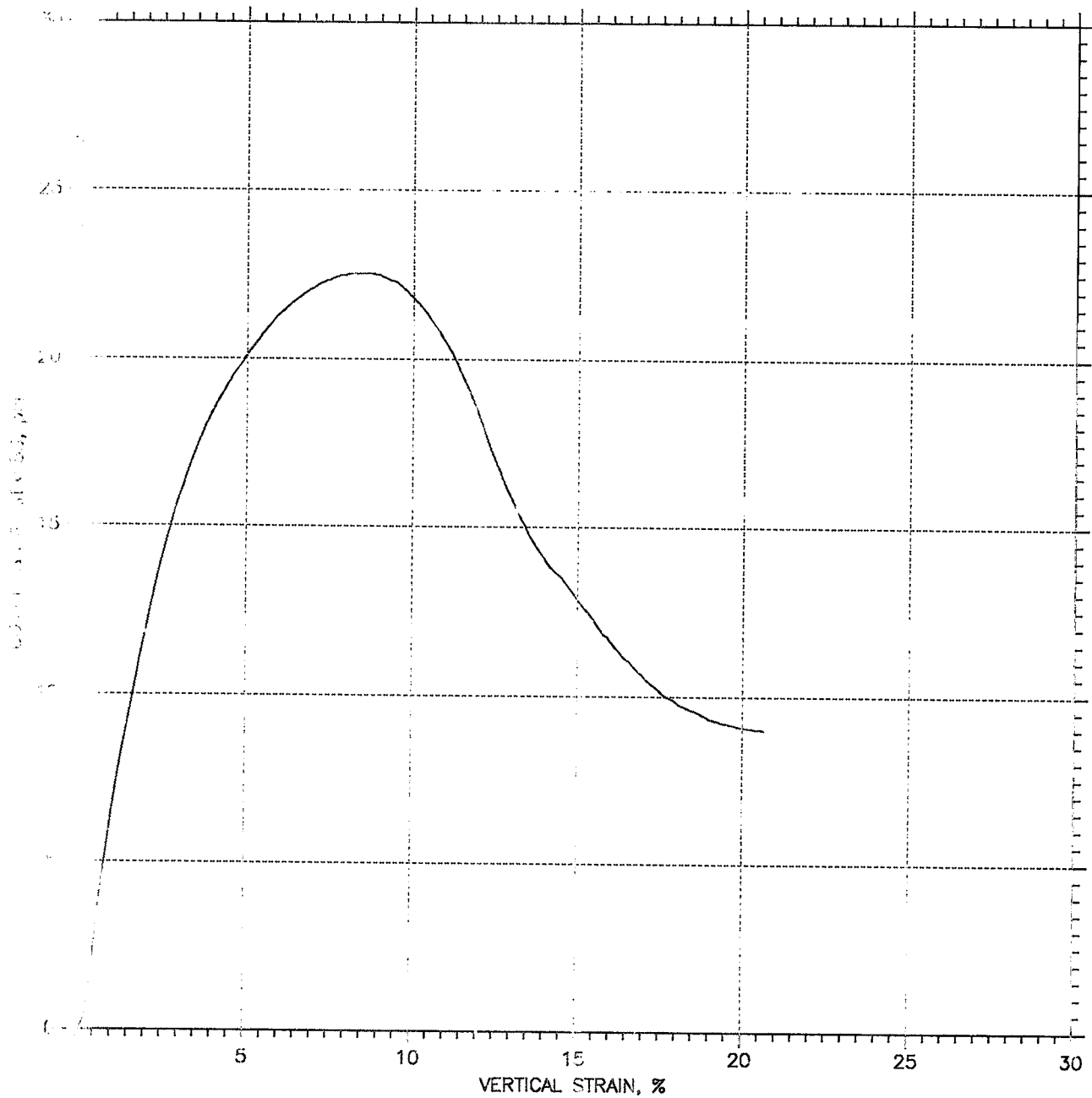
Symbol				
Test No.		UC-24-16		
Initial	Diameter, in	2.864		
	Height, in	5.732		
	Water Content, %	17.11		
	Dry Density, pcf	112.1		
	Saturation, %	95.38		
	Void Ratio	0.475		
Unconfined Compressive Strength, psi		22.52		
Undrained Shear Strength, psi		11.26		
Time to Failure, min		8.3358		
Strain Rate, %/min		1		
Measured Specific Gravity		2.65		
Liquid Limit		0		
Plastic Limit		0		
Plasticity Index		0		
Failure Sketch				



Project: IM-2-094(143)260
Location:
Project No.:
Boring No.: 2
Sample Type: Undisturbed
Description: T. L. 16" Brn Clay with 1/2" rock,
Remarks:

SS-389-16

# UNCONFINED COMPRESSION TEST REPORT



Project: IM-2-094(143)260	Location:	Project No.:
Boring No.: 2	Tested By: DT	Checked By: MD
Sample No.: SS-389-16	Test Date: 6/7/2016	Depth: 29.5-30.0
Test No.: UC-24-16	Sample Type: Undisturbed	Elevation:
Description: T. L. 16" Brn Clay with 1/2" rock,		
Remarks:		

## UNCONFINED COMPRESSION TEST

SS-389-16

Project: W-004 (0)260  
 Boring No.:  
 Sample No.:  
 Test No.: UC-24-1

Location:  
 Tested By: DT  
 Test Date: 6/7/2016  
 Sample Type: Undisturbed

Project No.:  
 Checked By: MD  
 Depth: 29.5-30.0  
 Elevation:

Soil Description: L. 16" Brn Clay with 1/2" rock,  
 Remarks:

Specimen Height: 5.73 in  
 Specimen Area: 6.41 in<sup>2</sup>  
 Specimen Volume: 36.12 cc

Liquid Limit: 0  
 Plastic Limit: 0  
 Measured Specific Gravity: 2.65

Cap Mass: 0 gm

Water Content Information  
 -----

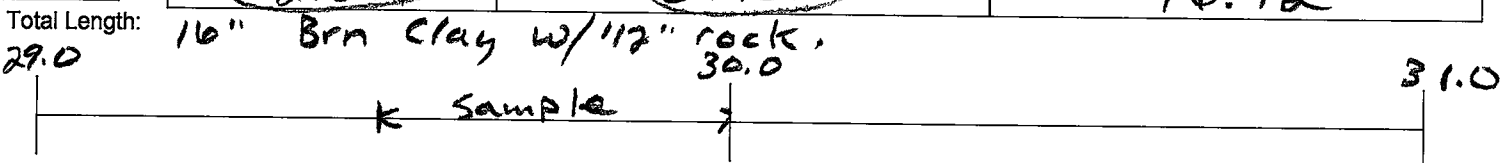
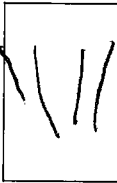
Container	s14
Wt. Container, gm	16.92
Wt. Container + wet soil, gm	65.24
Wt. Container + dry soil, gm	58.18
Wt. Dry Soil, gm	41.26
Water Content, %	17.11
Void Ratio	0.48
Degree of Saturation, %	95.38
Wet Unit Weight, pcf	131.31
Dry Unit Weight, pcf	112.13

# UNCONFINED COMPRESSION

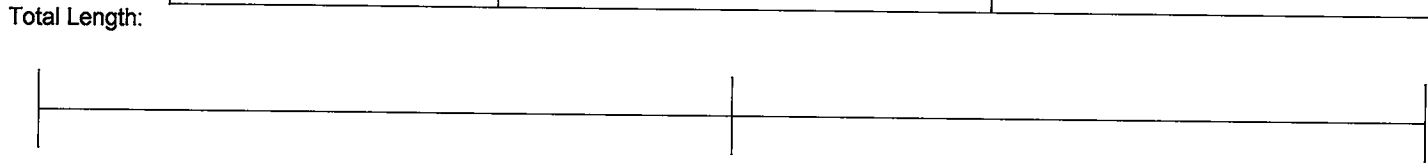
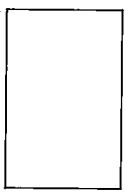
North Dakota Department of Transportation, Materials & Research  
SFN 50460 (5-2016)

Project Number  
**IM-2-094(143)260**  
Boring Number  
**2**

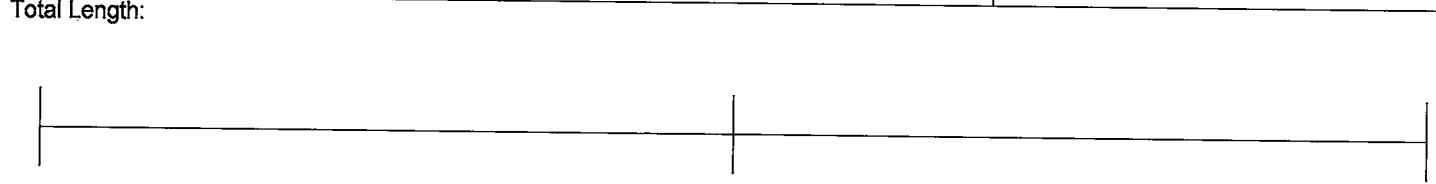
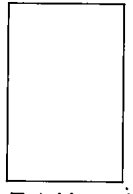
Field Sample Number <b>55-389-16</b>		Lab Number <b>UC-24-16</b>		Depth <b>29.5-30.0</b>	
Weight of Sample <b>1272.85</b>		Test Number <b>1</b> of <b>1</b>		AASHTO T-208 Tested by: <b>DT</b>	
Diameter	1. <b>2.867</b>	4. <b>2.857</b>	Height	1. <b>5.729</b>	Moisture Can Number <b>514</b>
	2. <b>2.869</b>	5. <b>2.862</b>		2. <b>5.737</b>	Wet Wt + Can <b>65.24</b>
	3. <b>2.866</b>	6. <b>2.866</b>		3. <b>5.730</b>	Dry Wt + Can <b>58.18</b>
	Average <b>2.864</b>			Average <b>5.732</b>	



Field Sample Number		Lab Number		Depth	
Weight of Sample		Test Number		of	
Diameter	1.	4.	Height	1.	Moisture Can Number
	2.	5.		2.	Wet Wt + Can
	3.	6.		3.	Dry Wt + Can
	Average			Average	



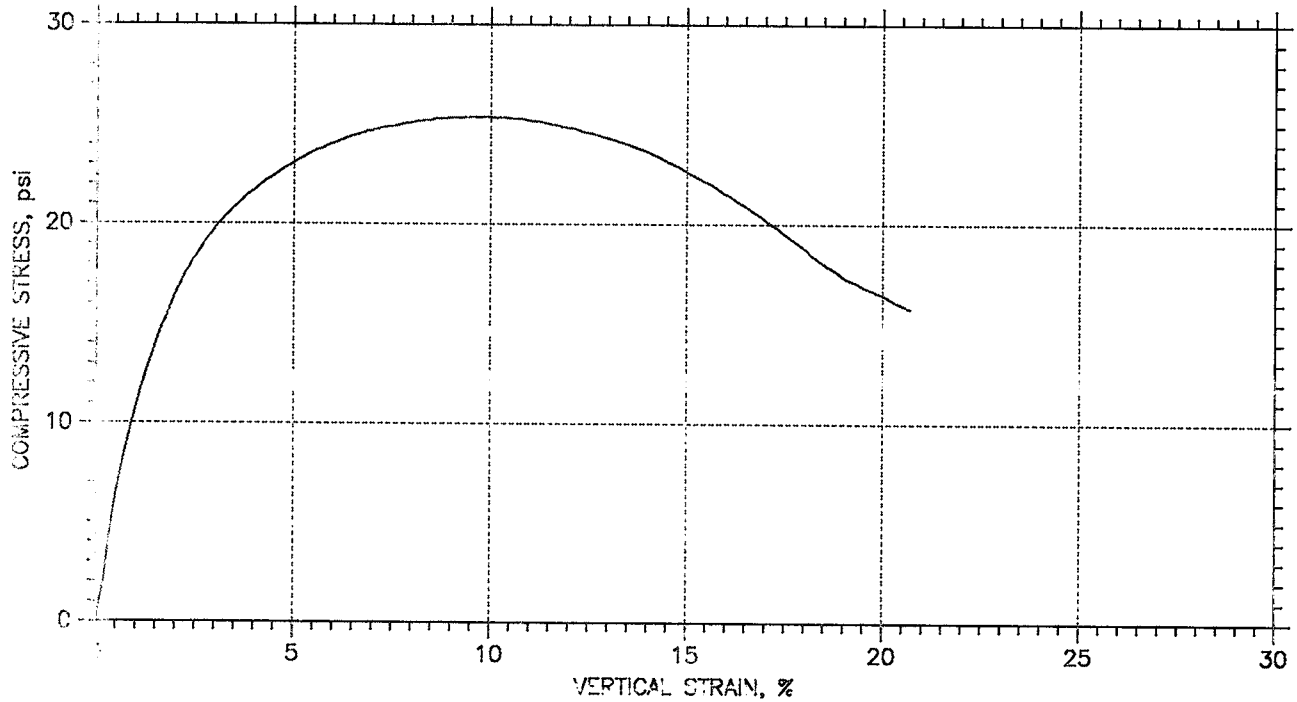
Field Sample Number		Lab Number		Depth	
Weight of Sample		Test Number		of	
Diameter	1.	4.	Height	1.	Moisture Can Number
	2.	5.		2.	Wet Wt + Can
	3.	6.		3.	Dry Wt + Can
	Average			Average	





SS-387-16

### UNCONFINED COMPRESSION TEST REPORT

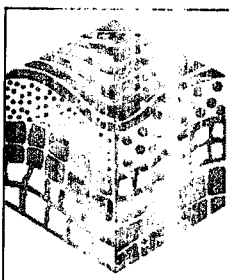
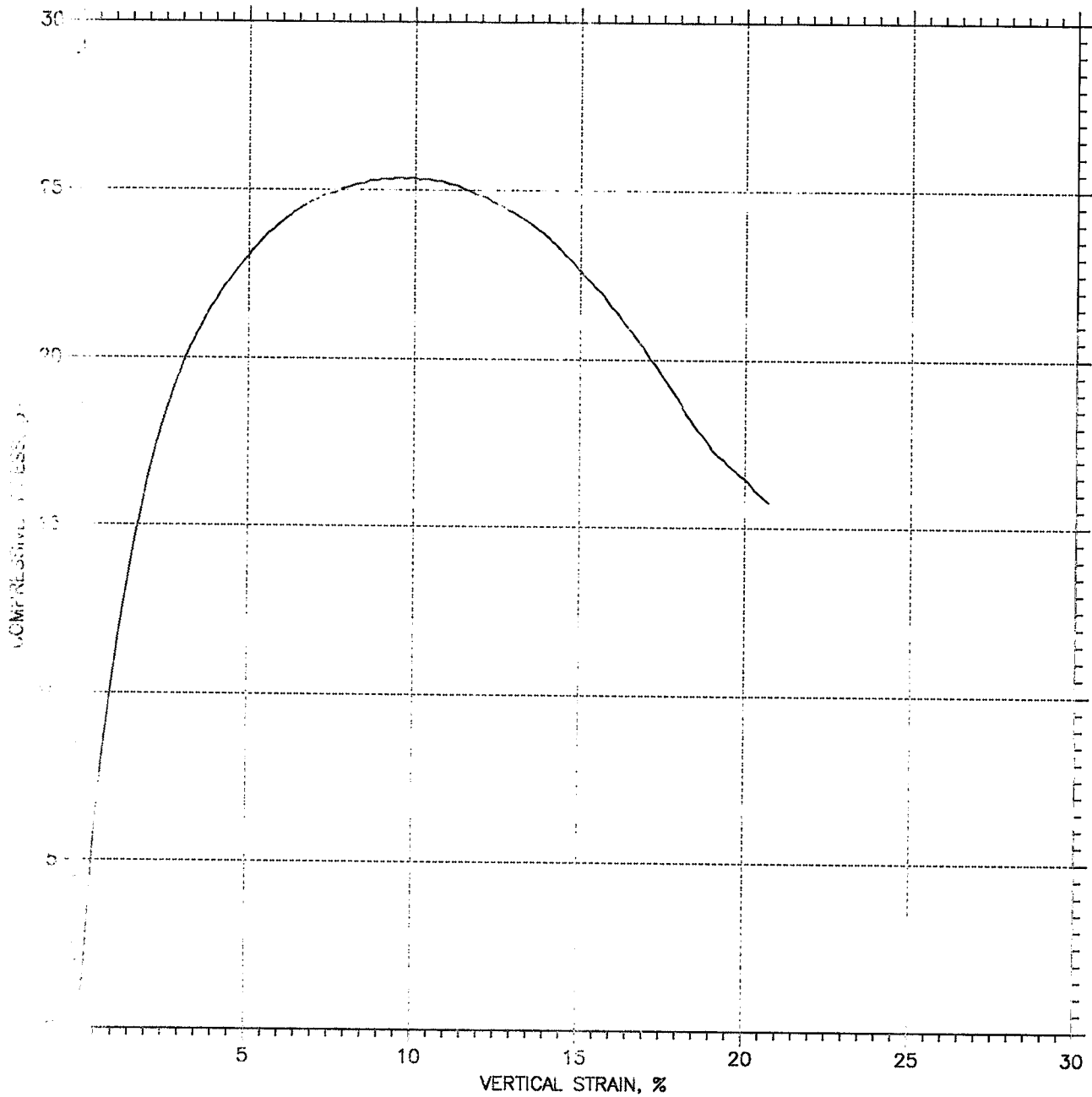


Symbol				
Test No.		UC-23-16		
Initial	Diameter, in	2.849		
	Height, in	5.742		
	Water Content, %	19.66		
	Dry Density, pcf	108.		
	Saturation %	97.82		
	Void Ratio	0.532		
Unconfined Compressive Strength, psi		25.37		
Undrained Shear Strength, psi		12.68		
Time to Failure, min		9.6706		
Strain Rate, %/min		1		
Measured Specific Gravity		2.65		
Liquid Limit		0		
Plastic Limit		0		
Plasticity Index		0		
Failure Sketch				

	Project: IM-2-094(143)260
	Location:
	Project No.:
	Boring No.: 2
	Sample Type: Undisturbed
	Description: T. L. 15 1/2" Brn Clay with I.O. & Coa Deposits, rock up to 2"
	Remarks:

SS-387-16

# UNCONFINED COMPRESSION TEST REPORT



Project: IM-2-094(143)260	Location:	Project No.:
Boring No.: 2	Tested By: DT	Checked By: MD
Sample No.: SS-387-16	Test Date: 6/6/2016	Depth: 24.5-25.0
Test No.: UC-23-16	Sample Type: Undisturbed	Elevation:
Description: T. L. 15 1/2" Brn Cly with I.O. & Coal Deposits, rock up to 2"		
Remarks:		

UNCONFINED COMPRESSION TEST

SS-387-16

Project: IM-2-094(03)260  
 Boring No.: 2  
 Sample No.: SS-387-16  
 Test No.: UC-23-16

Location:  
 Tested By: DT  
 Test Date: 6/6/2016  
 Sample Type: Undisturbed

Project No.:  
 Checked By: MD  
 Depth: 24.5-25.0  
 Elevation:

Soil Description: 1. L. 15 1/2" Brn Cly with I.O. & Coal Deposits, rock up to 2"  
 Remarks:

Specimen Height: 5.74 in  
 Specimen Area: 6.37 in<sup>2</sup>  
 Specimen Volume: 59.85 cc

Liquid Limit: 0  
 Plastic Limit: 0  
 Measured Specific Gravity: 2.65

Cap Mass: 0 gm

Water Content Information

Container ID	S 48
Wt. Container, gm	17.01
Wt. Container + Wet Soil, gm	76.24
Wt. Container + Dry Soil, gm	66.52
Wt. Dry Soil, gm	49.5
Water Content, %	19.66
Void Ratio	0.53
Degree of Saturation, %	97.82
Wet Unit Weight, pcf	129.17
Dry Unit Weight, pcf	107.95

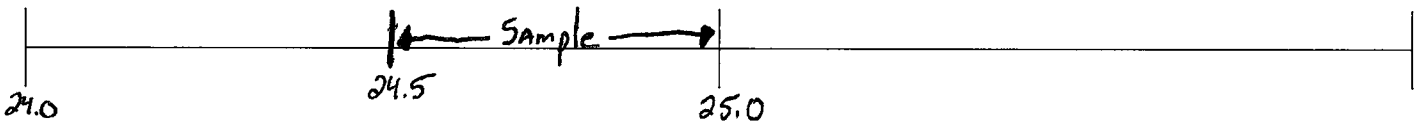
**UNCONFINED COMPRESSION**

North Dakota Department of Transportation, Materials & Research  
SFN 50460 (5-2016)

Project Number <i>IM-2-094(143)260</i>
Boring Number <i>2</i>
Depth <i>24.5 to 25.0</i>
AASHTO T-208 Tested by: <i>DT/MD</i>
Moisture Can Number <i>548</i>
Wet Wt + Can <i>76.24</i>
Dry Wt + Can <i>66.51</i>
Wt of Can <i>17.01</i>

Field Sample Number <i>SS-387-16</i>	Lab Number <i>UC-23-16</i>
Weight of Sample <i>1241.15</i>	Test Number <i>1</i> of <i>1</i>
Diameter	Height
1. <i>2.860</i>	1. <i>5.740</i>
2. <i>2.857</i>	2. <i>5.745</i>
3. <i>2.854</i>	3. <i>5.741</i>
Average <i>2.849</i>	Average <i>5.742</i>

Total Length: *15 1/2" Ben Cly with I.O. + Coal Deposits, Rock up to 2"*



Field Sample Number	Lab Number	Depth
Weight of Sample	Test Number of	AASHTO T-208 Tested by:
Diameter	Height	Moisture Can Number
1.	1.	
2.	2.	Wet Wt + Can
3.	3.	Dry Wt + Can
Average	Average	Wt of Can

Total Length:

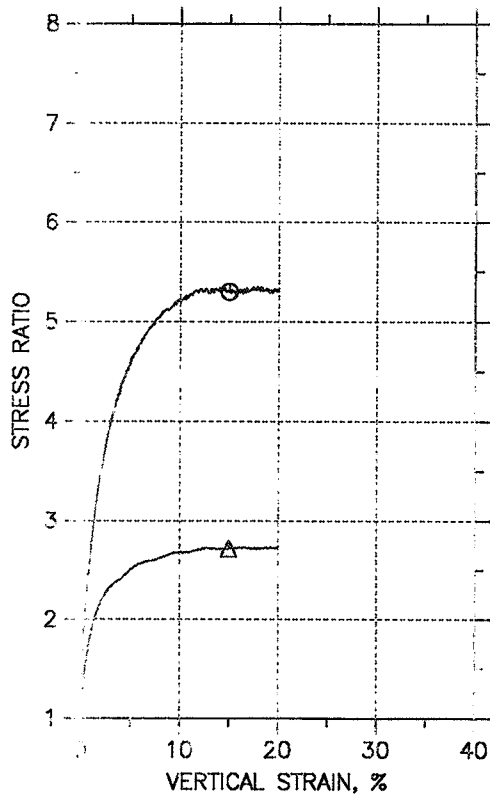
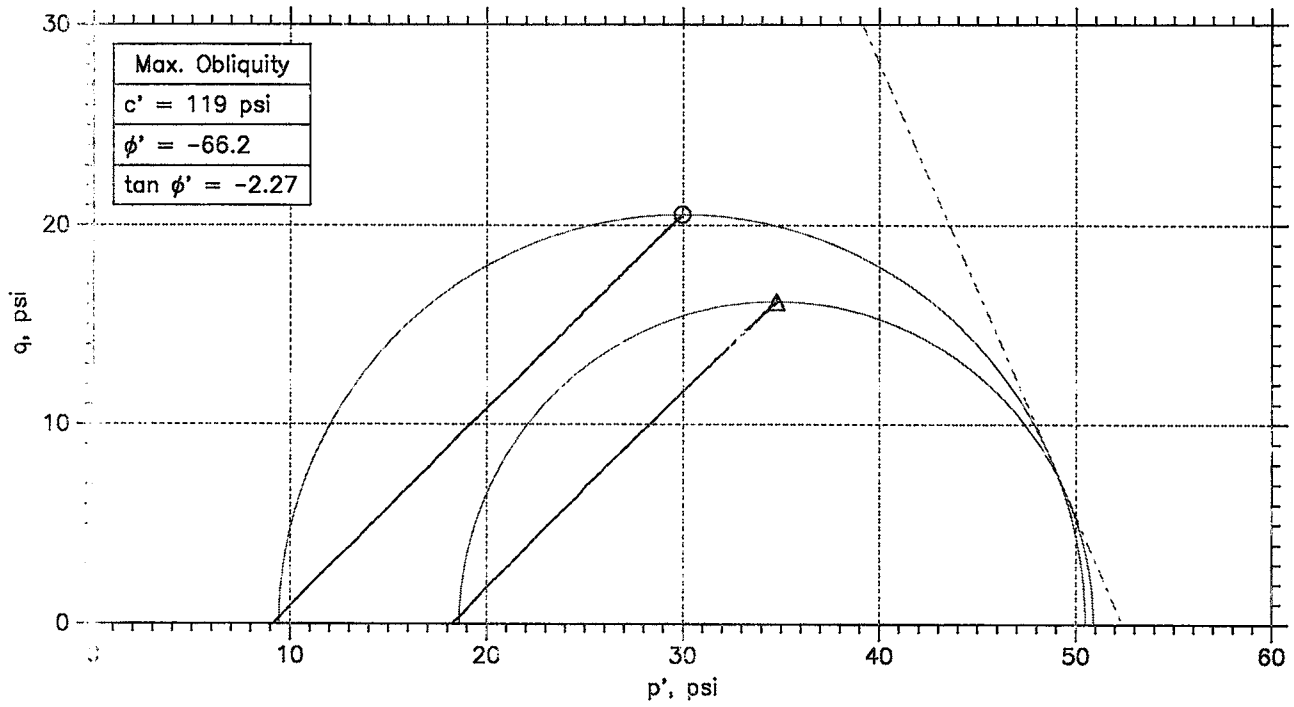


Field Sample Number	Lab Number	Depth
Weight of Sample	Test Number of	AASHTO T-208 Tested by:
Diameter	Height	Moisture Can Number
1.	1.	
2.	2.	Wet Wt + Can
3.	3.	Dry Wt + Can
Average	Average	Wt of Can

Total Length:



Test File



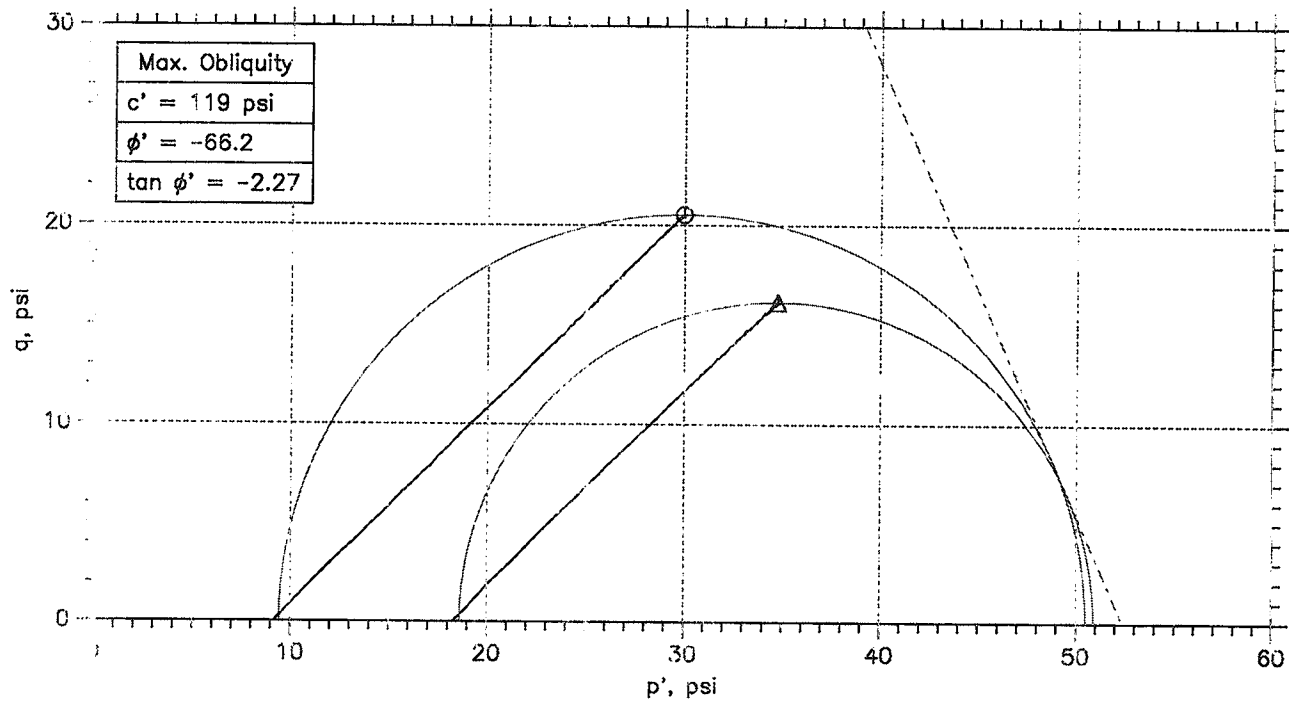
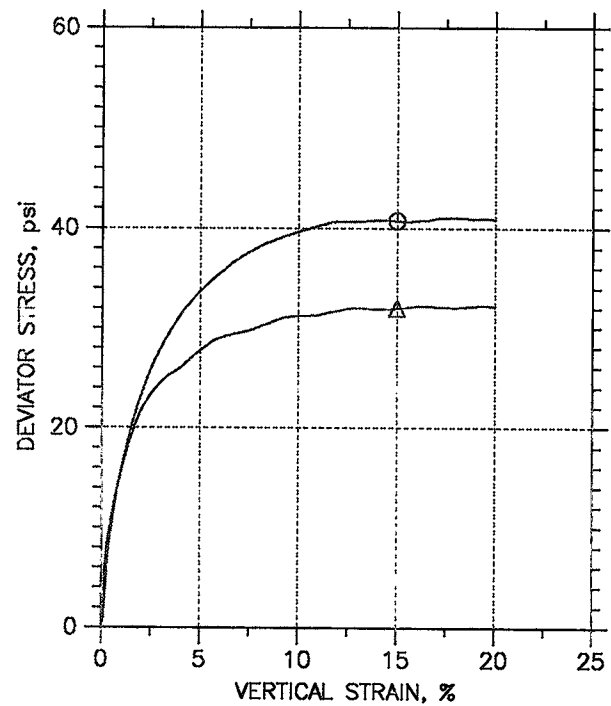
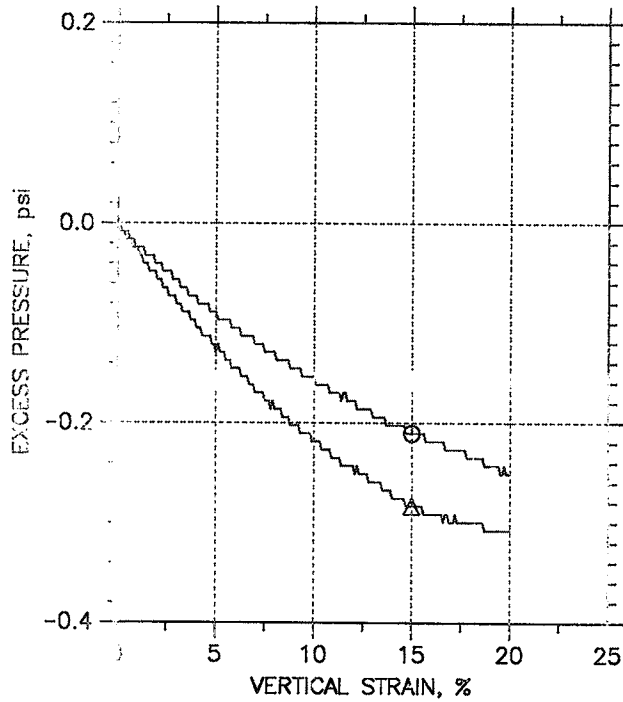
Symbol	⊙	△	
Sample No.	SS-385-16	SS-385-16	
Test No.	UU-41-16	UU-42-16	
Depth	19.3-19.8	19.8-20.3	
Initial	Diameter, in	2.845	2.836
	Height, in	5.751	5.749
	Water Content, %	18.8	20.9
	Dry Density, pcf	111.9	108.3
	Saturation, %	104.2	105.0
Before Shear	Void Ratio	0.478	0.528
	Water Content, %	18.0	19.9
	Dry Density, pcf	112.	108.3
	Saturation*, %	100.0	100.0
	Void Ratio	0.478	0.528
Back Press., psi	.0	.0	
Ver. Eff. Cons. Stress, psi	9.186	13.27	
Shear Strength, psi	20.36	16.	
Strain at Failure, %	15	15	
Strain Rate, %/min	1	1	
B-Value	----	----	
Estimated Specific Gravity	2.65	2.65	
Liquid Limit	----	----	
Plastic Limit	----	----	

	Project: IM-2-094(143)260	
	Location:	
	Project No.:	
	Boring No.: 2	
	Sample Type:	
	Description: T.L. 19 3/4" Brn Cly with I.O deposits, Rock up to 3/4"	
	Remarks:	

Phase calculations based on start and end of test.

\* Saturation is set to 100% for phase calculations.

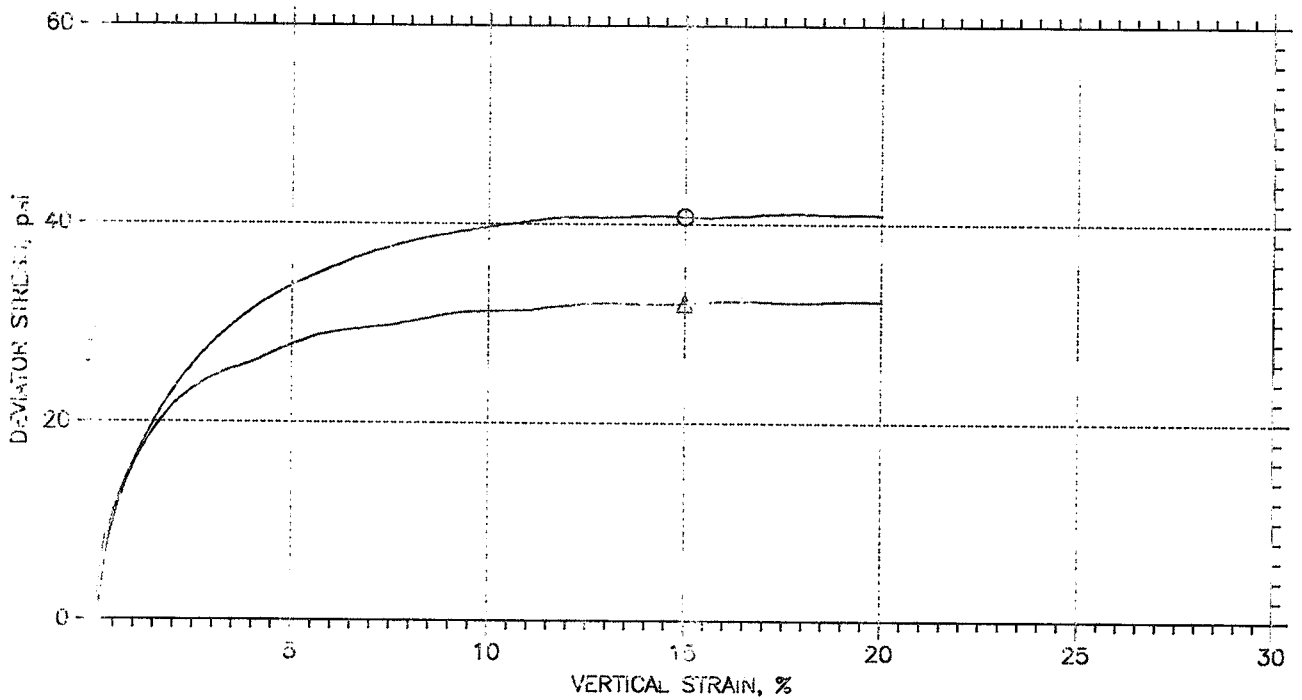
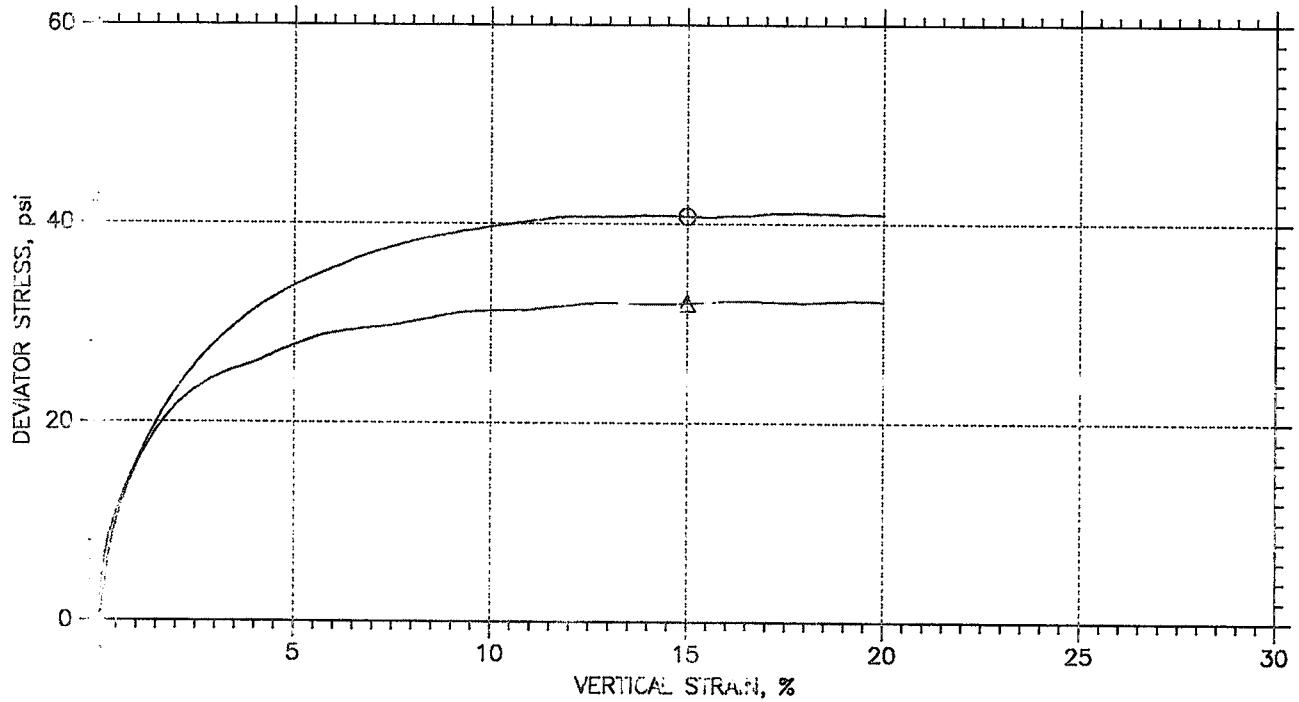
Test File




	Sample No.	Test No.	Depth	Tested By	Test Date	Checked By	Check Date	Test File
○	SS-385-16	UU-41-16	19.3-19.8	DT	6/6/16	MD		UU-41-2016.dat
△	SS-385-16	UU-42-16	19.8-20.3	DT	6/6/16	MD		UU-42-2016.dat

	Project: Iwi-2-094(143)260		Location:		Project No.:	
	Boring No.: 2		Sample Type:			
	Description: T.L. 19 3/4" Brn Cly with I.O deposits, Rock up to 3/4"					
	Remarks:					

Test File



Sample No.	Test No.	Depth	Tested By	Test Date	Checked By	Check Date	Test File
⊙ SS-385-16	UU-41-16	19.3-19.8	DT	6/6/16	MD		UU-41-2016.dat
△ SS-385-16	UU-42-16	19.8-20.3	DT	6/6/16	MD		UU-42-2016.dat

	Project: IM-2-094(143)260	Location:	Project No.:
	Boring No.: 2	Sample Type:	
	Description: T.L. 19 3/4" Brn Cly with I.O deposits, Rock up to 3/4"		
	Remarks:		

SS-385-16

TRIAXIAL TEST

Project: TM-2-094 143)260  
 Boring No.: 2  
 Sample No.: SS-385-16  
 Test No.: UU-41-1

Location:  
 Tested By: DT  
 Test Date: 6/6/16  
 Sample Type:

Project No.:  
 Checked By: MD  
 Depth: 19.3-19.8  
 Elevation:

Soil Description: T.L. 19 3/4" Brn Cly with I.O deposits, Rock up to 3/4"  
 Remarks:

Specimen Height: 5.75 in  
 Specimen Area: 6.357 in<sup>2</sup>  
 Specimen Volume: 599.10 cc

Piston Area: 0.16 in<sup>2</sup>  
 Piston Friction: 0.00 lb  
 Piston Weight: 0.00 lb

Filter Strip Correction: 0.00 psi  
 Membrane Correction: 4.20 lb/in  
 Correction Type: Uniform

Liquid Limit: ---

Plastic Limit: ---

Estimated Specific Gravity: 2.65

	Before Test Trimmings	Before Test Specimen	After Test Specimen	After Test Trimmings
Container ID	S 40	---		
Wt. Container + Wet Soil, gm	68.57	---	---	0
Wt. Container + Dry Soil, gm	60.4	---	---	0
Wt. Container, gm	16.93	---	---	0
Wt. Wet Soil, gm	51.64	1276.1	1074.2	0
Wt. Dry Soil, gm	43.47	1074.2	1074.2	0
Wt. Water, gm	8.17	201.89	0	0
Water Content, %	18.79	18.79	0.00	0.00
Void Ratio	---	0.48	0.48	---
Degree of Saturation, %	---	104.20	0.00	---
Dry Unit Weight, pcf	---	111.93	111.96	---

Initial

Height: 5.751 in  
 Area: 6.357 in<sup>2</sup>  
 Volume: 599.1 cc

Moisture: 18.79 %  
 Void Ratio: 0.48  
 Dry Unit Weight: 111.93 pcf  
 Saturation: 104.20 %

End of Initialization

Time: 4.8995 min  
 Total Vertical Stress: 9.186 psi  
 Total Horizontal Stress: 9.1869 psi  
 Pore Pressure: 0 psi  
 Effective Vertical Stress: 9.186 psi  
 Effective Horizontal Stress: 9.1869 psi

Height Change: 0.000416 in  
 Area Change: 0 in<sup>2</sup>  
 Volume Change: 0.13001 cc  
 Water Change: 0.0080445 cc  
 Correction: 8.2621 cc

Height: 5.7506 in  
 Area: 6.357 in<sup>2</sup>  
 Volume: 598.97 cc  
 Moisture: 18.02 %  
 Void Ratio: 0.48  
 Dry Unit Weight: 111.96 pcf  
 Saturation: 100.00 %

End of Consolidation/A

Time: 4.8885 min  
 Total Vertical Stress: 9.186 psi  
 Total Horizontal Stress: 9.1869 psi  
 Pore Pressure: 0 psi  
 Effective Vertical Stress: 9.186 psi  
 Effective Horizontal Stress: 9.1869 psi

Height Change: 0.000416 in  
 Area Change: 0 in<sup>2</sup>  
 Volume Change: 0.13001 cc  
 Water Change: 0.0080445 cc  
 Correction: 8.2621 cc

Height: 5.7506 in  
 Area: 6.357 in<sup>2</sup>  
 Volume: 598.97 cc  
 Moisture: 18.02 %  
 Void Ratio: 0.48  
 Dry Unit Weight: 111.96 pcf  
 Saturation: 100.00 %

End of Saturation

Time: 4.8885 min  
 Total Vertical Stress: 9.186 psi  
 Total Horizontal Stress: 9.1869 psi  
 Pore Pressure: 0 psi  
 Effective Vertical Stress: 9.186 psi  
 Effective Horizontal Stress: 9.1869 psi

Height Change: 0.000416 in  
 Area Change: 0 in<sup>2</sup>  
 Volume Change: 0.13001 cc  
 Water Change: 0.0080445 cc  
 Correction: 8.2621 cc

Height: 5.7506 in  
 Area: 6.357 in<sup>2</sup>  
 Volume: 598.97 cc  
 Moisture: 18.02 %  
 Void Ratio: 0.48  
 Dry Unit Weight: 111.96 pcf  
 Saturation: 100.00 %

End of Consolidation/B

Time: 4.8885 min  
 Total Vertical Stress: 9.186 psi  
 Total Horizontal Stress: 9.1869 psi  
 Pore Pressure: 0 psi  
 Effective Vertical Stress: 9.186 psi  
 Effective Horizontal Stress: 9.1869 psi

Height Change: 0.000416 in  
 Area Change: 0 in<sup>2</sup>  
 Volume Change: 0.13001 cc  
 Water Change: 0.0080445 cc  
 Correction: 8.2621 cc

Height: 5.7506 in  
 Area: 6.357 in<sup>2</sup>  
 Volume: 598.97 cc  
 Moisture: 18.02 %  
 Void Ratio: 0.48  
 Dry Unit Weight: 111.96 pcf  
 Saturation: 100.00 %

End of Shear

Time: 25.129 min  
 Total Vertical Stress: 50.198 psi  
 Total Horizontal Stress: 9.2595 psi  
 Pore Pressure: -0.18093 psi  
 Effective Vertical Stress: 50.438 psi  
 Effective Horizontal Stress: 9.5105 psi

Height Change: 1.1807 in  
 Area Change: -1.5885 in<sup>2</sup>  
 Volume Change: 0.13001 cc  
 Water Change: 0.0080445 cc  
 Correction: 201.88 cc

Height: 4.6003 in  
 Area: 7.9455 in<sup>2</sup>  
 Volume: 598.97 cc  
 Moisture: 0.00 %  
 Void Ratio: 0.48  
 Dry Unit Weight: 111.96 pcf  
 Saturation: 0.00 %

At Failure

Time: 20.105 min  
 Total Vertical Stress: 49.939 psi  
 Total Horizontal Stress: 9.2746 psi  
 Pore Pressure: -0.11046 psi  
 Effective Vertical Stress: 50.209 psi  
 Effective Horizontal Stress: 9.4851 psi

Height Change: 0.863 in  
 Area Change: -1.1223 in<sup>2</sup>  
 Volume Change: 0.13001 cc  
 Water Change: 0.0080445 cc  
 Correction: 0 cc

Height: 4.888 in  
 Area: 7.4793 in<sup>2</sup>  
 Volume: 598.97 cc  
 Moisture: 18.02 %  
 Void Ratio: 0.48  
 Dry Unit Weight: 111.96 pcf  
 Saturation: 100.00 %



55-385-16

TRIAXIAL TEST

Project: TM-2-094(143)260  
 Boring No.: 2  
 Sample No.: SS-385-16  
 Test No.: DU-42-16

Location:  
 Tested By: DT  
 Test Date: 6/6/16  
 Sample Type:

Project No.:  
 Checked By: MD  
 Depth: 19.8-20.3  
 Elevation:

Soil Description: T.L. 19 3/4" Brn Cly with I.O and Coal deposits, Rock up to 3/4"  
 Remarks:

Specimen Height: 1.75 in  
 Specimen Area: 6.32 in<sup>2</sup>  
 Specimen Volume: 595.11 cc

Piston Area: 0.16 in<sup>2</sup>  
 Piston Friction: 0.00 lb  
 Piston Weight: 0.00 lb

Filter Strip Correction: 0.00 psi  
 Membrane Correction: 4.20 lb/in  
 Correction Type: Uniform

Liquid Limit: ---

Plastic Limit: ---

Estimated Specific Gravity: 2.65

	Before Test Trimmings	Before Test Specimen	After Test Specimen	After Test Trimmings
Container ID	S 28	---		
Wt. Container + Wet Soil, gm	74.69	---	---	0
Wt. Container + Dry Soil, gm	64.45	---	---	0
Wt. Container, gm	15.47	---	---	0
Wt. Wet Soil, gm	59.22	1248.3	1032.4	0
Wt. Dry Soil, gm	48.98	1032.4	1032.4	0
Wt. Water, gm	10.24	215.84	0	0
Water Content, %	20.91	20.91	0.00	0.00
Void Ratio	---	0.53	0.53	---
Degree of Saturation, %	---	105.02	0.00	---
Dry Unit Weight, pcf	---	108.3	108.3	---

Initial

Height: 5.749 in  
 Area: 6.3169 in<sup>2</sup>  
 Volume: 595.11 cc

Moisture: 20.91 %  
 Void Ratio: 0.53  
 Dry Unit Weight: 108.3 pcf  
 Saturation: 105.02 %

End of Initialation

Time: 4.9986 min  
 Total Vertical Stress: 18.27 psi  
 Total Horizontal Stress: 18.269 psi  
 Pore Pressure: 0 psi  
 Effective Vertical Stress: 18.27 psi  
 Effective Horizontal Stress: 18.269 psi

Height Change: 0 in  
 Area Change: 0 in<sup>2</sup>  
 Volume Change: 0 cc  
 Water Change: 0.24509 cc  
 Correction: 10.081 cc

Height: 5.749 in  
 Area: 6.3169 in<sup>2</sup>  
 Volume: 595.11 cc

Moisture: 19.91 %  
 Void Ratio: 0.53  
 Dry Unit Weight: 108.3 pcf  
 Saturation: 100.00 %

End of Consolidation/A

Time: 4.9946 min  
 Total Vertical Stress: 18.27 psi  
 Total Horizontal Stress: 18.269 psi  
 Pore Pressure: 0 psi  
 Effective Vertical Stress: 18.27 psi  
 Effective Horizontal Stress: 18.269 psi

Height Change: 0 in  
 Area Change: 0 in<sup>2</sup>  
 Volume Change: 0 cc  
 Water Change: 0.24509 cc  
 Correction: 10.081 cc

Height: 5.749 in  
 Area: 6.3169 in<sup>2</sup>  
 Volume: 595.11 cc

Moisture: 19.91 %  
 Void Ratio: 0.53  
 Dry Unit Weight: 108.3 pcf  
 Saturation: 100.00 %

End of Shear

Time: 15.05 min  
 Total Vertical Stress: 18.27 psi  
 Total Horizontal Stress: 18.269 psi  
 Pore Pressure: 0 psi  
 Effective Vertical Stress: 18.27 psi  
 Effective Horizontal Stress: 18.269 psi

Height Change: 0 in  
 Area Change: 0 in<sup>2</sup>  
 Volume Change: 0 cc  
 Water Change: 0.24509 cc  
 Correction: 10.081 cc

Height: 5.749 in  
 Area: 6.3169 in<sup>2</sup>  
 Volume: 595.11 cc

Moisture: 19.91 %  
 Void Ratio: 0.53  
 Dry Unit Weight: 108.3 pcf  
 Saturation: 100.00 %

End of Consolidation/B

Time: 4.9946 min  
 Total Vertical Stress: 18.27 psi  
 Total Horizontal Stress: 18.269 psi  
 Pore Pressure: 0 psi  
 Effective Vertical Stress: 18.27 psi  
 Effective Horizontal Stress: 18.269 psi

Height Change: 0 in  
 Area Change: 0 in<sup>2</sup>  
 Volume Change: 0 cc  
 Water Change: 0.24509 cc  
 Correction: 10.081 cc

Height: 5.749 in  
 Area: 6.3169 in<sup>2</sup>  
 Volume: 595.11 cc

Moisture: 19.91 %  
 Void Ratio: 0.53  
 Dry Unit Weight: 108.3 pcf  
 Saturation: 100.00 %

End of Shear

Time: 15.053 min  
 Total Vertical Stress: 50.557 psi  
 Total Horizontal Stress: 18.414 psi  
 Pore Pressure: 30.976 psi  
 Effective Vertical Stress: 50.864 psi  
 Effective Horizontal Stress: 18.722 psi

Height Change: 1.1501 in  
 Area Change: -1.1797 in<sup>2</sup>  
 Volume Change: 0 cc  
 Water Change: 0.24509 cc  
 Correction: 215.6 cc

Height: 4.5989 in  
 Area: 7.8965 in<sup>2</sup>  
 Volume: 595.11 cc

Moisture: 0.00 %  
 Void Ratio: 0.53  
 Dry Unit Weight: 108.3 pcf  
 Saturation: 0.00 %

At Failure

Time: 20.05 min  
 Total Vertical Stress: 50.338 psi  
 Total Horizontal Stress: 18.343 psi  
 Pore Pressure: 30.98331 psi  
 Effective Vertical Stress: 50.622 psi  
 Effective Horizontal Stress: 18.626 psi

Height Change: 0.88235 in  
 Area Change: -1.1117 in<sup>2</sup>  
 Volume Change: 2.9422e-017 cc  
 Water Change: 0.24509 cc  
 Correction: 0 cc

Height: 4.8866 in  
 Area: 7.4286 in<sup>2</sup>  
 Volume: 595.11 cc

Moisture: 19.91 %  
 Void Ratio: 0.53  
 Dry Unit Weight: 108.3 pcf  
 Saturation: 100.00 %

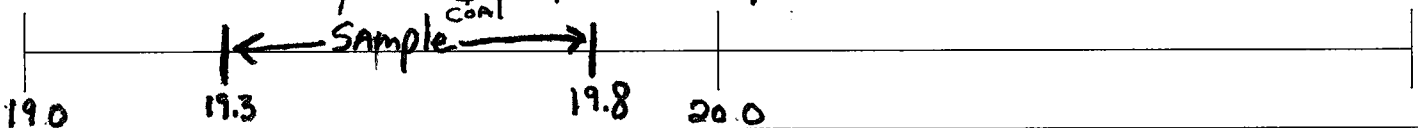
**TRIAxIAL UU(Q) CCU(R) CD(S)**

North Dakota Department of Transportation, Materials & Research  
SFN 50459 (5-2016)

Project Number <b>IM-2-094(143)260</b>
Boring Number <b>2</b>

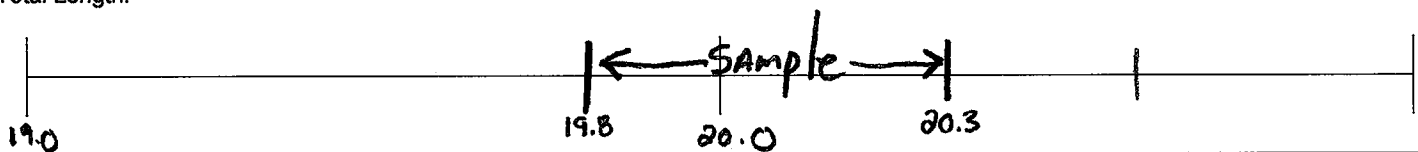
Field Sample Number <b>55-385-16</b>		Lab Number <b>UU-41-16</b>		Depth <b>19.3 to 19.8</b>
Weight of Sample <b>1276.07</b>		Confining Pressure <b>9.2</b>		Test Number <b>1</b> of <b>1</b>
Diameter	<b>2.836</b>	<b>2.851</b>	Height	Moisture Can Number <b>540</b>
	<b>2.837</b>	<b>2.855</b>		Wet Wt + Can <b>68.57</b>
	<b>2.857</b>	<b>2.834</b>		Dry Wt + Can <b>60.40</b>
	Average <b>2.845</b>			Average <b>5.751</b>

Total Length: **19 3/4" Ben Cly with I.O. Deposits, Rock up to 3/4"**



Field Sample Number <b>55-385-16</b>		Lab Number <b>UU-42-16</b>		Depth <b>19.8 to 20.3</b>
Weight of Sample <b>1248.26</b>		Confining Pressure <b>18.3</b>		Test Number <b>2</b> of <b>2</b>
Diameter	<b>2.864</b>	<b>2.843</b>	Height	Moisture Can Number <b>528</b>
	<b>2.838</b>	<b>2.816</b>		Wet Wt + Can <b>74.69</b>
	<b>2.823</b>	<b>2.832</b>		Dry Wt + Can <b>64.45</b>
	Average <b>2.836</b>			Average <b>5.749</b>

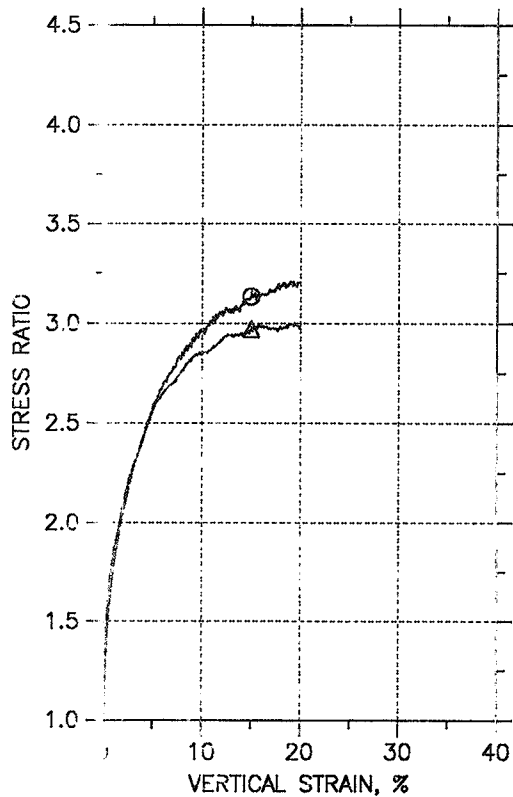
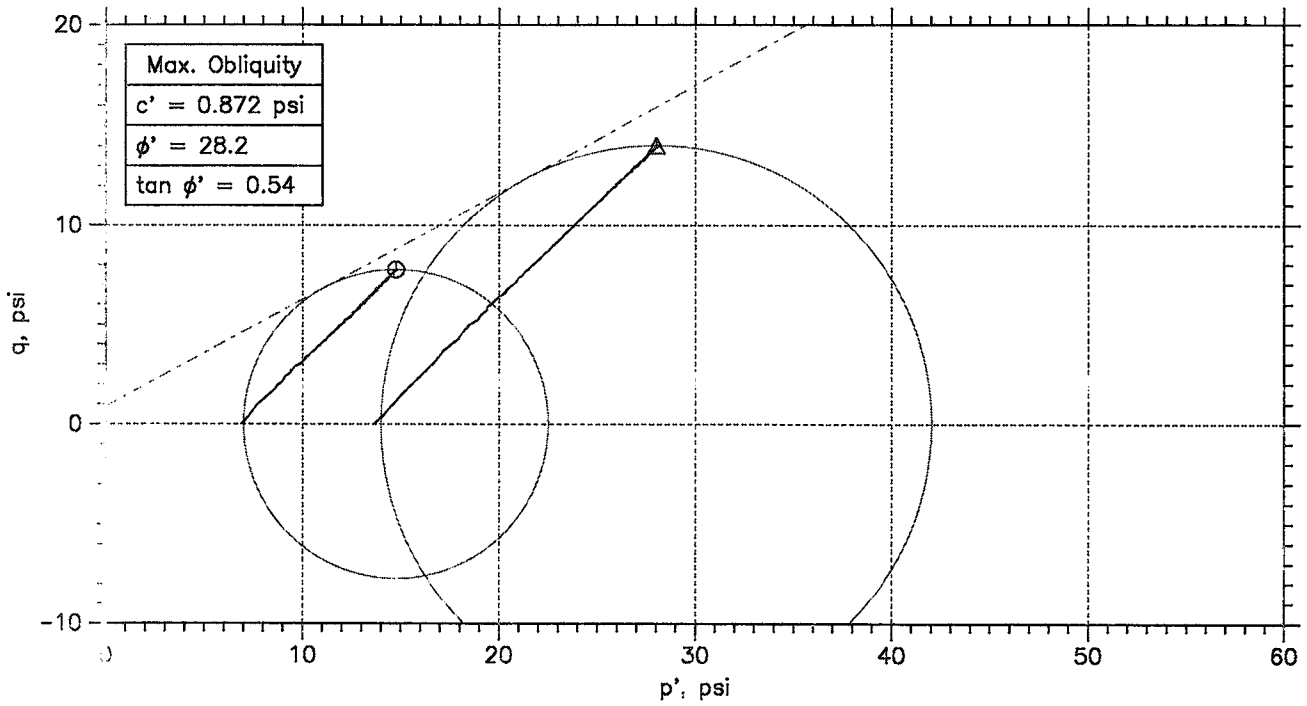
Total Length:



Field Sample Number		Lab Number		Depth
Weight of Sample		Confining Pressure		Test Number of
Diameter			Height	Moisture Can Number
				Wet Wt + Can
				Dry Wt + Can
	Average			Average

Total Length:

Test File



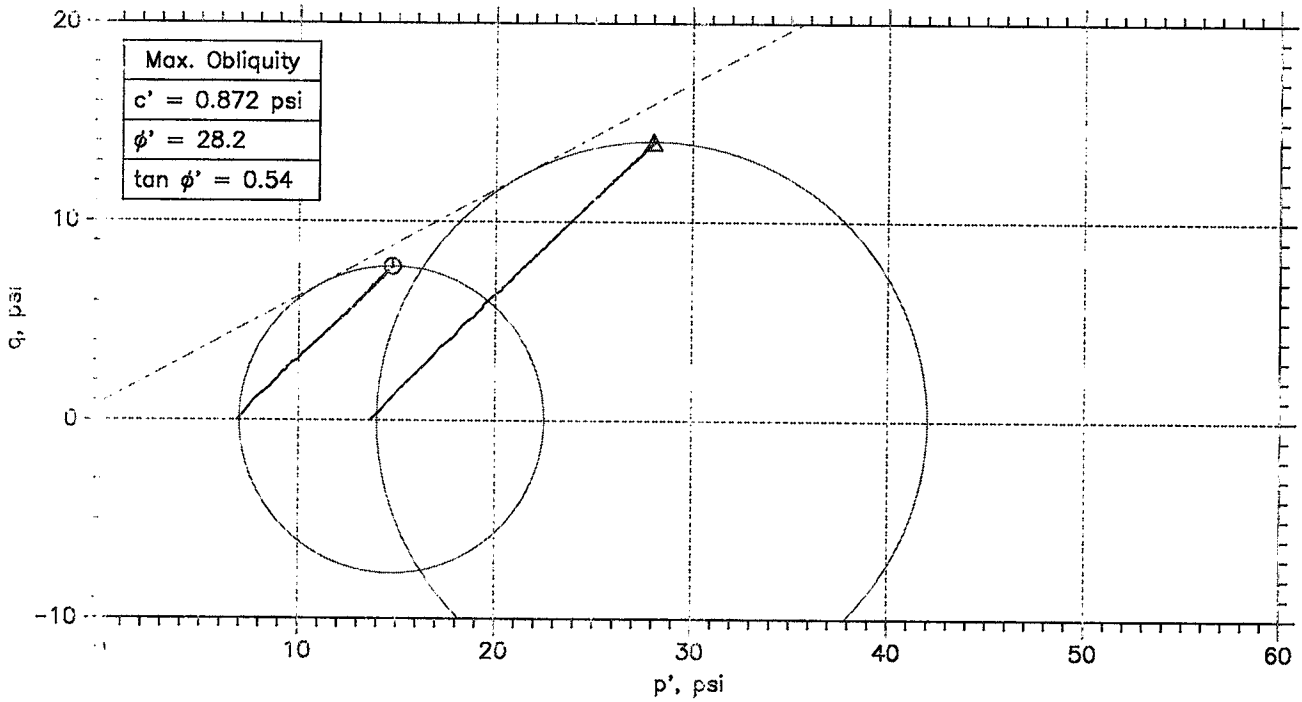
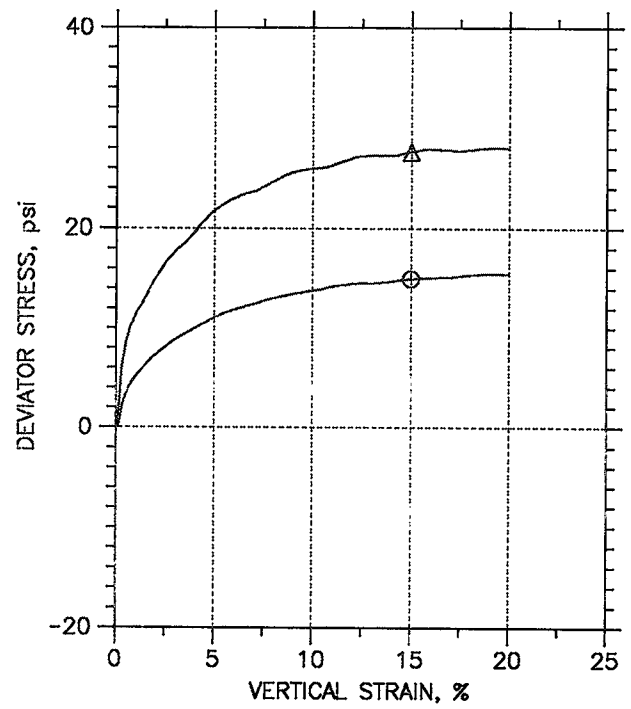
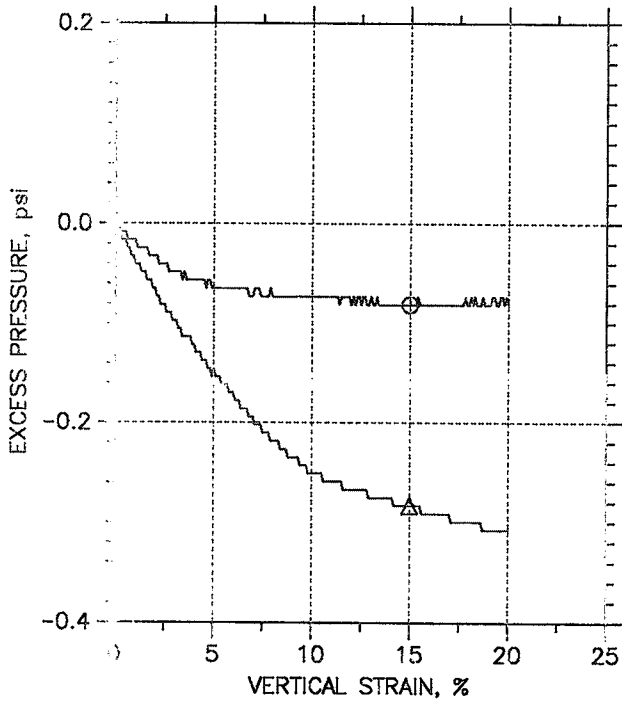
Symbol	○	△	
Sample No.	SS-383-16	SS-383-16S	
Test No.	UU-39-16	UU-40-16	
Depth	14.3-14.8	14.8-15.3	
Initial	Diameter, in	2.817	2.846
	Height, in	5.734	5.735
	Water Content, %	20.1	20.7
	Dry Density, pcf	107.4	108.3
	Saturation, %	98.3	103.9
Before Shear	Void Ratio	0.541	0.527
	Water Content, %	20.0	19.9
	Dry Density, pcf	107.4	108.4
	Saturation*, %	98.2	100.0
	Void Ratio	0.54	0.526
Back Press., psi	-0.008095	.0	
Var. Eff. Cons. Stress, psi	6.867	13.67	
Shear Strength, psi	7.483	13.8	
Strain at Failure, %	15	15	
Strain Rate, %/min	1	1	
B-Value	---	---	
Estimated Specific Gravity	2.65	2.65	
Liquid Limit	---	---	
Plastic Limit	---	---	

	Project: IM-2-094(143)260				
	Location:				
	Project No.:				
	Boring No.: 2				
	Sample Type:				
	Description: T.L. 19 1/4" Brn Cly with I.O deposits, Rock up to 1/2"				
Remarks:					

Phase calculations based on start and end of test.

\* Saturation is set to 100% for phase calculations.

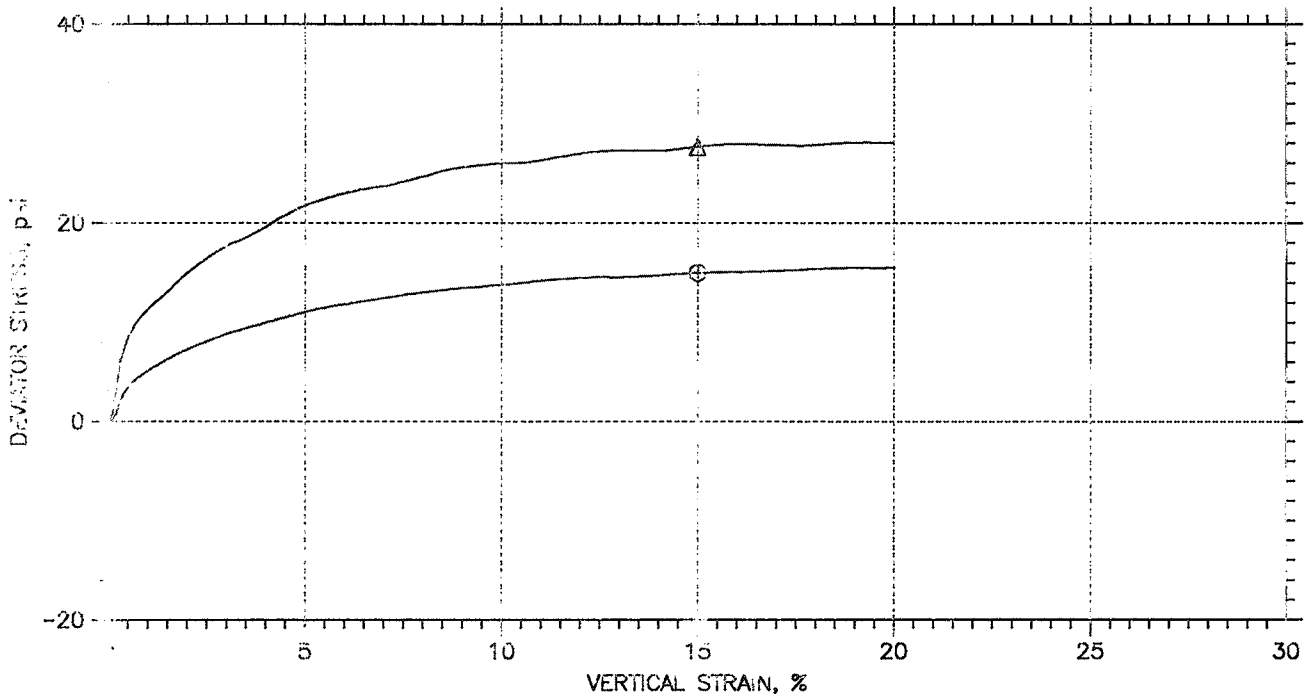
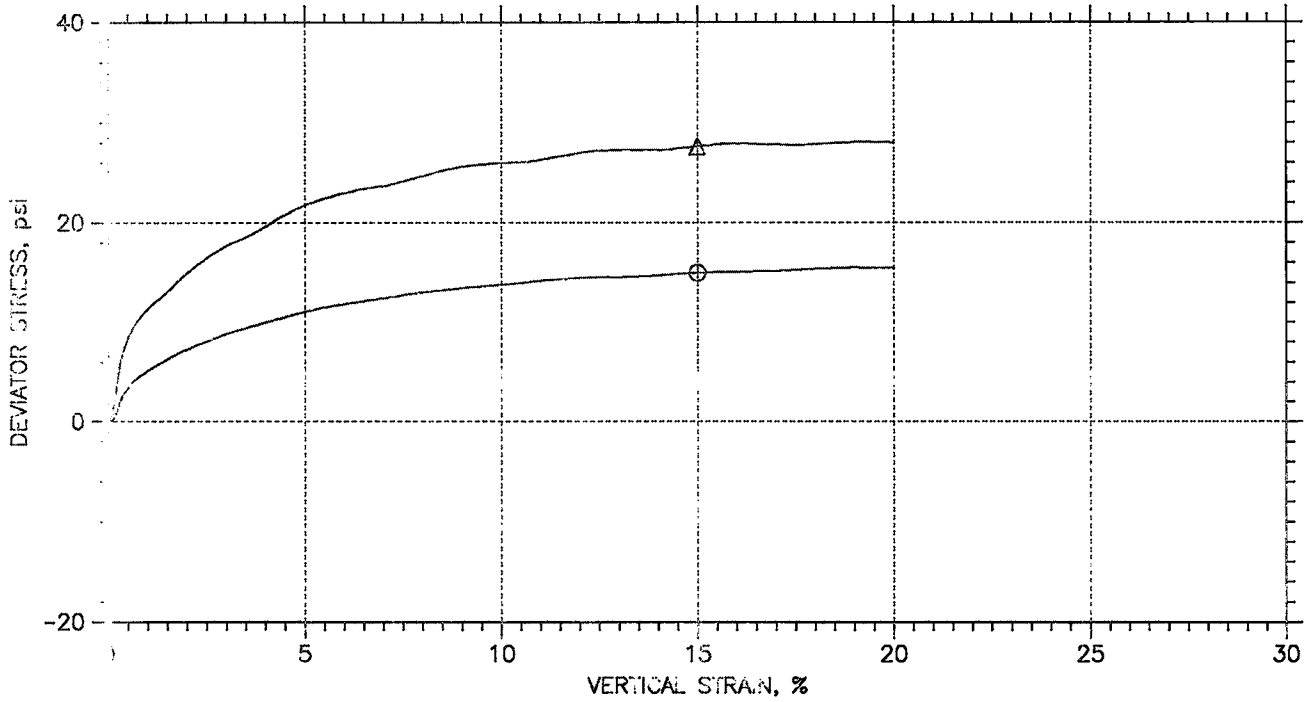
Test File




Sample No.	Test No.	Depth	Tested By	Test Date	Checked By	Check Date	Test File
○ SS-383-16	UU-39-16	14.3-14.8	DT	6/6/16	MD		UU-39-2016.dat
△ SS-383-16	UU-40-16	14.8-15.3	DT	6/6/16	MD		UU-40-2016.dat

	Project: IM-2-094(143)260		Location:		Project No.:	
	Boring No.: 2		Sample Type:			
	Description: T.L. 19 1/4" Brn Clay with I.O deposits, Rock up to 1/2"					
	Remarks:					

Test File



Sample No.	Test No.	Depth	Tested By	Test Date	Checked By	Check Date	Test File
⊙ SS-383-16	UU-39-16	14.3-14.8	DT	6/6/16	MD		UU-39-2016.dat
△ SS-383-16	UU-40-16	14.8-15.3	DT	6/6/16	MD		UU-40-2016.dat

 <p>North Carolina Department of Transportation</p>	Project: IM-2-094(143)260		Location:		Project No.:		
	Boring No.: 2		Sample Type:				
	Description: T.L. 19 1/4" Brn Cly with I.O deposits, Rock up to 1/2"						
	Remarks:						

55-383-16

TRIAxIAL TEST

Project: EM-2-044 143)260  
Boring No.: 2  
Sample No.: SS-383-16  
Test No.: UU-39-14

Location:  
Tested By: DT  
Test Date: 6/6/16  
Sample Type:

Project No.:  
Checked By: MD  
Depth: 14.3-14.8  
Elevation:

Soil Description: I.L. 19 1/4" Brn Cly with I.O deposits, Rock up to 1/2"  
Remarks:

Specimen Height: 5.73 in  
Specimen Area: 6.23 in<sup>2</sup>  
Specimen Volume: 585.63 cc

Piston Area: 0.16 in<sup>2</sup>  
Piston Friction: 0.00 lb  
Piston Weight: 0.00 lb

Filter Strip Correction: 0.00 psi  
Membrane Correction: 4.20 lb/in  
Correction Type: Uniform

Liquid Limit: ---

Plastic Limit: ---

Estimated Specific Gravity: 2.65

	Before Test Trimmings	Before Test Specimen	After Test Specimen	After Test Trimmings
Container ID	S 11	---		
Wt. Container + Wet Soil, gm	82.91	---	---	0
Wt. Container + Dry Soil, gm	71.88	---	---	0
Wt. Container, gm	16.89	---	---	0
Wt. Wet Soil, gm	66.02	1209.3	1007.2	0
Wt. Dry Soil, gm	54.99	1007.2	1007.2	0
Wt. Water, gm	11.03	202.03	0	0
Water Content, %	20.06	20.06	0.00	0.00
Void Ratio	---	0.54	0.54	---
Degree of Saturation, %	---	98.29	0.00	---
Dry Unit Weight, pcf	---	107.37	107.4	---

Initial

Height: 5.734 in  
Area: 6.2325 in<sup>2</sup>  
Volume: 585.63 cc

Moisture: 20.06 %  
Void Ratio: 0.54  
Dry Unit Weight: 107.37 pcf  
Saturation: 98.29 %

End of Initialization

Time: 11.09 min  
Total Vertical Stress: 6.8587 psi  
Total Horizontal Stress: 6.8619 psi  
Pore Pressure: -0.0080946 psi  
Effective Vertical Stress: 6.8668 psi  
Effective Horizontal Stress: 6.87 psi

Height Change: 0.00046222 in  
Area Change: 0 in<sup>2</sup>  
Volume Change: 0.14162 cc  
Water Change: 0.22203 cc  
Correction: 0 cc

Height: 5.7335 in  
Area: 6.2325 in<sup>2</sup>  
Volume: 585.49 cc  
Moisture: 20.04 %  
Void Ratio: 0.54  
Dry Unit Weight: 107.4 pcf  
Saturation: 98.25 %

End of Consolidation/A

Time: 11.129 min  
Total Vertical Stress: 6.8587 psi  
Total Horizontal Stress: 6.8619 psi  
Pore Pressure: -0.0080946 psi  
Effective Vertical Stress: 6.8668 psi  
Effective Horizontal Stress: 6.87 psi

Height Change: 0.00046222 in  
Area Change: 0 in<sup>2</sup>  
Volume Change: 0.14162 cc  
Water Change: 0.22203 cc  
Correction: 0 cc

Height: 5.7335 in  
Area: 6.2325 in<sup>2</sup>  
Volume: 585.49 cc  
Moisture: 20.04 %  
Void Ratio: 0.54  
Dry Unit Weight: 107.4 pcf  
Saturation: 98.25 %

End of Saturation

Time: 11.129 min  
Total Vertical Stress: 6.8587 psi  
Total Horizontal Stress: 6.8619 psi  
Pore Pressure: -0.0080946 psi  
Effective Vertical Stress: 6.8668 psi  
Effective Horizontal Stress: 6.87 psi

Height Change: 0.00046222 in  
Area Change: 0 in<sup>2</sup>  
Volume Change: 0.14162 cc  
Water Change: 0.22203 cc  
Correction: 0 cc

Height: 5.7335 in  
Area: 6.2325 in<sup>2</sup>  
Volume: 585.49 cc  
Moisture: 20.04 %  
Void Ratio: 0.54  
Dry Unit Weight: 107.4 pcf  
Saturation: 98.25 %

End of Consolidation/B

Time: 11.129 min  
Total Vertical Stress: 6.8587 psi  
Total Horizontal Stress: 6.8619 psi  
Pore Pressure: -0.0080946 psi  
Effective Vertical Stress: 6.8668 psi  
Effective Horizontal Stress: 6.87 psi

Height Change: 0.00046222 in  
Area Change: 0 in<sup>2</sup>  
Volume Change: 0.14162 cc  
Water Change: 0.22203 cc  
Correction: 0 cc

Height: 5.7335 in  
Area: 6.2325 in<sup>2</sup>  
Volume: 585.49 cc  
Moisture: 20.04 %  
Void Ratio: 0.54  
Dry Unit Weight: 107.4 pcf  
Saturation: 98.25 %

End of Shear

Time: 26.347 min  
Total Vertical Stress: 22.43 psi  
Total Horizontal Stress: 6.9358 psi  
Pore Pressure: -0.0080946 psi  
Effective Vertical Stress: 22.531 psi  
Effective Horizontal Stress: 7.0397 psi

Height Change: 1.1476 in  
Area Change: -1.5576 in<sup>2</sup>  
Volume Change: 0.14162 cc  
Water Change: 0.22203 cc  
Correction: 201.81 cc

Height: 4.5864 in  
Area: 7.7901 in<sup>2</sup>  
Volume: 585.49 cc  
Moisture: 0.00 %  
Void Ratio: 0.54  
Dry Unit Weight: 107.4 pcf  
Saturation: 0.00 %

At Failure

Time: 26.347 min  
Total Vertical Stress: 21.903 psi  
Total Horizontal Stress: 6.9358 psi  
Pore Pressure: -0.0080946 psi  
Effective Vertical Stress: 21.984 psi  
Effective Horizontal Stress: 7.0168 psi

Height Change: 0.86049 in  
Area Change: -1.0977 in<sup>2</sup>  
Volume Change: 0.14162 cc  
Water Change: 0.22203 cc  
Correction: 0 cc

Height: 4.8735 in  
Area: 7.3302 in<sup>2</sup>  
Volume: 585.49 cc  
Moisture: 20.04 %  
Void Ratio: 0.54  
Dry Unit Weight: 107.4 pcf  
Saturation: 98.25 %

TRIAxIAL TEST

44-383-16

Project: IM-2-094(143)260  
 Boring No.: 2  
 Sample No.: SS-38-16  
 Test No.: UU-40-1

Location:  
 Tested By: DT  
 Test Date: 6/6/16  
 Sample Type:

Project No.:  
 Checked By: MD  
 Depth: 14.8-15.3  
 Elevation:

Soil Description: T.L. 19 1/4" Brn Cly with I.O deposits, Rock up to 1/2"  
 Remarks:

Specimen Height: 1.74 in  
 Specimen Area: 6.36 in<sup>2</sup>  
 Specimen Volume: 597.85 cc

Piston Area: 0.16 in<sup>2</sup>  
 Piston Friction: 0.00 lb  
 Piston Weight: 0.00 lb

Filter Strip Correction: 0.00 psi  
 Membrane Correction: 4.20 lb/in  
 Correction Type: Uniform

Liquid Limit: ---

Elastic Limit: ---

Estimated Specific Gravity: 2.65

	Before Test Trimmings	Before Test Specimen	After Test Specimen	After Test Trimmings
Container ID	S 17	---		
Wt. Container + Wet Soil, gm	83.79	---	---	0
Wt. Container + Dry Soil, gm	72.36	---	---	0
Wt. Container, gm	17.04	---	---	0
Wt. Wet Soil, gm	66.75	1251.8	1037.5	0
Wt. Dry Soil, gm	55.32	1037.5	1037.5	0
Wt. Water, gm	11.43	214.36	0	0
Water Content, %	20.66	20.66	0.00	0.00
Void Ratio	---	0.53	0.53	---
Degree of Saturation, %	---	103.88	0.00	---
Dry Unit Weight, pcf	---	108.33	108.4	---

Initial

Height: 5.735 in  
 Area: 6.3615 in<sup>2</sup>  
 Volume: 597.85 cc  
 Moisture: 20.66 %  
 Void Ratio: 0.53  
 Dry Unit Weight: 108.33 pcf  
 Saturation: 103.88 %

End of Initialiation

Time: 4.2565 min  
 Total Vertical Stress: 13.674 psi  
 Total Horizontal Stress: 13.667 psi  
 Pore Pressure: 0 psi  
 Effective Vertical Stress: 13.674 psi  
 Effective Horizontal Stress: 13.667 psi

Height Change: 0.0012018 in  
 Area Change: 0 in<sup>2</sup>  
 Volume Change: 0.37584 cc  
 Water Change: 0.0058993 cc  
 Correction: 8.3791 cc

Height: 5.7338 in  
 Area: 6.3615 in<sup>2</sup>  
 Volume: 597.48 cc  
 Moisture: 19.85 %  
 Void Ratio: 0.53  
 Dry Unit Weight: 108.4 pcf  
 Saturation: 100.00 %

End of Consolidation/A

Time: 4.2565 min  
 Total Vertical Stress: 13.674 psi  
 Total Horizontal Stress: 13.667 psi  
 Pore Pressure: 0 psi  
 Effective Vertical Stress: 13.674 psi  
 Effective Horizontal Stress: 13.667 psi

Height Change: 0.0012018 in  
 Area Change: 0 in<sup>2</sup>  
 Volume Change: 0.37584 cc  
 Water Change: 0.0058993 cc  
 Correction: 8.3791 cc

Height: 5.7338 in  
 Area: 6.3615 in<sup>2</sup>  
 Volume: 597.48 cc  
 Moisture: 19.85 %  
 Void Ratio: 0.53  
 Dry Unit Weight: 108.4 pcf  
 Saturation: 100.00 %

End of Saturation

Time: 4.2565 min  
 Total Vertical Stress: 13.674 psi  
 Total Horizontal Stress: 13.667 psi  
 Pore Pressure: 0 psi  
 Effective Vertical Stress: 13.674 psi  
 Effective Horizontal Stress: 13.667 psi

Height Change: 0.0012018 in  
 Area Change: 0 in<sup>2</sup>  
 Volume Change: 0.37584 cc  
 Water Change: 0.0058993 cc  
 Correction: 8.3791 cc

Height: 5.7338 in  
 Area: 6.3615 in<sup>2</sup>  
 Volume: 597.48 cc  
 Moisture: 19.85 %  
 Void Ratio: 0.53  
 Dry Unit Weight: 108.4 pcf  
 Saturation: 100.00 %

End of Consolidation/B

Time: 4.2565 min  
 Total Vertical Stress: 13.674 psi  
 Total Horizontal Stress: 13.667 psi  
 Pore Pressure: 0 psi  
 Effective Vertical Stress: 13.674 psi  
 Effective Horizontal Stress: 13.667 psi

Height Change: 0.0012018 in  
 Area Change: 0 in<sup>2</sup>  
 Volume Change: 0.37584 cc  
 Water Change: 0.0058993 cc  
 Correction: 8.3791 cc

Height: 5.7338 in  
 Area: 6.3615 in<sup>2</sup>  
 Volume: 597.48 cc  
 Moisture: 19.85 %  
 Void Ratio: 0.53  
 Dry Unit Weight: 108.4 pcf  
 Saturation: 100.00 %

End of Shear

Time: 14.342 min  
 Total Vertical Stress: 41.769 psi  
 Total Horizontal Stress: 13.788 psi  
 Pore Pressure: -0.0076 psi  
 Effective Vertical Stress: 42.077 psi  
 Effective Horizontal Stress: 14.096 psi

Height Change: 1.148 in  
 Area Change: -1.5871 in<sup>2</sup>  
 Volume Change: 0.37584 cc  
 Water Change: 0.0058993 cc  
 Correction: 214.35 cc

Height: 4.587 in  
 Area: 7.9486 in<sup>2</sup>  
 Volume: 597.48 cc  
 Moisture: 0.00 %  
 Void Ratio: 0.53  
 Dry Unit Weight: 108.4 pcf  
 Saturation: 0.00 %

At Failure

Time: 19.585 min  
 Total Vertical Stress: 41.325 psi  
 Total Horizontal Stress: 13.725 psi  
 Pore Pressure: -0.08331 psi  
 Effective Vertical Stress: 41.608 psi  
 Effective Horizontal Stress: 14.008 psi

Height Change: 0.86127 in  
 Area Change: -1.1241 in<sup>2</sup>  
 Volume Change: 0.37584 cc  
 Water Change: 0.0058993 cc  
 Correction: 0 cc

Height: 4.8737 in  
 Area: 7.4856 in<sup>2</sup>  
 Volume: 597.48 cc  
 Moisture: 19.85 %  
 Void Ratio: 0.53  
 Dry Unit Weight: 108.4 pcf  
 Saturation: 100.00 %

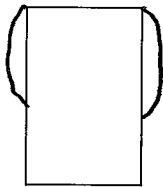
**TRIAXIAL UU(Q) CCU(R) CD(S)**

North Dakota Department of Transportation, Materials & Research  
SFN 50459 (5-2016)

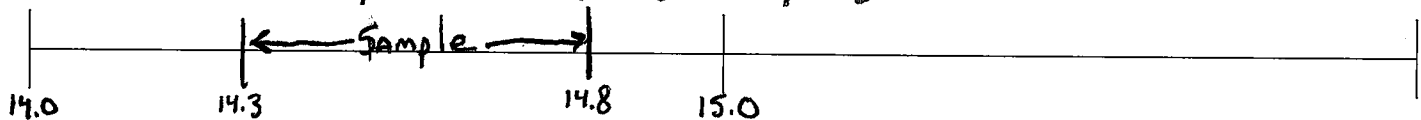
Project Number  
**IM-2-094(143)260**

Boring Number  
**2**

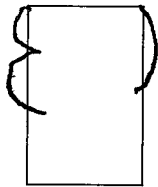
Field Sample Number <b>55-383-16</b>		Lab Number <b>UU-39-16</b>		Depth <b>14.3 to 14.8</b>	
Weight of Sample <b>1209.25</b>		Confining Pressure <b>6.9</b>		Test Number <b>1</b> of <b>2</b>	
Diameter	<b>2.784</b>	<b>2.815</b>	Height	<b>5.732</b>	Moisture Can Number <b>511</b>
	<b>2.822</b>	<b>2.829</b>		<b>5.734</b>	Wet Wt + Can <b>82.91</b>
	<b>2.822</b>	<b>2.832</b>		<b>5.735</b>	Dry Wt + Can <b>71.88</b>
	Average <b>2.817</b>		Average	<b>5.734</b>	Wt of Can <b>16.89</b>



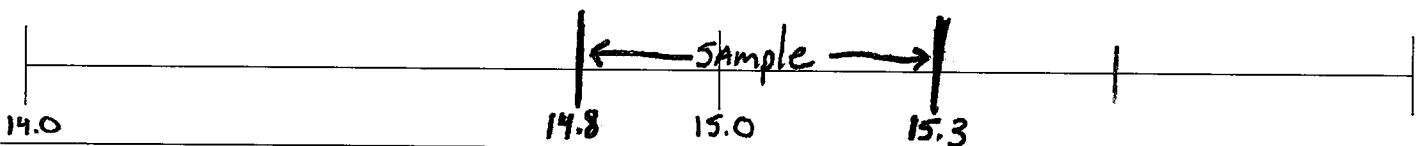
Total Length: **19 1/4" Ban Cly with I.O. Deposits Rock up to 1/2"**



Field Sample Number <b>55-383-16</b>		Lab Number <b>UU-40-16</b>		Depth <b>14.8 to 15.3</b>	
Weight of Sample <b>1251.84</b>		Confining Pressure <b>13.7</b>		Test Number <b>2</b> of <b>2</b>	
Diameter	<b>2.848</b>	<b>2.841</b>	Height	<b>5.736</b>	Moisture Can Number <b>517</b>
	<b>2.850</b>	<b>2.845</b>		<b>5.736</b>	Wet Wt + Can <b>83.79</b>
	<b>2.847</b>	<b>2.842</b>		<b>5.732</b>	Dry Wt + Can <b>72.36</b>
	Average <b>2.846</b>		Average	<b>5.735</b>	Wt of Can <b>17.04</b>



Total Length:



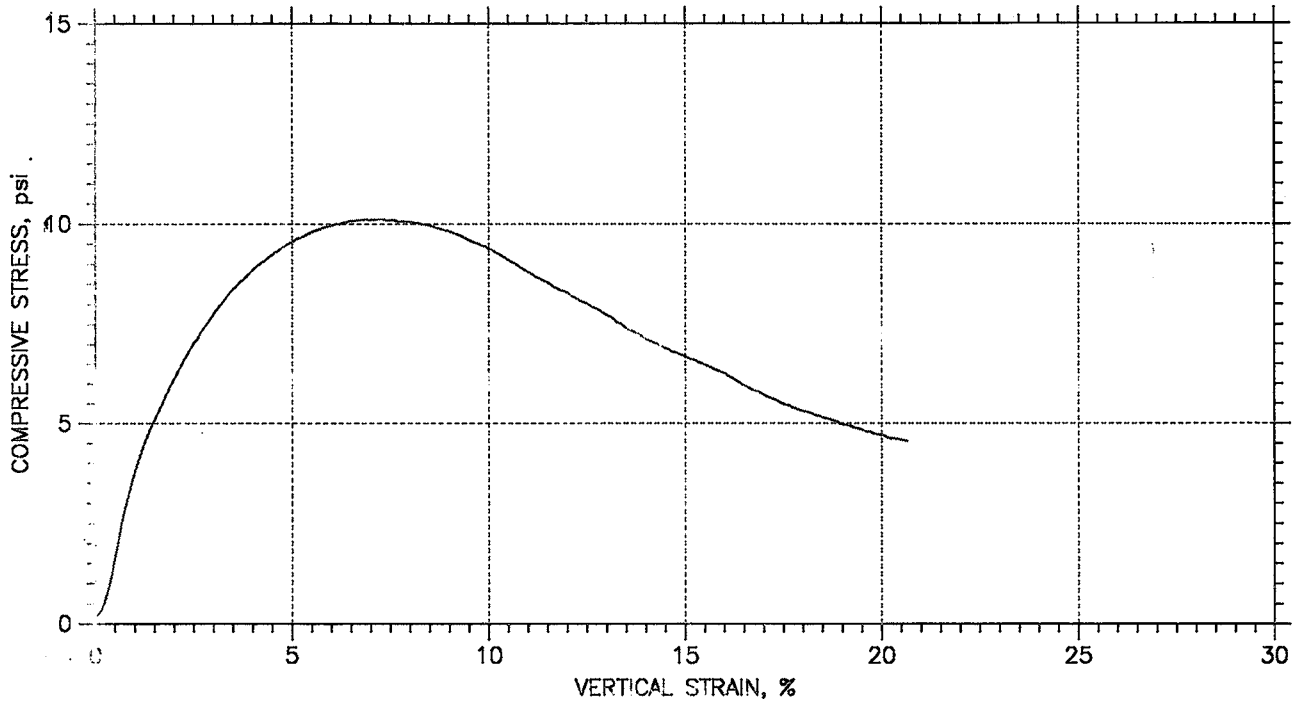
Field Sample Number		Lab Number		Depth	
Weight of Sample		Confining Pressure		Test Number of	
Diameter			Height		Moisture Can Number
					Wet Wt + Can
					Dry Wt + Can
	Average		Average	Wt of Can	

Total Length:



55-381-16

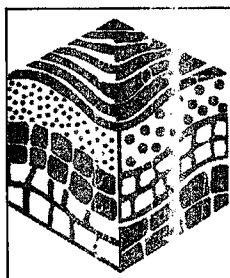
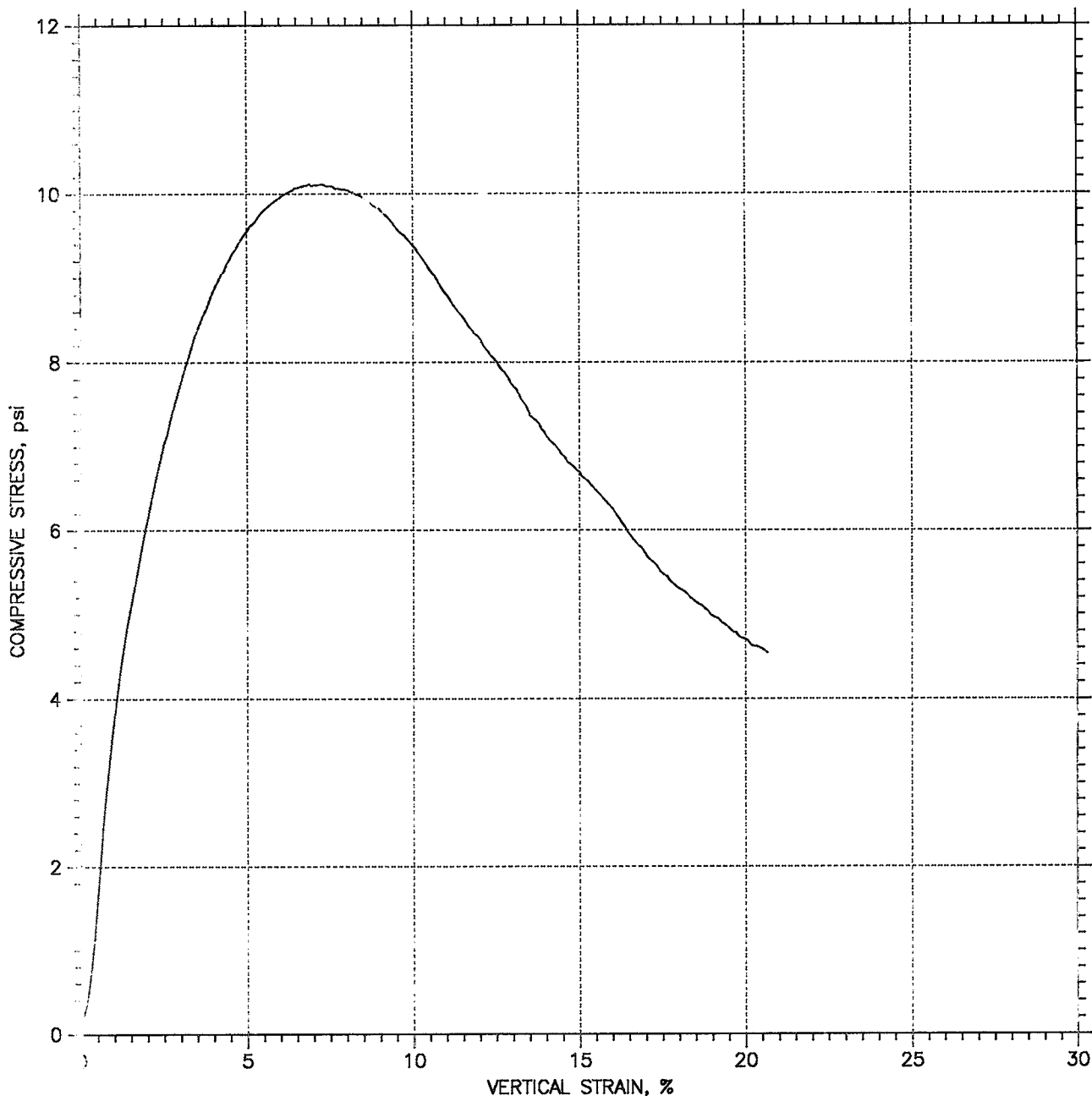
### UNCONFINED COMPRESSION TEST REPORT



Symbol				
Test No.		UC-22-16		
Initial	Diameter, in	2.835		
	Height, in	5.737		
	Water Content, %	22.71		
	Dry Density, pcf	103.6		
	Saturation, %	100.72		
	Void Ratio	0.598		
Unconfined Compressive Strength, psi		10.11		
Undrained Shear Strength, psi		5.056		
Time to Failure, min		7.0849		
Strain Rate, %/min		1		
Measured Specific Gravity		2.65		
Liquid Limit		0		
Plastic Limit		0		
Plasticity Index		0		
Failure Sketch				

	Project: IM-2-094(143)260
	Location:
	Project No.:
	Boring No.: 2
	Sample Type: Undisturbed
	Description: T. L. 14 1/2" Brn Cly, rock up to 3/4"
Remarks:	

# UNCONFINED COMPRESSION TEST REPORT



Project: IM-2-094(143)260	Location:	Project No.:
Boring No.: 2	Tested By: DT	Checked By: MD
Sample No.: SS-381-16	Test Date: 6/6/2016	Depth: 9.3-9.8
Test No.: UC-22-16	Sample Type: Undisturbed	Elevation:
Description: T. L. 14 1/2" Brn Cly, rock up to 3/4"		
Remarks:		

## UNCONFINED COMPRESSION TEST

SS-381-16

Project: IM-2-094(143)260  
 Boring No.: 2  
 Sample No.: SS-381-16  
 Test No.: UC-22-16

Location:  
 Tested By: DT  
 Test Date: 6/6/2016  
 Sample Type: Undisturbed

Project No.:  
 Checked By: MD  
 Depth: 9.3-9.8  
 Elevation:

Soil Description: T. L. 14 1/2" Brn Cly, rock up to 3/4"  
 Remarks:

Specimen Height: 5.74 in  
 Specimen Area: 6.31 in<sup>2</sup>  
 Specimen Volume: 593.45 cc

Liquid Limit: 0  
 Plastic Limit: 0  
 Measured Specific Gravity: 2.65

Cap Mass: 0 gm

Water Content Information  
 -----

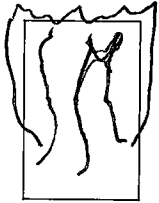
Container ID	S 65
Wt. Container, gm	15.33
Wt. Container + Wet Soil, gm	93.4
Wt. Container + Dry Soil, gm	78.95
Wt. Dry Soil, gm	63.62
Water Content, %	22.71
Void Ratio	0.60
Degree of Saturation, %	100.72
Wet Unit Weight, pcf	127.07
Dry Unit Weight, pcf	103.55

# UNCONFINED COMPRESSION

North Dakota Department of Transportation, Materials & Research  
SFN 50460 (5-2016)

Project Number	IM-2-094(143)260
Boring Number	2

Field Sample Number 55-381-16		Lab Number UC-22-16		Depth 9.3 to 9.8	
Weight of Sample 1207.96		Test Number 1 of 1		AASHTO T-208 Tested by: DT/MD	
Diameter	1.	4.	Height	1.	Moisture Can Number 565
	2.	5.		2.	
	3.	6.		3.	
	Average 2.835			Average 5.737	
Wet Wt + Can 93.40		Dry Wt + Can 78.95		Wt of Can 15.33	



Total Length: 14 1/2" BRN Cly, Rock up to 3/4"



Field Sample Number		Lab Number		Depth	
Weight of Sample		Test Number of		AASHTO T-208 Tested by:	
Diameter	1.	4.	Height	1.	Moisture Can Number
	2.	5.		2.	
	3.	6.		3.	
	Average			Average	
Wet Wt + Can		Dry Wt + Can		Wt of Can	

Total Length:

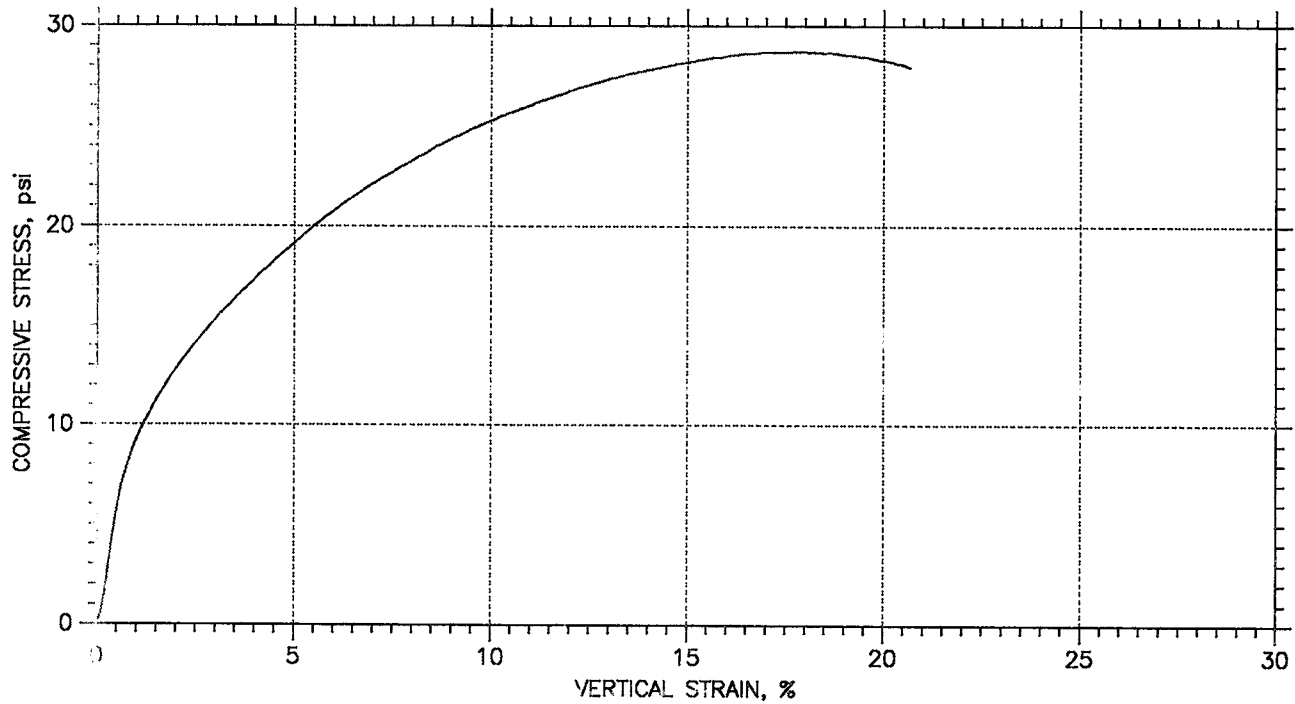


Field Sample Number		Lab Number		Depth	
Weight of Sample		Test Number of		AASHTO T-208 Tested by:	
Diameter	1.	4.	Height	1.	Moisture Can Number
	2.	5.		2.	
	3.	6.		3.	
	Average			Average	
Wet Wt + Can		Dry Wt + Can		Wt of Can	

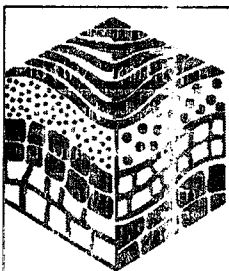
Total Length:



### UNCONFINED COMPRESSION TEST REPORT



Symbol				
Test No.		UC-21-16		
Initial	Diameter, in	2.829		
	Height, in	5.733		
	Water Content, %	26.14		
	Dry Density, pcf	100.3		
	Saturation, %	106.65		
	Void Ratio	0.649		
Unconfined Compressive Strength, psi		28.72		
Undrained Shear Strength, psi		14.36		
Time to Failure, min		17.334		
Strain Rate, %/min		1		
Measured Specific Gravity		2.65		
Liquid Limit		0		
Plastic Limit		0		
Plasticity Index		0		
Failure Sketch				



Project: IM-2-094(143)260
Location:
Project No.:
Boring No.: 2
Sample Type: Undisturbed
Description: T. L. 14 3/4" Brn Blk Cly, rock up to 1"
Remarks:

## UNCONFINED COMPRESSION TEST

SS-379-16

Project: IM-2-094(143)260  
 Boring No.: 2  
 Sample No.: SS-379-16  
 Test No.: UC-21-16

Location:  
 Tested By: DT  
 Test Date: 6/6/2016  
 Sample Type: Undisturbed

Project No.:  
 Checked By: MD  
 Depth: 4.6-5.1  
 Elevation:

Soil Description: T. L. 14 3/4" Brn Blk Cly, rock up to 1"  
 Remarks:

Specimen Height: 5.73 in  
 Specimen Area: 6.29 in<sup>2</sup>  
 Specimen Volume: 590.53 cc

Liquid Limit: 0  
 Plastic Limit: 0  
 Measured Specific Gravity: 2.65

Cap Mass: 0 gm

Water Content Information  
 -----

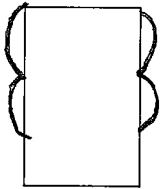
Container ID	S 56
Wt. Container, gm	17.09
Wt. Container + Wet Soil, gm	74.28
Wt. Container + Dry Soil, gm	62.43
Wt. Dry Soil, gm	45.34
Water Content, %	26.14
Void Ratio	0.65
Degree of Saturation, %	106.65
Wet Unit Weight, pcf	126.51
Dry Unit Weight, pcf	100.3

# UNCONFINED COMPRESSION

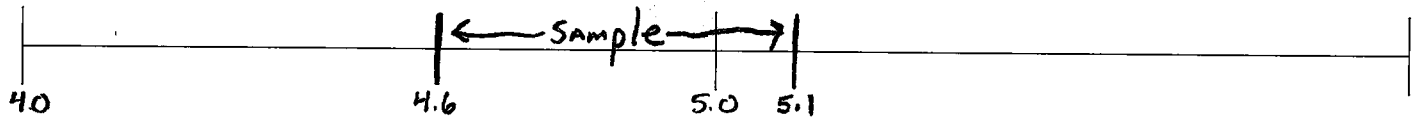
North Dakota Department of Transportation, Materials & Research  
 SFN 50460 (5-2016)

Project Number  
 IM-2-094(143) 260  
 Boring Number  
 2

Field Sample Number 55-379-16			Lab Number UC-21-16			Depth 4.6 to 5.1		
Weight of Sample 1196.72			Test Number 1 of 1			AASHTO T-208 Tested by: OT/MD		
Diameter	1.	4.	Height	1.	Moisture Can Number 556	Wet Wt + Can 74.28		
	2.	5.		2.		Dry Wt + Can 62.43		
	3.	6.		3.		Wt of Can 17.09		
	Average 2.829			Average 5.733				



Total Length: 14 3/4" BAN BIK Clay with Rock up to 1"



Field Sample Number			Lab Number			Depth		
Weight of Sample			Test Number of			AASHTO T-208 Tested by:		
Diameter	1.	4.	Height	1.	Moisture Can Number	Wet Wt + Can		
	2.	5.		2.		Dry Wt + Can		
	3.	6.		3.		Wt of Can		
	Average			Average				

Total Length:

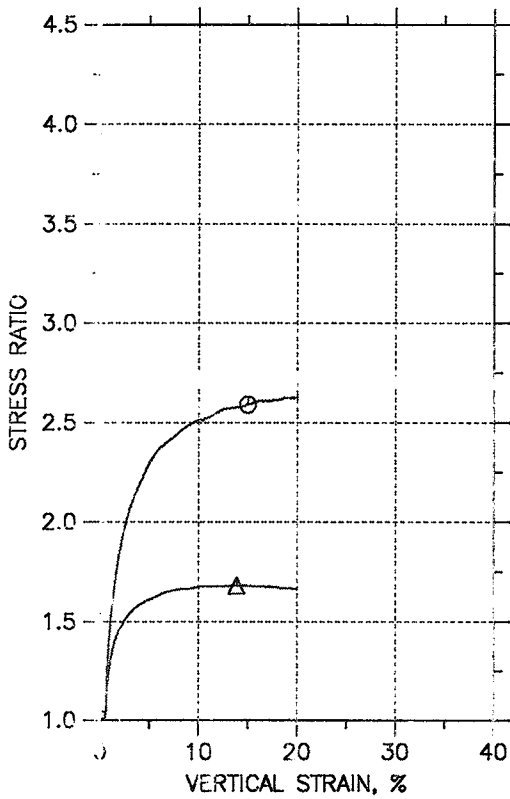
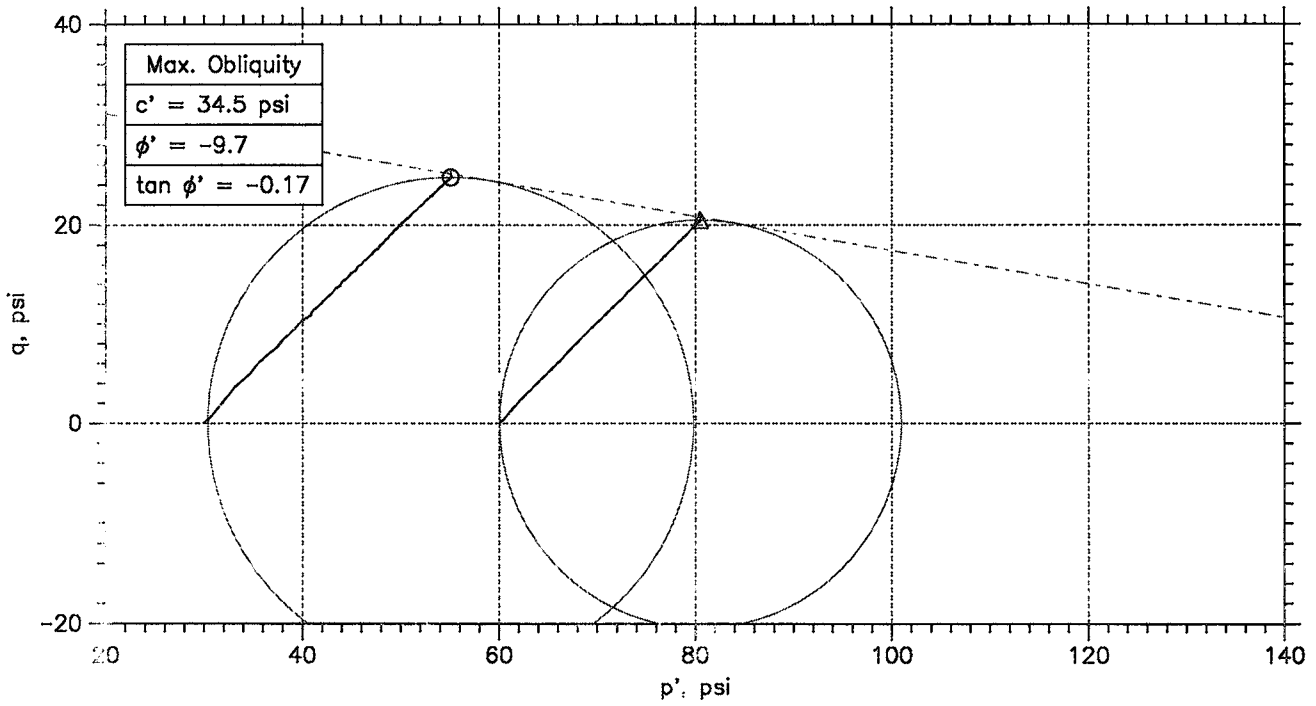


Field Sample Number			Lab Number			Depth		
Weight of Sample			Test Number of			AASHTO T-208 Tested by:		
Diameter	1.	4.	Height	1.	Moisture Can Number	Wet Wt + Can		
	2.	5.		2.		Dry Wt + Can		
	3.	6.		3.		Wt of Can		
	Average			Average				

Total Length:



Test File



Symbol	○	△	
Sample No.	SS-351-16	SS-351-16	
Test No.	UU-35-16	UU-36-16	
Depth	34.1-34.6	34.6-35.1	
Initial	Diameter, in	2.862	2.865
	Height, in	5.747	5.752
	Water Content, %	16.4	15.3
	Dry Density, pcf	109.5	113.2
	Saturation, %	84.8	87.8
Before Shear	Void Ratio	0.511	0.462
	Water Content, %	16.3	15.1
	Dry Density, pcf	109.5	113.4
	Saturation*, %	84.7	87.0
	Void Ratio	0.511	0.459
	Back Press., psi	.0	.0
	Ver. Eff. Cons. Stress, psi	29.99	59.97
	Shear Strength, psi	24.13	20.46
	Strain at Failure, %	15	13.9
	Strain Rate, %/min	1	1
	B-value	---	---
	Estimated Specific Gravity	2.65	2.65
	Liquid Limit	---	---
	Plastic Limit	---	---

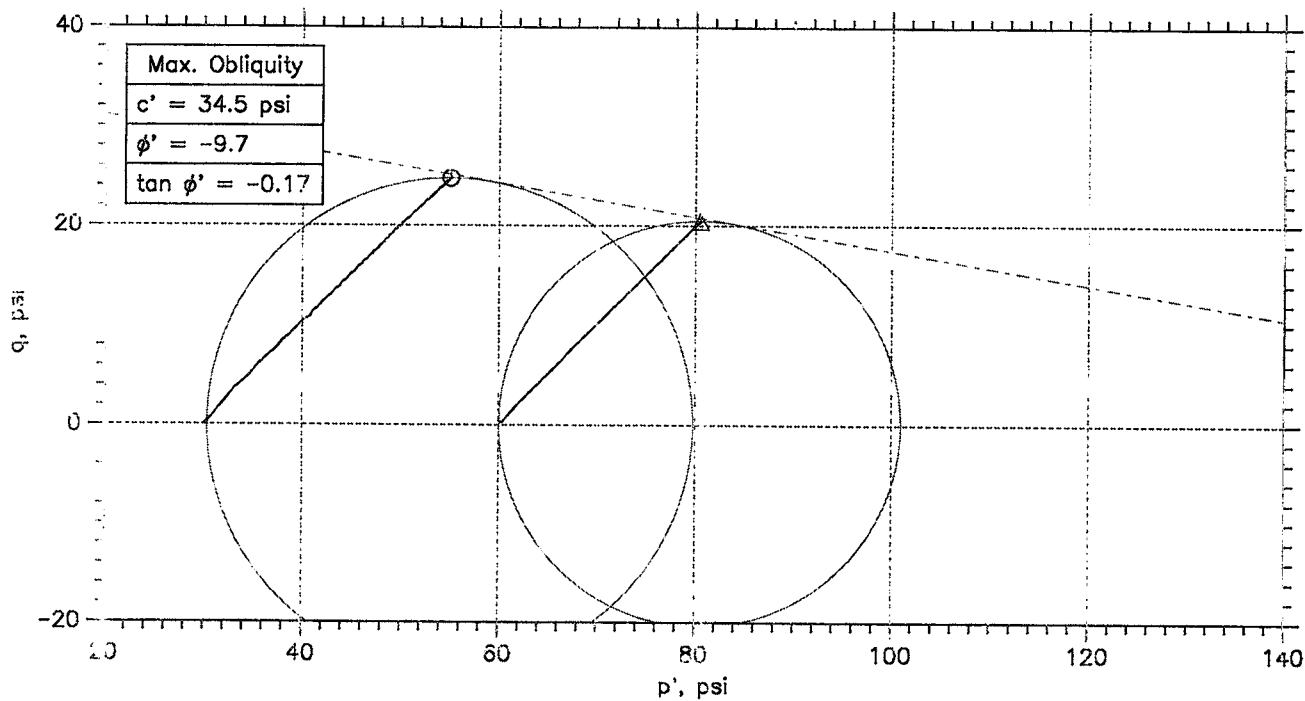
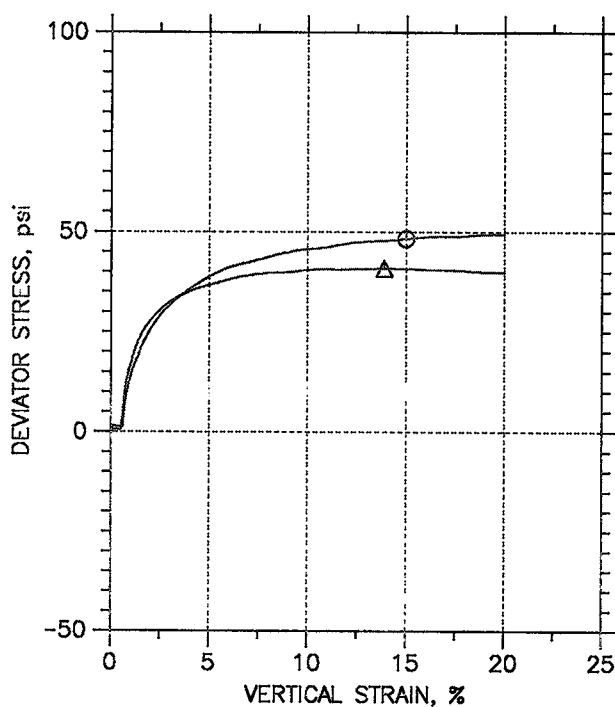
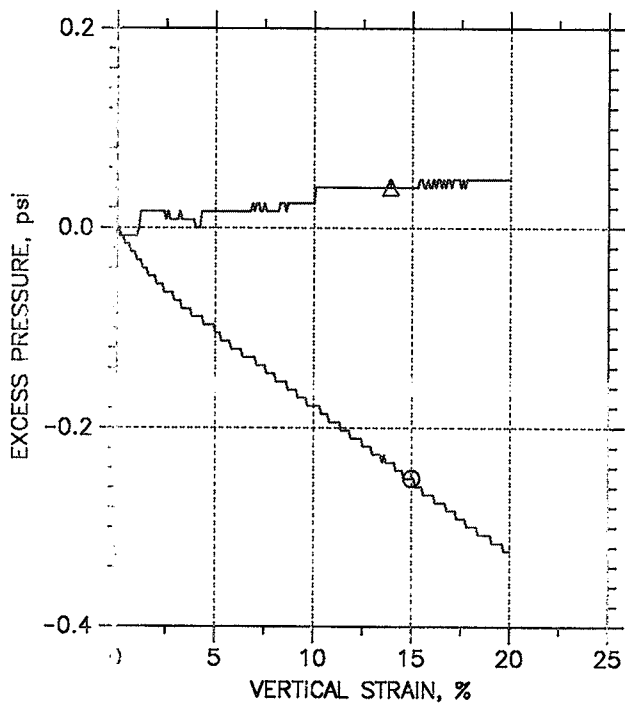
	Project: IM-2-094(143)260	
	Location:	
	Project No.:	
	Boring No.: 1	
	Sample Type:	
	Description: T.L. 24" Brn Clay with I.O. deposits, Rock upto 1/2".	
Remarks:		

Phase calculations based on start and end of test.

\* Saturation is set to 100% for phase calculations.



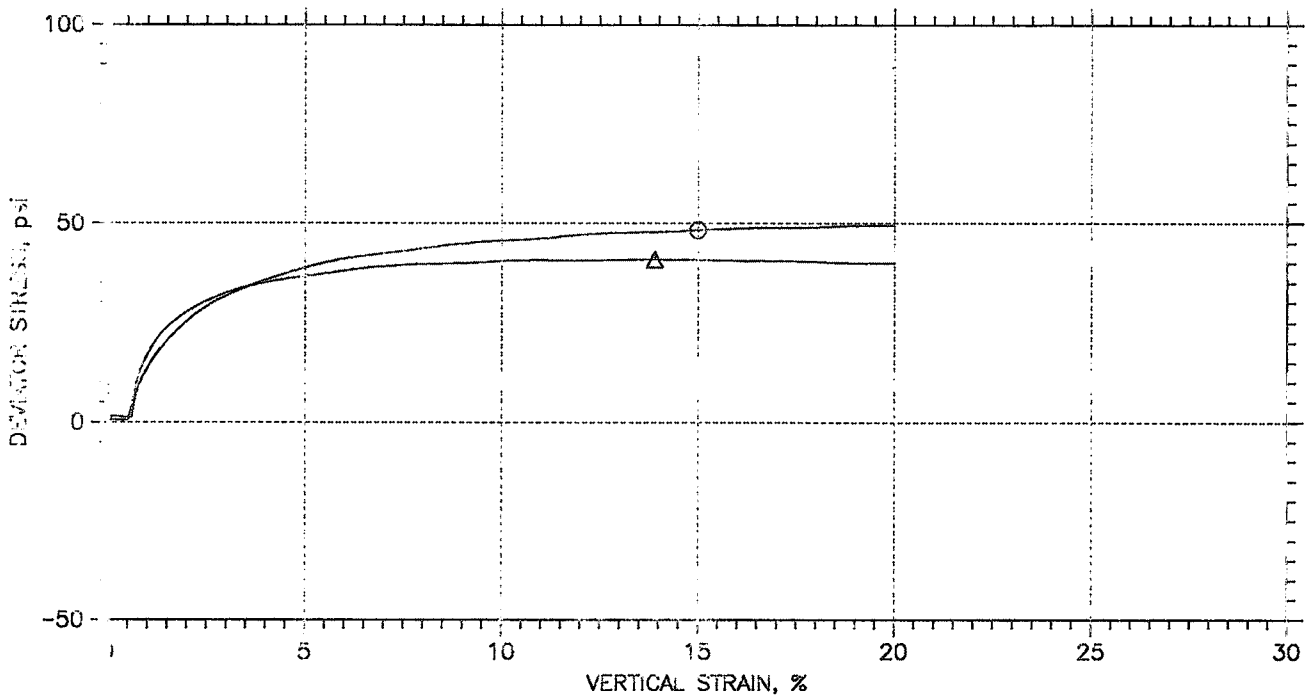
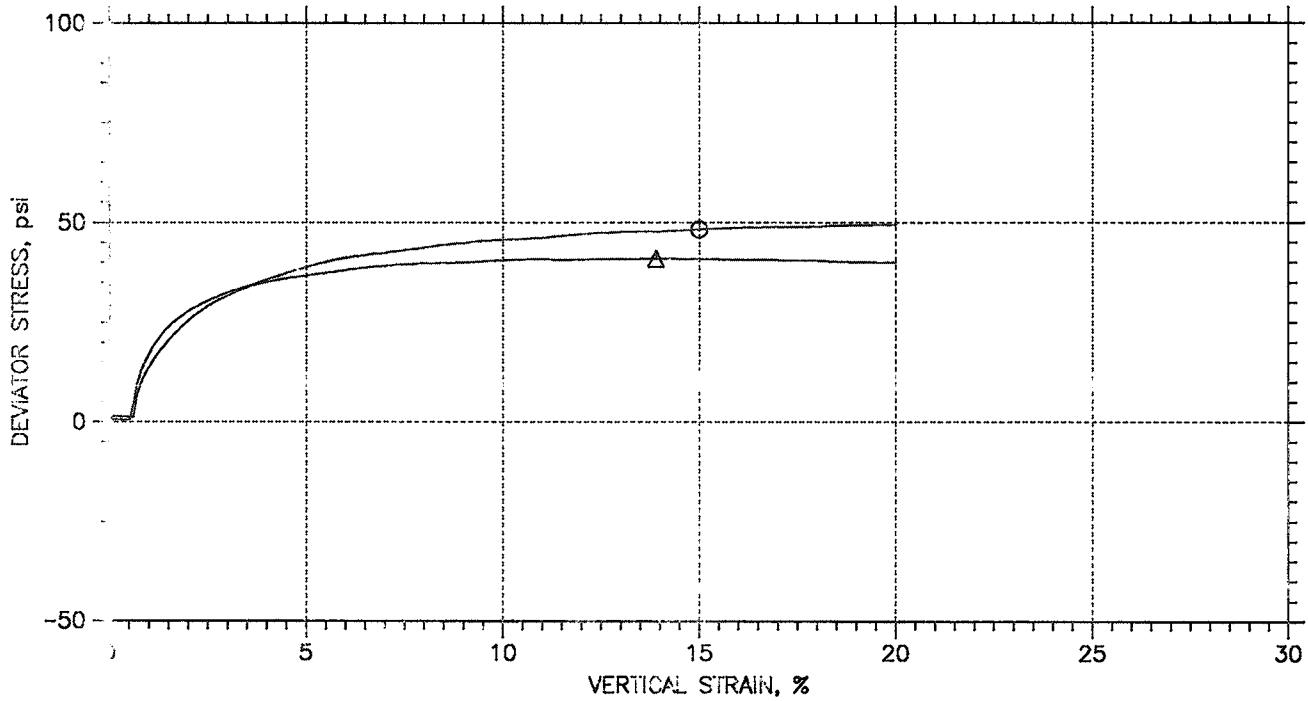
Test File



Sample No.	Test No.	Depth	Tested By	Test Date	Checked By	Check Date	Test File
⊙ SS-351-16	UU-35-16	34.1-34.6	DT	5/26/16	MD		UU-35-2016.dat
△ SS-351-16	UU-36-16	34.6-35.1	DT	5/26/16	MD		UU-36-2016.dat

	Project: IM-2-094(143)260	Location:	Project No.:
	Boring No.: 1	Sample Type:	
	Description: T.L. 24" Brn Clay with I.O. deposits, Rock upto 1/2".		
	Remarks:		

Test File



Symbol	Sample No.	Test No.	Depth	Tested By	Test Date	Checked By	Check Date	Test File
○	SS-351-16	UU-35-16	34.1-34.6	DT	5/26/16	MD		UU-35-2016.dat
△	SS-351-16	UU-36-16	34.6-35.1	DT	5/26/16	MD		UU-36-2016.dat

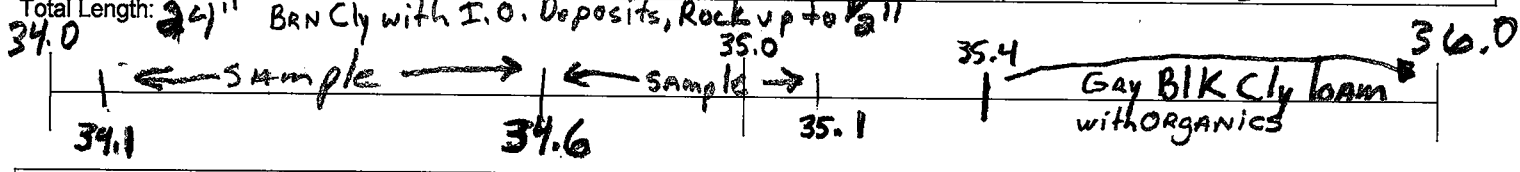
	Project: 11M-2-094(143)260	Location:	Project No.:
	Boring No.: 1	Sample Type:	
	Description: T.L. 24" Brn Clay with I.O. deposits, Rock upto 1/2".		
	Remarks:		

**TRIAxIAL UU(Q) CCU(R) CD(S)**

North Dakota Department of Transportation, Materials & Research  
 SFN 50459 (5-2016)

Project Number  
**EM-2-094(143)260**  
 Boring Number  
**1**

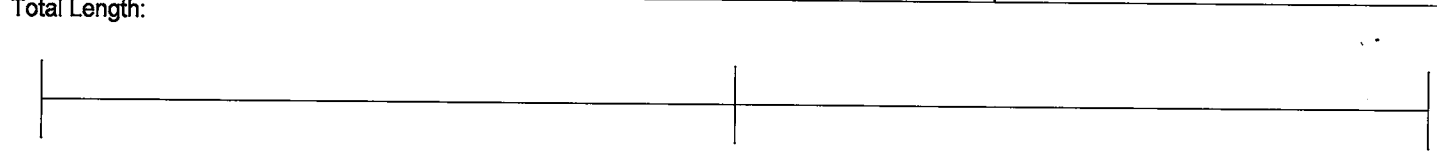
Field Sample Number <b>SS-351-16</b>		Lab Number <b>UU-35-16</b>		Depth <b>34.1 to 34.6</b>	
Weight of Sample <b>1236.02</b>		Confining Pressure <b>30.00</b>		Test Number <b>1</b> of <b>2</b>	
Diameter	<b>2.850</b>	<b>2.858</b>	Height	<b>5.744</b>	
	<b>2.859</b>	<b>2.869</b>		<b>5.748</b>	
	<b>2.863</b>	<b>2.871</b>		<b>5.749</b>	
	Average <b>2.862</b>			Average <b>5.747</b>	
Moisture Can Number <b>S49</b>		Wet Wt + Can <b>74.94</b>		Dry Wt + Can <b>66.56</b>	
Wt of Can <b>15.33</b>					



Field Sample Number <b>SS-351-16</b>		Lab Number <b>UU-36-16</b>		Depth <b>34.6 to 35.1</b>	
Weight of Sample <b>1270.25</b>		Confining Pressure <b>60.00</b>		Test Number <b>2</b> of <b>2</b>	
Diameter	<b>2.859</b>	<b>2.863</b>	Height	<b>5.756</b>	
	<b>2.872</b>	<b>2.866</b>		<b>5.748</b>	
	<b>2.869</b>	<b>2.863</b>		<b>5.752</b>	
	Average <b>2.865</b>			Average <b>5.752</b>	
Moisture Can Number <b>S21</b>		Wet Wt + Can <b>62.29</b>		Dry Wt + Can <b>56.28</b>	
Wt of Can <b>16.99</b>					



Field Sample Number		Lab Number		Depth	
Weight of Sample		Confining Pressure		Test Number of	
Diameter			Height		
	Average			Average	
Moisture Can Number		Wet Wt + Can		Dry Wt + Can	
Wt of Can					



99-351-16

TRIAXIAL TEST

Project: IM-2-094(143)260  
 Boring No.: 1  
 Sample No.: SS-351-16  
 Test No.: UU-35-16

Location:  
 Tested By: DT  
 Test Date: 5/26/16  
 Sample Type:

Project No.:  
 Checked By: MD  
 Depth: 34.1-34.6  
 Elevation:

Soil Description: T.L. 24" Brn Clay with I.O. deposits, Rock upto 1/2".  
 Remarks:

Specimen Height: 5.75 in  
 Specimen Area: 6.43 in<sup>2</sup>  
 Specimen Volume: 605.86 cc

Piston Area: 0.16 in<sup>2</sup>  
 Piston Friction: 0.00 lb  
 Piston Weight: 0.00 lb

Filter Strip Correction: 0.00 psi  
 Membrane Correction: 4.20 lb/in  
 Correction Type: Uniform

Liquid Limit: ---

Elastic Limit: ---

Estimated Specific Gravity: 2.65

	Before Test Trimmings	Before Test Specimen	After Test Specimen	After Test Trimmings
Container ID	s49	---		
Wt. Container + Wet Soil, gm	74.94	---	---	0
Wt. Container + Dry Soil, gm	66.56	---	---	0
Wt. Container, gm	15.33	---	---	0
Wt. Wet Soil, gm	59.61	1236	1062.3	0
Wt. Dry Soil, gm	51.23	1062.3	1062.3	0
Wt. Water, gm	8.38	173.76	0	0
Water Content, %	16.36	16.36	0.00	0.00
Void Ratio	---	0.51	0.51	---
Degree of Saturation, %	---	84.76	0.00	---
Dry Unit Weight, pcf	---	109.46	109.47	---

Initial  
 Height: 5.747 in  
 Area: 6.4332 in<sup>2</sup>  
 Volume: 605.86 cc  
 Moisture: 16.36 %  
 Void Ratio: 0.51  
 Dry Unit Weight: 109.46 pcf  
 Saturation: 84.76 %

End of Initialization  
 Time: 11.072 min  
 Total Vertical Stress: 29.993 psi  
 Total Horizontal Stress: 29.982 psi  
 Pore Pressure: 0 psi  
 Effective Vertical Stress: 29.993 psi  
 Effective Horizontal Stress: 29.982 psi  
 Height Change: 0.00018489 in  
 Area Change: 0 in<sup>2</sup>  
 Volume Change: 0.058474 cc  
 Water Change: 0.16679 cc  
 Correction: 0 cc  
 Height: 5.7468 in  
 Area: 6.4332 in<sup>2</sup>  
 Volume: 605.8 cc  
 Moisture: 16.34 %  
 Void Ratio: 0.51  
 Dry Unit Weight: 109.47 pcf  
 Saturation: 84.70 %

End of Consolidation/A  
 Time: 11.072 min  
 Total Vertical Stress: 29.993 psi  
 Total Horizontal Stress: 29.982 psi  
 Pore Pressure: 0 psi  
 Effective Vertical Stress: 29.993 psi  
 Effective Horizontal Stress: 29.982 psi  
 Height Change: 0.00018489 in  
 Area Change: 0 in<sup>2</sup>  
 Volume Change: 0.058474 cc  
 Water Change: 0.16679 cc  
 Correction: 0 cc  
 Height: 5.7468 in  
 Area: 6.4332 in<sup>2</sup>  
 Volume: 605.8 cc  
 Moisture: 16.34 %  
 Void Ratio: 0.51  
 Dry Unit Weight: 109.47 pcf  
 Saturation: 84.70 %

End of Saturation  
 Time: 11.072 min  
 Total Vertical Stress: 29.993 psi  
 Total Horizontal Stress: 29.982 psi  
 Pore Pressure: 0 psi  
 Effective Vertical Stress: 29.993 psi  
 Effective Horizontal Stress: 29.982 psi  
 Height Change: 0.00018489 in  
 Area Change: 0 in<sup>2</sup>  
 Volume Change: 0.058474 cc  
 Water Change: 0.16679 cc  
 Correction: 0 cc  
 Height: 5.7468 in  
 Area: 6.4332 in<sup>2</sup>  
 Volume: 605.8 cc  
 Moisture: 16.34 %  
 Void Ratio: 0.51  
 Dry Unit Weight: 109.47 pcf  
 Saturation: 84.70 %

End of Consolidation/B  
 Time: 11.072 min  
 Total Vertical Stress: 29.993 psi  
 Total Horizontal Stress: 29.982 psi  
 Pore Pressure: 0 psi  
 Effective Vertical Stress: 29.993 psi  
 Effective Horizontal Stress: 29.982 psi  
 Height Change: 0.00018489 in  
 Area Change: 0 in<sup>2</sup>  
 Volume Change: 0.058474 cc  
 Water Change: 0.16679 cc  
 Correction: 0 cc  
 Height: 5.7468 in  
 Area: 6.4332 in<sup>2</sup>  
 Volume: 605.8 cc  
 Moisture: 16.34 %  
 Void Ratio: 0.51  
 Dry Unit Weight: 109.47 pcf  
 Saturation: 84.70 %

End of Shear  
 Time: 31.422 min  
 Total Vertical Stress: 79.485 psi  
 Total Horizontal Stress: 30.047 psi  
 Pore Pressure: -0.02379 psi  
 Effective Vertical Stress: 79.809 psi  
 Effective Horizontal Stress: 30.371 psi  
 Height Change: 1.1498 in  
 Area Change: -1.6082 in<sup>2</sup>  
 Volume Change: 0.058474 cc  
 Water Change: 0.16679 cc  
 Correction: 173.59 cc  
 Height: 4.5972 in  
 Area: 3.0414 in<sup>2</sup>  
 Volume: 605.8 cc  
 Moisture: 0.00 %  
 Void Ratio: 0.51  
 Dry Unit Weight: 109.47 pcf  
 Saturation: 0.00 %

At Failure  
 Time: 26.834 min  
 Total Vertical Stress: 78.354 psi  
 Total Horizontal Stress: 30.095 psi  
 Pore Pressure: -0.05093 psi  
 Effective Vertical Stress: 78.605 psi  
 Effective Horizontal Stress: 30.346 psi  
 Height Change: 0.86221 in  
 Area Change: -1.1334 in<sup>2</sup>  
 Volume Change: 0.058474 cc  
 Water Change: 0.16679 cc  
 Correction: 0 cc  
 Height: 4.8848 in  
 Area: 7.5667 in<sup>2</sup>  
 Volume: 605.8 cc  
 Moisture: 16.34 %  
 Void Ratio: 0.51  
 Dry Unit Weight: 109.47 pcf  
 Saturation: 84.70 %

SS-351-16

TRIAXIAL TEST

Project: EM-2-094 143)260  
 Boring No.: 1  
 Sample No.: SS-35-16  
 Test No.: UU-36-15

Location:  
 Tested By: DT  
 Test Date: 5/26/16  
 Sample Type:

Project No.:  
 Checked By: MD  
 Depth: 34.6-35.1  
 Elevation:

Soil Description: T.L. 24" Brn Clay with I.O. deposits, Rock upto 1/2"  
 Remarks:

Specimen Height: 0.75 in  
 Specimen Area: 6.15 in<sup>2</sup>  
 Specimen Volume: 607.66 cc

Piston Area: 0.16 in<sup>2</sup>  
 Piston Friction: 0.00 lb  
 Piston Weight: 0.00 lb

Filter Strip Correction: 0.00 psi  
 Membrane Correction: 4.20 lb/in  
 Correction Type: Uniform

Liquid Limit: ---

Plastic Limit: ---

Estimated Specific Gravity: 2.65

	Before Test Trimmings	Before Test Specimen	After Test Specimen	After Test Trimmings
Container ID	s21	---		
Wt. Container + Wet Soil, gm	62.29	---	---	0
Wt. Container + Dry Soil, gm	56.28	---	---	0
Wt. Container, gm	16.99	---	---	0
Wt. Wet Soil, gm	45.3	1270.3	1101.7	0
Wt. Dry Soil, gm	39.29	1101.7	1101.7	0
Wt. Water, gm	6.01	168.53	0	0
Water Content, %	15.30	15.30	0.00	0.00
Void Ratio	---	0.46	0.46	---
Degree of Saturation, %	---	87.81	0.00	---
Dry Unit Weight, pcf	---	113.19	113.39	---

Initial  
 Height: 5.752 in  
 Area: 6.4467 in<sup>2</sup>  
 Volume: 607.66 cc  
 Moisture: 15.30 %  
 Void Ratio: 0.46  
 Dry Unit Weight: 113.19 pcf  
 Saturation: 87.81 %

End of Initialization  
 Time: 11:07 min  
 Total Vertical Stress: 59.966 psi  
 Total Horizontal Stress: 59.973 psi  
 Pore Pressure: 0 psi  
 Effective Vertical Stress: 59.966 psi  
 Effective Horizontal Stress: 59.973 psi  
 Height Change: 0.0035129 in  
 Area Change: 0 in<sup>2</sup>  
 Volume Change: 1.1133 cc  
 Water Change: 2.5281 cc  
 Correction: 0 cc  
 Height: 5.7485 in  
 Area: 6.4467 in<sup>2</sup>  
 Volume: 606.54 cc  
 Moisture: 15.07 %  
 Void Ratio: 0.46  
 Dry Unit Weight: 113.39 pcf  
 Saturation: 87.00 %

End of Consolidation/A  
 Time: 11:07 min  
 Total Vertical Stress: 59.966 psi  
 Total Horizontal Stress: 59.973 psi  
 Pore Pressure: 0 psi  
 Effective Vertical Stress: 59.966 psi  
 Effective Horizontal Stress: 59.973 psi  
 Height Change: 0.0035129 in  
 Area Change: 0 in<sup>2</sup>  
 Volume Change: 1.1133 cc  
 Water Change: 2.5281 cc  
 Correction: 0 cc  
 Height: 5.7485 in  
 Area: 6.4467 in<sup>2</sup>  
 Volume: 606.54 cc  
 Moisture: 15.07 %  
 Void Ratio: 0.46  
 Dry Unit Weight: 113.39 pcf  
 Saturation: 87.00 %

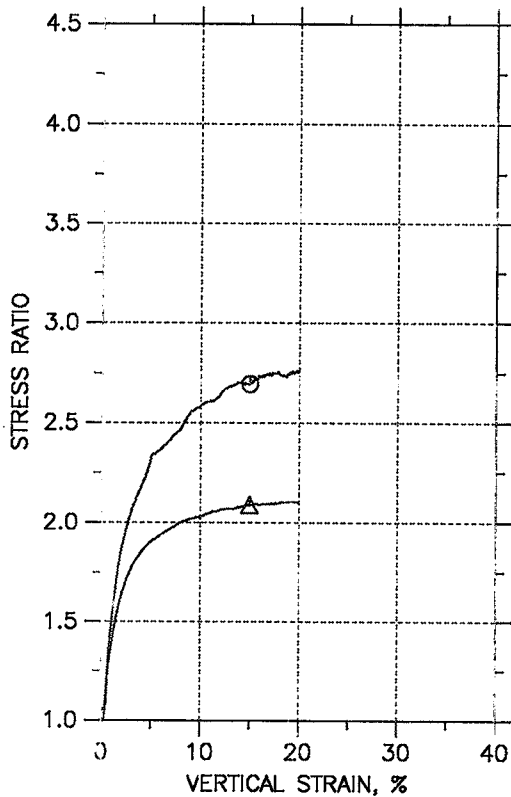
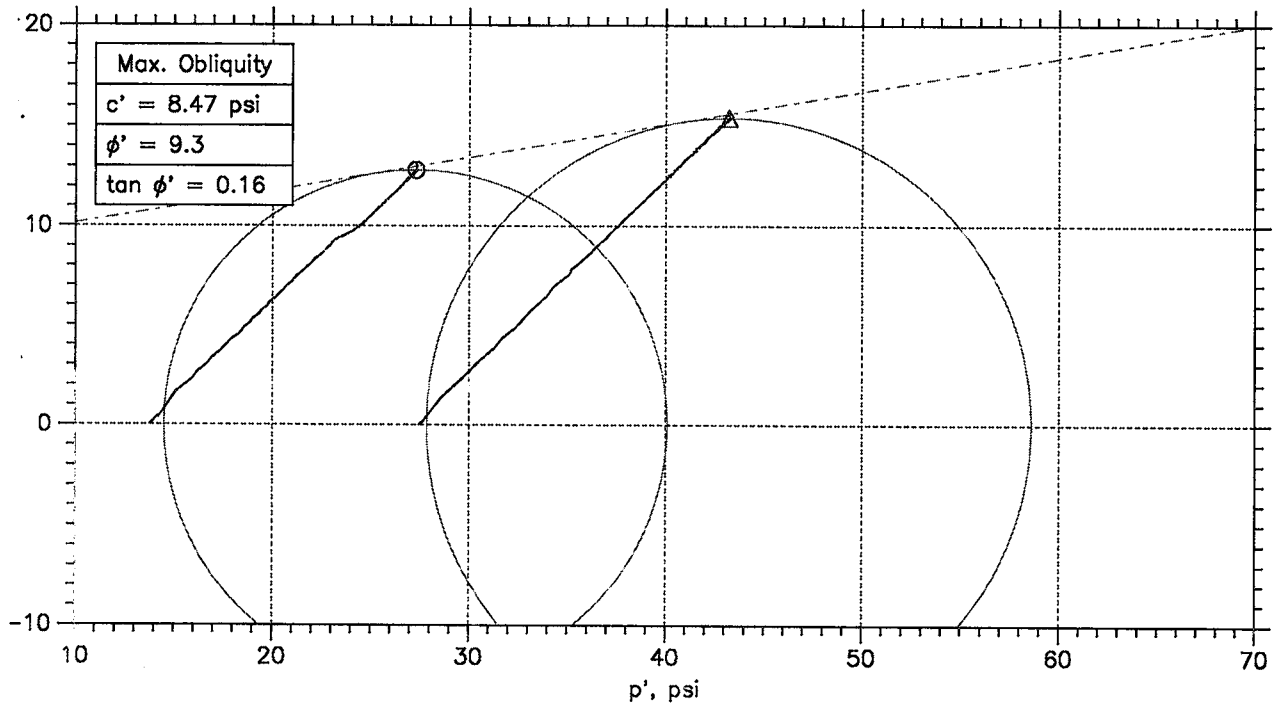
End of Saturation  
 Time: 11:07 min  
 Total Vertical Stress: 59.966 psi  
 Total Horizontal Stress: 59.973 psi  
 Pore Pressure: 0 psi  
 Effective Vertical Stress: 59.966 psi  
 Effective Horizontal Stress: 59.973 psi  
 Height Change: 0.0035129 in  
 Area Change: 0 in<sup>2</sup>  
 Volume Change: 1.1133 cc  
 Water Change: 2.5281 cc  
 Correction: 0 cc  
 Height: 5.7485 in  
 Area: 6.4467 in<sup>2</sup>  
 Volume: 606.54 cc  
 Moisture: 15.07 %  
 Void Ratio: 0.46  
 Dry Unit Weight: 113.39 pcf  
 Saturation: 87.00 %

End of Consolidation/B  
 Time: 11:07 min  
 Total Vertical Stress: 59.966 psi  
 Total Horizontal Stress: 59.973 psi  
 Pore Pressure: 0 psi  
 Effective Vertical Stress: 59.966 psi  
 Effective Horizontal Stress: 59.973 psi  
 Height Change: 0.0035129 in  
 Area Change: 0 in<sup>2</sup>  
 Volume Change: 1.1133 cc  
 Water Change: 2.5281 cc  
 Correction: 0 cc  
 Height: 5.7485 in  
 Area: 6.4467 in<sup>2</sup>  
 Volume: 606.54 cc  
 Moisture: 15.07 %  
 Void Ratio: 0.46  
 Dry Unit Weight: 113.39 pcf  
 Saturation: 87.00 %

End of Shear  
 Time: 31:44 min  
 Total Vertical Stress: 99.848 psi  
 Total Horizontal Stress: 60.022 psi  
 Pore Pressure: 3.48568 psi  
 Effective Vertical Stress: 99.8 psi  
 Effective Horizontal Stress: 59.973 psi  
 Height Change: 1.1533 in  
 Area Change: -1.602 in<sup>2</sup>  
 Volume Change: 1.1133 cc  
 Water Change: 2.5281 cc  
 Correction: 166 cc  
 Height: 4.5987 in  
 Area: 8.0487 in<sup>2</sup>  
 Volume: 606.54 cc  
 Moisture: 0.00 %  
 Void Ratio: 0.46  
 Dry Unit Weight: 113.39 pcf  
 Saturation: 0.00 %

At Failure  
 Time: 25:35 min  
 Total Vertical Stress: 101.01 psi  
 Total Horizontal Stress: 60.094 psi  
 Pore Pressure: 3.40473 psi  
 Effective Vertical Stress: 100.97 psi  
 Effective Horizontal Stress: 60.054 psi  
 Height Change: 0.80256 in  
 Area Change: -1.0142 in<sup>2</sup>  
 Volume Change: 1.1133 cc  
 Water Change: 2.5281 cc  
 Correction: 0 cc  
 Height: 4.9494 in  
 Area: 7.4609 in<sup>2</sup>  
 Volume: 606.54 cc  
 Moisture: 15.07 %  
 Void Ratio: 0.46  
 Dry Unit Weight: 113.39 pcf  
 Saturation: 87.00 %

Test File



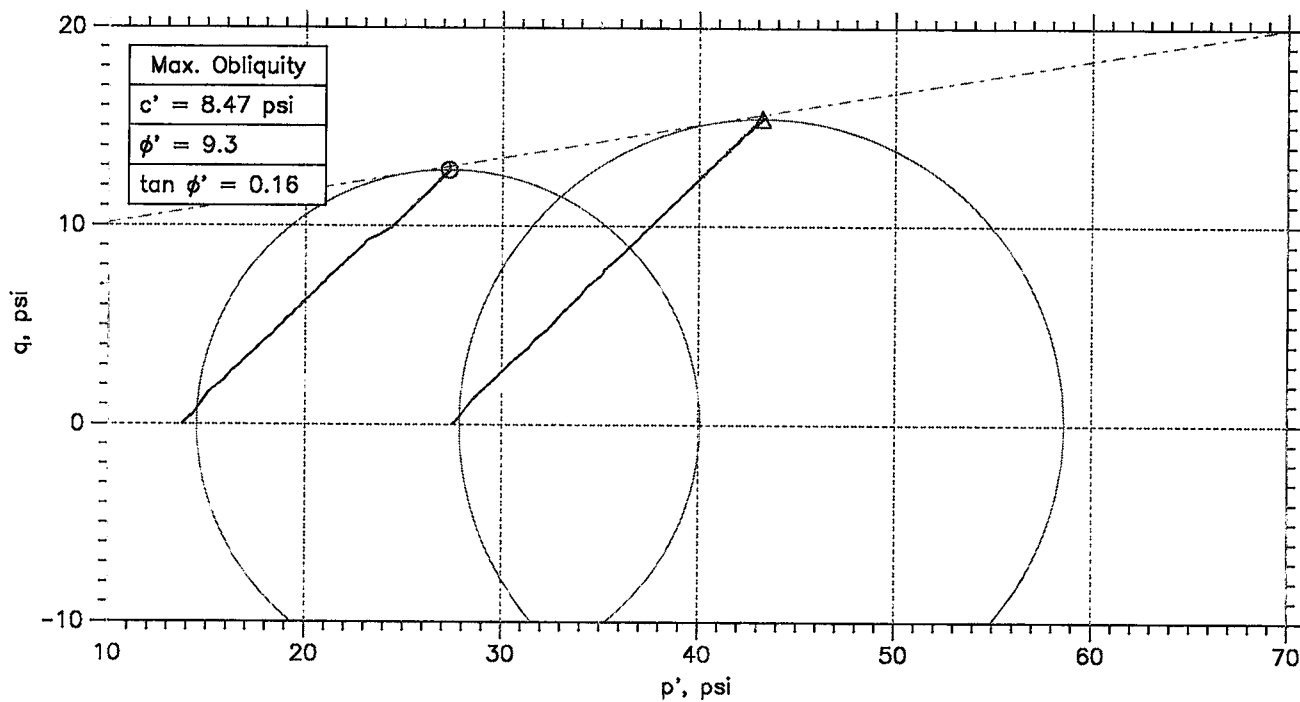
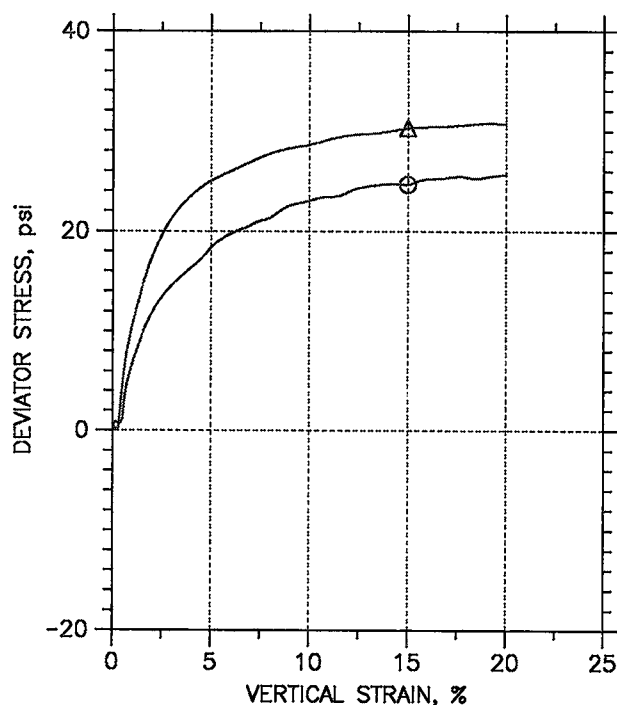
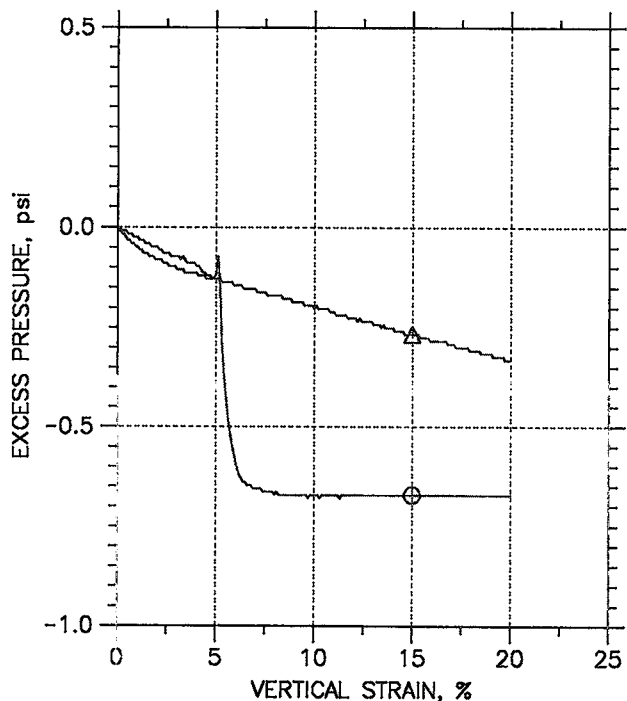
Symbol	○	△		
Sample No.	SS-349-16	SS-349-16		
Test No.	UU-37-16	UU-38-16		
Depth	29.2-29.7	29.8-30.3		
Initial	Diameter, in	2.837	2.851	
	Height, in	5.748	5.737	
	Water Content, %	20.3	21.1	
	Dry Density, pcf	106.3	106.4	
	Saturation, %	96.7	100.9	
	Void Ratio	0.556	0.555	
Before Shear	Water Content, %	20.3	20.8	
	Dry Density, pcf	106.3	106.7	
	Saturation*, %	96.8	100.0	
	Void Ratio	0.556	0.55	
Back Press., psi	.0	.0		
Ver. Eff. Cons. Stress, psi	13.72	27.39		
Shear Strength, psi	12.32	15.14		
Strain at Failure, %	15	15		
Strain Rate, %/min	1	1		
B-Value	---	---		
Estimated Specific Gravity	2.65	2.65		
Liquid Limit	---	---		
Plastic Limit	---	---		

	Project: IM-2-094(143)260	
	Location:	
	Project No.:	
	Boring No.: 1	
	Sample Type:	
	Description: T.L. 20" Brn Silty Clay with I.O. & Coal deposits, Rock up to 1/2"	
Remarks:		

Phase calculations based on start and end of test.

\* Saturation is set to 100% for phase calculations.

Test File

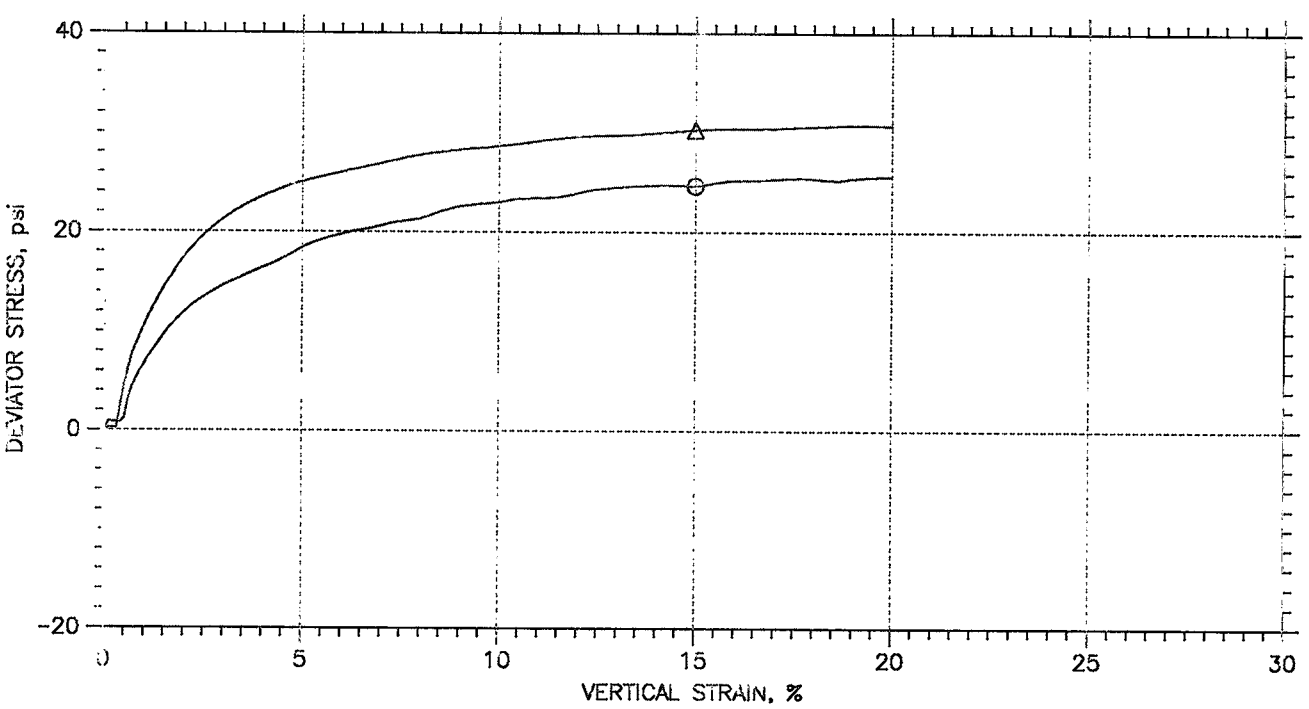
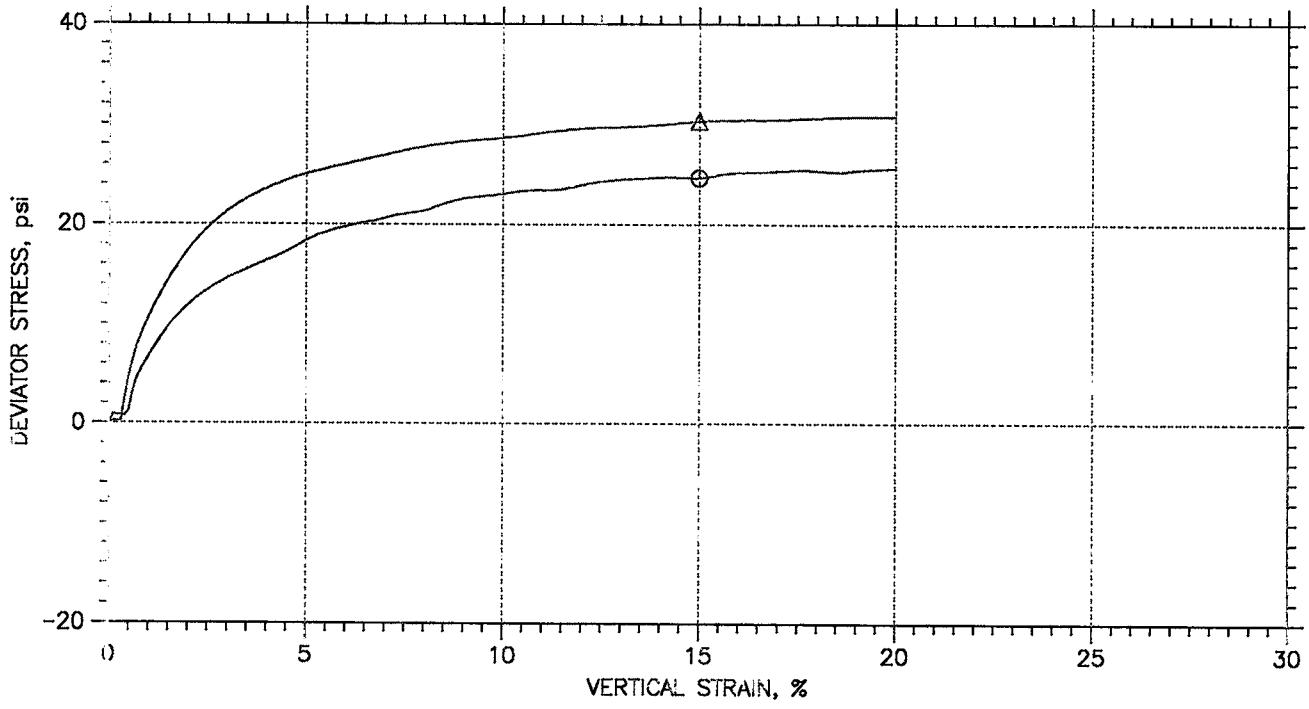


	Sample No.	Test No.	Depth	Tested By	Test Date	Checked By	Check Date	Test File
⊙	SS-349-16	UU-37-16	29.2-29.7	DT	5/26/16	MD		UU-37-2016.dat
△	SS-349-16	UU-38-16	29.8-30.3	DT	5/26/16	MD		UU-38-2016.dat

	Project: IM-2-094(143)260		Location:		Project No.:	
	Boring No.: 1		Sample Type:			
	Description: T.L. 20" Brn Silty Clay with I.O.& Coal deposits, Rock up to 1/2"					
	Remarks:					

SS-349-16

Test File



	Sample No.	Test No.	Depth	Tested By	Test Date	Checked By	Check Date	Test File
○	SS-349-16	UU-37-16	29.2-29.7	DT	5/26/16	MD		UU-37-2016.dat
△	SS-349-16	UU-38-16	29.8-30.3	DT	5/26/16	MD		UU-38-2016.dat

	Project: IM-2-094(143)260		Location:		Project No.:	
	Boring No.: 1		Sample Type:			
	Description: T.L. 20" Brn Silty Clay with I.O.& Coal deposits, Rock up to 1/2"					
	Remarks:					



55-349-16

TRIAxIAL TEST

Project: IM-2-094(143)260  
Boring No.: 1  
Sample No.: SS-349-16  
Test No.: UU-37-16

Location:  
Tested By: DT  
Test Date: 5/26/16  
Sample Type:

Project No.:  
Checked By: MD  
Depth: 29.2-29.7  
Elevation:

Soil Description: T.L. 20" Brn Silty Clay with I.O.& Coal deposits, Rock up to 1/2"  
Remarks:

Specimen Height: 5.75 in  
Specimen Area: 6.32 in<sup>2</sup>  
Specimen Volume: 595.42 cc

Piston Area: 0.16 in<sup>2</sup>  
Piston Friction: 0.00 lb  
Piston Weight: 0.00 lb

Filter Strip Correction: 0.00 psi  
Membrane Correction: 4.20 lb/in  
Correction Type: Uniform

Liquid Limit: ---

Plastic Limit: ---

Estimated Specific Gravity: 2.65

	Before Test Trimmings	Before Test Specimen	After Test Specimen	After Test Trimmings
Container ID	S 61	---		
Wt. Container + Wet Soil, gm	81.13	---	---	0
Wt. Container + Dry Soil, gm	70.02	---	---	0
Wt. Container, gm	15.3	---	---	0
Wt. Wet Soil, gm	65.83	1219.8	1013.9	0
Wt. Dry Soil, gm	54.72	1013.9	1013.9	0
Wt. Water, gm	11.11	205.86	0	0
Water Content, %	20.30	20.30	0.00	0.00
Void Ratio	---	0.56	0.56	---
Degree of Saturation, %	---	96.74	0.00	---
Dry Unit Weight, pcf	---	106.31	106.33	---

Initial

Height: 5.748 in  
Area: 6.3213 in<sup>2</sup>  
Volume: 595.42 cc

Moisture: 20.30 %  
Void Ratio: 0.56  
Dry Unit Weight: 106.31 pcf  
Saturation: 96.74 %

End of Initialization  
Time: 4.3227 min

Total Vertical Stress: 13.718 psi  
Total Horizontal Stress: 13.724 psi  
Pore Pressure: 0 psi  
Effective Vertical Stress: 13.718 psi  
Effective Horizontal Stress: 13.724 psi

Height Change: 0.000416 in  
Area Change: 0 in<sup>2</sup>  
Volume Change: 0.12928 cc  
Water Change: 0.041295 cc  
Correction: 0 cc

Height: 5.7476 in  
Area: 6.3213 in<sup>2</sup>  
Volume: 595.29 cc

Moisture: 20.30 %  
Void Ratio: 0.56  
Dry Unit Weight: 106.33 pcf  
Saturation: 96.78 %

End of Consolidation/A  
Time: 4.3227 min

Total Vertical Stress: 13.718 psi  
Total Horizontal Stress: 13.724 psi  
Pore Pressure: 0 psi  
Effective Vertical Stress: 13.718 psi  
Effective Horizontal Stress: 13.724 psi

Height Change: 0.000416 in  
Area Change: 0 in<sup>2</sup>  
Volume Change: 0.12928 cc  
Water Change: 0.041295 cc  
Correction: 0 cc

Height: 5.7476 in  
Area: 6.3213 in<sup>2</sup>  
Volume: 595.29 cc

Moisture: 20.30 %  
Void Ratio: 0.56  
Dry Unit Weight: 106.33 pcf  
Saturation: 96.78 %

End of Saturation  
Time: 4.3227 min

Total Vertical Stress: 13.718 psi  
Total Horizontal Stress: 13.724 psi  
Pore Pressure: 0 psi  
Effective Vertical Stress: 13.718 psi  
Effective Horizontal Stress: 13.724 psi

Height Change: 0.000416 in  
Area Change: 0 in<sup>2</sup>  
Volume Change: 0.12928 cc  
Water Change: 0.041295 cc  
Correction: 0 cc

Height: 5.7476 in  
Area: 6.3213 in<sup>2</sup>  
Volume: 595.29 cc

Moisture: 20.30 %  
Void Ratio: 0.56  
Dry Unit Weight: 106.33 pcf  
Saturation: 96.78 %

End of Consolidation/B  
Time: 4.3227 min

Total Vertical Stress: 13.718 psi  
Total Horizontal Stress: 13.724 psi  
Pore Pressure: 0 psi  
Effective Vertical Stress: 13.718 psi  
Effective Horizontal Stress: 13.724 psi

Height Change: 0.000416 in  
Area Change: 0 in<sup>2</sup>  
Volume Change: 0.12928 cc  
Water Change: 0.041295 cc  
Correction: 0 cc

Height: 5.7476 in  
Area: 6.3213 in<sup>2</sup>  
Volume: 595.29 cc

Moisture: 20.30 %  
Void Ratio: 0.56  
Dry Unit Weight: 106.33 pcf  
Saturation: 96.78 %

End of Shear  
Time: 24.621 min

Total Vertical Stress: 39.43 psi  
Total Horizontal Stress: 13.837 psi  
Pore Pressure: -0.67185 psi  
Effective Vertical Stress: 40.102 psi  
Effective Horizontal Stress: 14.509 psi

Height Change: 1.15 in  
Area Change: -1.5793 in<sup>2</sup>  
Volume Change: 0.12928 cc  
Water Change: 0.041295 cc  
Correction: 205.82 cc

Height: 4.598 in  
Area: 7.9007 in<sup>2</sup>  
Volume: 595.29 cc

Moisture: 0.00 %  
Void Ratio: 0.56  
Dry Unit Weight: 106.33 pcf  
Saturation: 0.00 %

At Failure  
Time: 19.539 min

Total Vertical Stress: 38.513 psi  
Total Horizontal Stress: 13.877 psi  
Pore Pressure: -0.67185 psi  
Effective Vertical Stress: 39.184 psi  
Effective Horizontal Stress: 14.549 psi

Height Change: 0.86255 in  
Area Change: -1.1156 in<sup>2</sup>  
Volume Change: 0.12928 cc  
Water Change: 0.041295 cc  
Correction: 0 cc

Height: 4.8854 in  
Area: 7.4369 in<sup>2</sup>  
Volume: 595.29 cc

Moisture: 20.30 %  
Void Ratio: 0.56  
Dry Unit Weight: 106.33 pcf  
Saturation: 96.78 %

SS-349-16

TRIAxIAL TEST

Project: IM-2-094(143)260  
Boring No.: 1  
Sample No.: SS-349-16  
Test No.: UU-38-16

Location:  
Tested By: DT  
Test Date: 5/26/16  
Sample Type:

Project No.:  
Checked By: MD  
Depth: 29.8-30.3  
Elevation:

Soil Description: T.L. 20" Brn Silty Clay with I.O.& Coal deposits, Rock up to 1/2"  
Remarks:

Specimen Height: 5.74 in  
Specimen Area: 6.38 in<sup>2</sup>  
Specimen Volume: 600.16 cc

Piston Area: 0.16 in<sup>2</sup>  
Piston Friction: 0.00 lb  
Piston Weight: 0.00 lb

Filter Strip Correction: 0.00 psi  
Membrane Correction: 4.20 lb/in  
Correction Type: Uniform

Liquid Limit: ---

Plastic Limit: ---

Estimated Specific Gravity: 2.65

	Before Test Trimmings	Before Test Specimen	After Test Specimen	After Test Trimmings
Container ID	S 69	---		
Wt. Container + Wet Soil, gm	79.14	---	---	0
Wt. Container + Dry Soil, gm	68.01	---	---	0
Wt. Container, gm	15.36	---	---	0
Wt. Wet Soil, gm	63.78	1238.7	1022.5	0
Wt. Dry Soil, gm	52.65	1022.5	1022.5	0
Wt. Water, gm	11.13	216.15	0	0
Water Content, %	21.14	21.14	0.00	0.00
Void Ratio	---	0.56	0.55	---
Degree of Saturation, %	---	100.86	0.00	---
Dry Unit Weight, pcf	---	106.36	106.73	---

Initial

Height: 5.737 in  
Area: 6.3839 in<sup>2</sup>  
Volume: 600.16 cc

Moisture: 21.14 %  
Void Ratio: 0.56  
Dry Unit Weight: 106.36 pcf  
Saturation: 100.86 %

End of Initialization

Time: 3.5448 min  
Total Vertical Stress: 27.387 psi  
Total Horizontal Stress: 27.391 psi  
Pore Pressure: 0 psi  
Effective Vertical Stress: 27.387 psi  
Effective Horizontal Stress: 27.391 psi

Height Change: 0.006656 in  
Area Change: 0 in<sup>2</sup>  
Volume Change: 2.0889 cc  
Water Change: -0.016625 cc  
Correction: 3.9462 cc

Height: 5.7303 in  
Area: 6.3839 in<sup>2</sup>  
Volume: 598.08 cc

Moisture: 20.76 %  
Void Ratio: 0.55  
Dry Unit Weight: 106.73 pcf  
Saturation: 100.00 %

End of Consolidation/A

Time: 3.5448 min  
Total Vertical Stress: 27.387 psi  
Total Horizontal Stress: 27.391 psi  
Pore Pressure: 0 psi  
Effective Vertical Stress: 27.387 psi  
Effective Horizontal Stress: 27.391 psi

Height Change: 0.006656 in  
Area Change: 0 in<sup>2</sup>  
Volume Change: 2.0889 cc  
Water Change: -0.016625 cc  
Correction: 3.9462 cc

Height: 5.7303 in  
Area: 6.3839 in<sup>2</sup>  
Volume: 598.08 cc

Moisture: 20.76 %  
Void Ratio: 0.55  
Dry Unit Weight: 106.73 pcf  
Saturation: 100.00 %

End of Saturation

Time: 3.5448 min  
Total Vertical Stress: 27.387 psi  
Total Horizontal Stress: 27.391 psi  
Pore Pressure: 0 psi  
Effective Vertical Stress: 27.387 psi  
Effective Horizontal Stress: 27.391 psi

Height Change: 0.006656 in  
Area Change: 0 in<sup>2</sup>  
Volume Change: 2.0889 cc  
Water Change: -0.016625 cc  
Correction: 3.9462 cc

Height: 5.7303 in  
Area: 6.3839 in<sup>2</sup>  
Volume: 598.08 cc

Moisture: 20.76 %  
Void Ratio: 0.55  
Dry Unit Weight: 106.73 pcf  
Saturation: 100.00 %

End of Consolidation/B

Time: 3.5448 min  
Total Vertical Stress: 27.387 psi  
Total Horizontal Stress: 27.391 psi  
Pore Pressure: 0 psi  
Effective Vertical Stress: 27.387 psi  
Effective Horizontal Stress: 27.391 psi

Height Change: 0.006656 in  
Area Change: 0 in<sup>2</sup>  
Volume Change: 2.0889 cc  
Water Change: -0.016625 cc  
Correction: 3.9462 cc

Height: 5.7303 in  
Area: 6.3839 in<sup>2</sup>  
Volume: 598.08 cc

Moisture: 20.76 %  
Void Ratio: 0.55  
Dry Unit Weight: 106.73 pcf  
Saturation: 100.00 %

End of Shear

Time: 23.868 min  
Total Vertical Stress: 58.427 psi  
Total Horizontal Stress: 27.593 psi  
Pore Pressure: -0.33188 psi  
Effective Vertical Stress: 58.758 psi  
Effective Horizontal Stress: 27.925 psi

Height Change: 1.1528 in  
Area Change: -1.5775 in<sup>2</sup>  
Volume Change: 2.0889 cc  
Water Change: -0.016625 cc  
Correction: 216.17 cc

Height: 4.5842 in  
Area: 7.9614 in<sup>2</sup>  
Volume: 598.08 cc

Moisture: 0.00 %  
Void Ratio: 0.55  
Dry Unit Weight: 106.73 pcf  
Saturation: 0.00 %

At Failure

Time: 18.78 min  
Total Vertical Stress: 57.839 psi  
Total Horizontal Stress: 27.561 psi  
Pore Pressure: -0.26712 psi  
Effective Vertical Stress: 58.106 psi  
Effective Horizontal Stress: 27.828 psi

Height Change: 0.86621 in  
Area Change: -1.1355 in<sup>2</sup>  
Volume Change: 2.0889 cc  
Water Change: -0.016625 cc  
Correction: 0 cc

Height: 4.8708 in  
Area: 7.5194 in<sup>2</sup>  
Volume: 598.08 cc

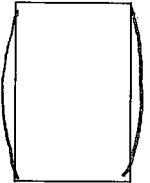
Moisture: 20.76 %  
Void Ratio: 0.55  
Dry Unit Weight: 106.73 pcf  
Saturation: 100.00 %

**TRIAXIAL UU(Q) CCU(R) CD(S)**

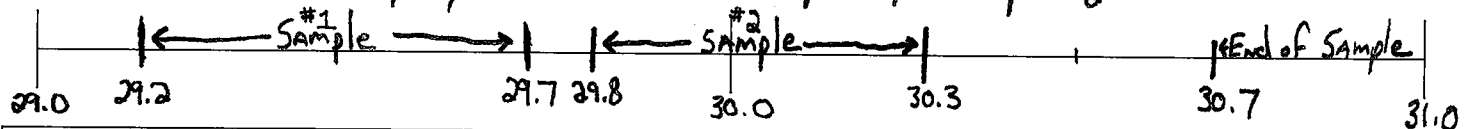
North Dakota Department of Transportation, Materials & Research  
SFN 50459 (5-2016)

Project Number  
**IM-2-094(143)260**  
Boring Number  
**1**

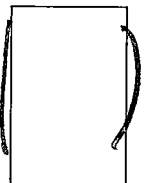
Field Sample Number <b>55-349-16</b>		Lab Number <b>UU-37-16</b>		Depth <b>29.2 to 29.7</b>	
Weight of Sample <b>1219.81</b>		Confining Pressure <b>13.8</b>		Test Number <b>1</b> of <b>2</b>	
Diameter	<b>2.840</b>	<b>2.840</b>	Height	<b>5.746</b>	Moisture Can Number <b>561</b>
	<b>2.830</b>	<b>2.834</b>		<b>5.747</b>	Wet Wt + Can <b>81.13</b>
	<b>2.837</b>	<b>2.842</b>		<b>5.751</b>	Dry Wt + Can <b>70.02</b>
	Average <b>2.837</b>			Average <b>5.748</b>	



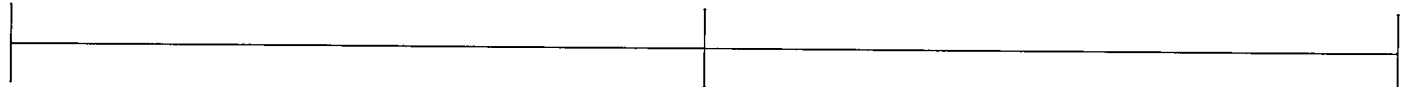
Total Length: **20"** **BRN Silty Clay with I.O. + Coal Deposits, Rock up to 1/2"**



Field Sample Number <b>55-349-16</b>		Lab Number <b>UU-38-16</b>		Depth <b>29.8 to 30.3</b>	
Weight of Sample <b>1238.66</b>		Confining Pressure <b>27.5</b>		Test Number <b>2</b> of <b>2</b>	
Diameter	<b>2.862</b>	<b>2.836</b>	Height	<b>5.738</b>	Moisture Can Number <b>569</b>
	<b>2.850</b>	<b>2.846</b>		<b>5.738</b>	Wet Wt + Can <b>79.14</b>
	<b>2.863</b>	<b>2.850</b>		<b>5.736</b>	Dry Wt + Can <b>68.01</b>
	Average <b>2.851</b>			Average <b>5.737</b>	



Total Length:



Field Sample Number		Lab Number		Depth	
Weight of Sample		Confining Pressure		Test Number of	
Diameter			Height		Moisture Can Number
					Wet Wt + Can
					Dry Wt + Can
	Average			Average	

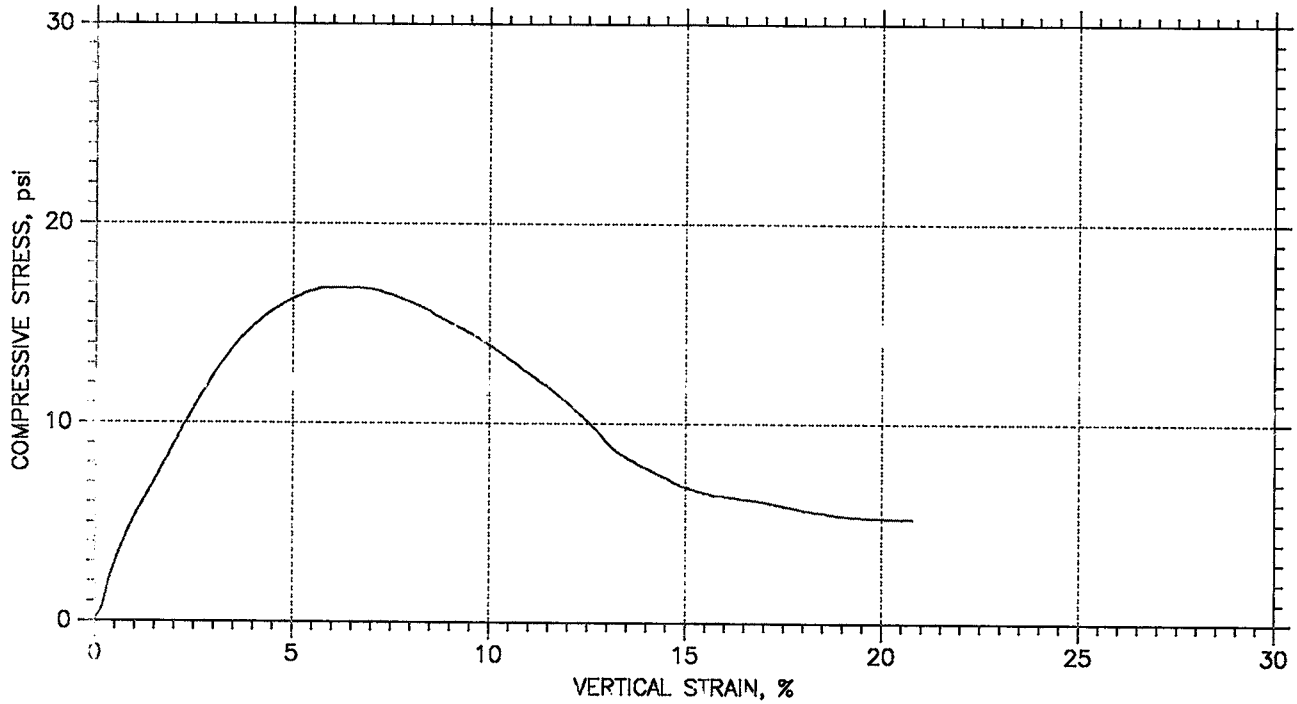
Total Length:






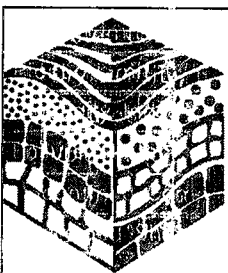
AASHTO T-234 Tested By:

55-347-16

### UNCONFINED COMPRESSION TEST REPORT



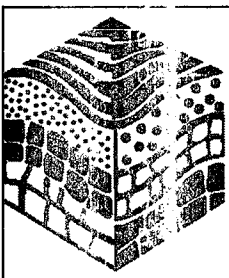
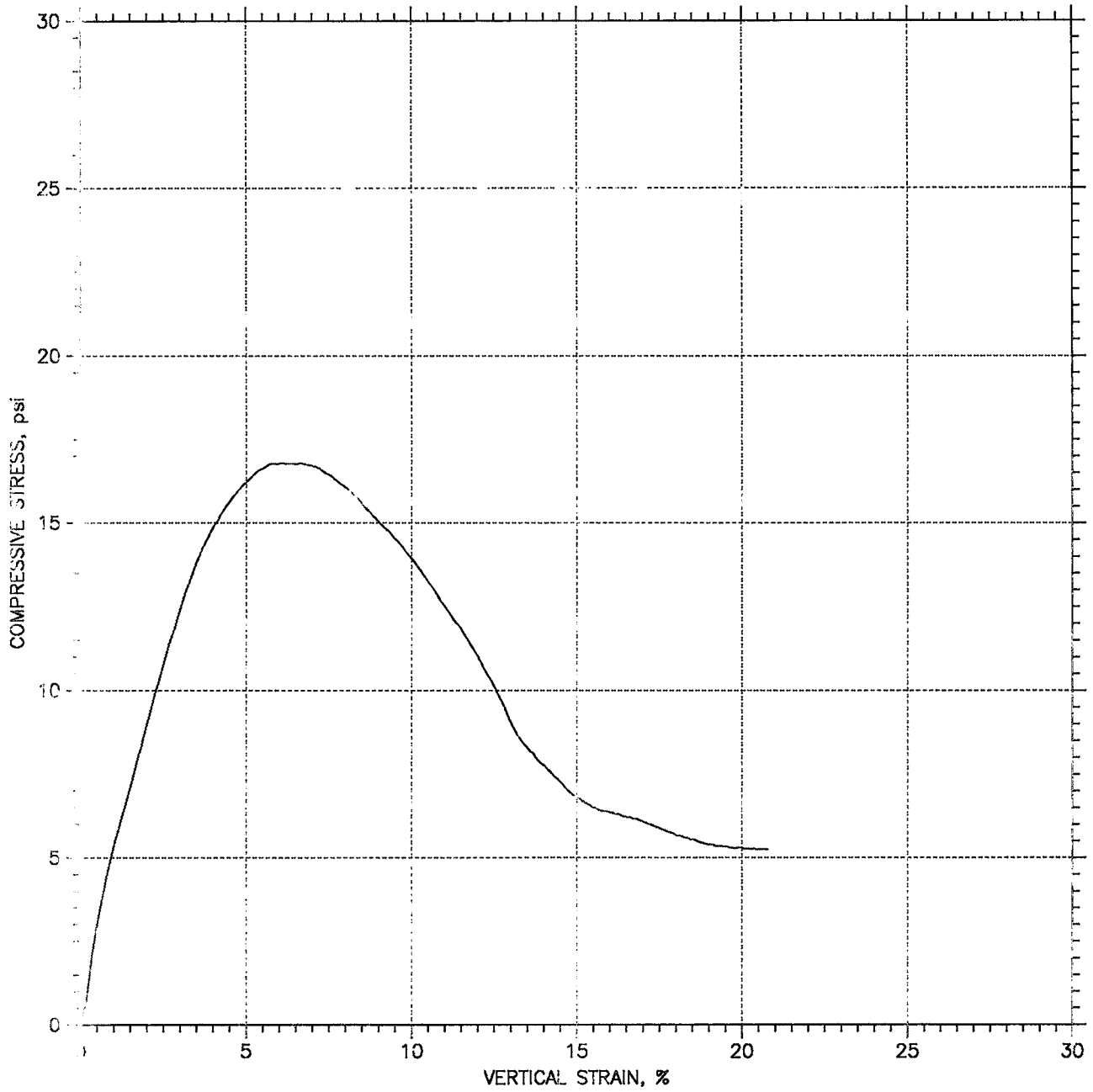
Symbol				
Test No.		UC-20-16		
Initial	Diameter, in	2.848		
	Height, in	5.738		
	Water Content, %	19.87		
	Dry Density, pcf	108.4		
	Saturation, %	100.22		
	Void Ratio	0.525		
Unconfined Compressive Strength, psi		16.78		
Undrained Shear Strength, psi		8.39		
Time to Failure, min		5.918		
Strain Rate, %/min		1		
Measured Specific Gravity		2.65		
Liquid Limit		0		
Plastic Limit		0		
Plasticity Index		0		
Failure Sketch				



Project: IM-2-094(143)260
Location:
Project No.:
Boring No.: 1
Sample Type: Undisturbed
Description: T. L. 18" Brn Silty Clay with I.O. deposits, rock up to 1/2".
Remarks:

SS-347-16

# UNCONFINED COMPRESSION TEST REPORT



Project: IM-2-094(143)260	Location:	Project No.:
Boring No.: 1	Tested By: DT	Checked By: MD
Sample No.: SS-347-16	Test Date: 5/26/2016	Depth: 24.2-24.7
Test No.: UC-20-16	Sample Type: Undisturbed	Elevation:
Description: T. L. 18" Brn Silty Clay with I.O. deposits, rock up to 1/2".		
Remarks:		

## UNCONFINED COMPRESSION TEST

SS-347-16

Project: IM-2-094(043)260  
 Boring No.: 1  
 Sample No.: SS-347-16  
 Test No.: UC-20-16

Location:  
 Tested By: DT  
 Test Date: 5/26/2016  
 Sample Type: Undisturbed

Project No.:  
 Checked By: MD  
 Depth: 24.2-24.7  
 Elevation:

Soil Description: 1. L. 18" Brn Silty Clay with I.O. deposits, rock up to 1/2".  
 Remarks:

Specimen Height: 5.74 in  
 Specimen Area: 6.37 in<sup>2</sup>  
 Specimen Volume: 559.01 cc

Liquid Limit: 0  
 Plastic Limit: 0  
 Measured Specific Gravity: 2.65

Cap Mass: 0 gm

Water Content Information  
 -----

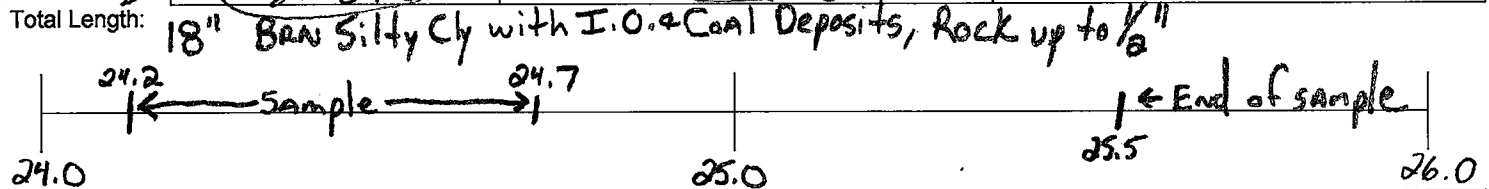
Container ID	s42
Wt. Container, gm	16.95
Wt. Container + Wet Soil, gm	87.22
Wt. Container + Dry Soil, gm	75.57
Wt. Dry Soil, gm	58.62
Water Content, %	19.87
Void Ratio	0.53
Degree of Saturation, %	100.22
Wet Unit Weight,pcf	130
Dry Unit Weight, pcf	108.45

**UNCONFINED COMPRESSION**

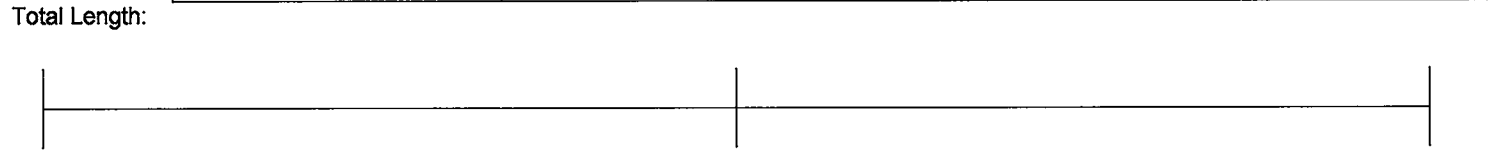
North Dakota Department of Transportation, Materials & Research  
SFN 50460 (5-2016)

Project Number <b>IM-2-094(143)260</b>
Boring Number <b>1</b>

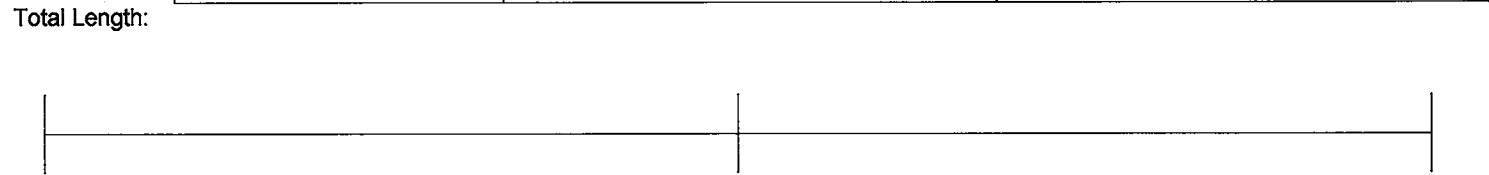
Field Sample Number <b>55-347-16</b>	Lab Number <b>UC-20-16</b>	Depth <b>24.2 to 24.7</b>
Weight of Sample <b>1247.37</b>	Test Number <b>1</b> of <b>1</b>	AASHTO T-208 Tested by: <b>DT/m</b>
Diameter	Height	Moisture Can Number
1. <b>2.853</b> 4. <b>2.840</b>	1. <b>5.740</b>	<b>542</b>
2. <b>2.846</b> 5. <b>2.850</b>	2. <b>5.737</b>	Wet Wt + Can <b>87.22</b>
3. <b>2.857</b> 6. <b>2.843</b>	3. <b>5.738</b>	Dry Wt + Can <b>75.57</b>
Average <b>2.848</b>	Average <b>5.738</b>	Wt of Can <b>16.95</b>



Field Sample Number	Lab Number	Depth
Weight of Sample	Test Number of	AASHTO T-208 Tested by:
Diameter	Height	Moisture Can Number
1.      4.	1.	
2.      5.	2.	Wet Wt + Can
3.      6.	3.	Dry Wt + Can
Average	Average	Wt of Can

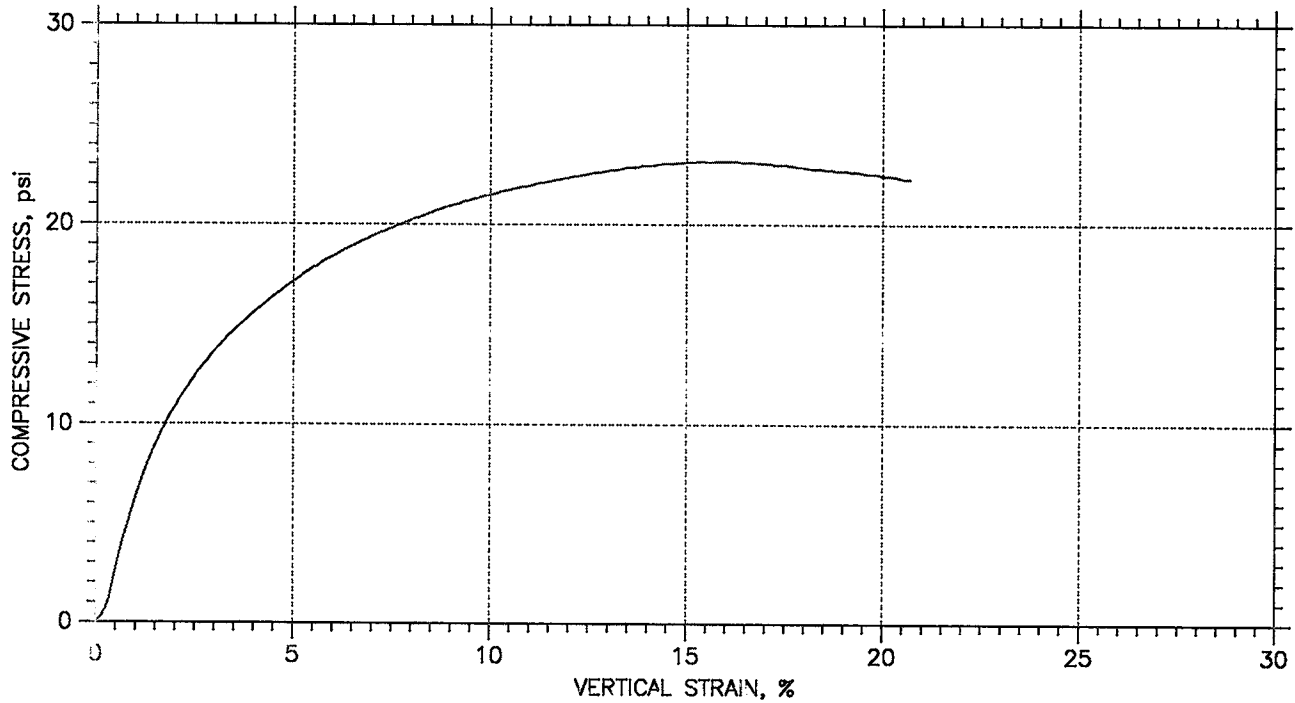


Field Sample Number	Lab Number	Depth
Weight of Sample	Test Number of	AASHTO T-208 Tested by:
Diameter	Height	Moisture Can Number
1.      4.	1.	
2.      5.	2.	Wet Wt + Can
3.      6.	3.	Dry Wt + Can
Average	Average	Wt of Can

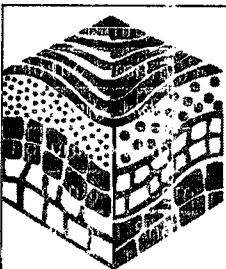


SS-345-16

### UNCONFINED COMPRESSION TEST REPORT



<b>Symbol</b>				
<b>Test No.</b>		UC-19-16		
<b>Initial</b>	Diameter, in	2.847		
	Height, in	5.715		
	Water Content, %	21.56		
	Dry Density, pcf	106.4		
	Saturation, %	103.03		
	Void Ratio	0.555		
<b>Unconfined Compressive Strength, psi</b>		23.17		
<b>Undrained Shear Strength, psi</b>		11.59		
<b>Time to Failure, min</b>		14.917		
<b>Strain Rate, %/min</b>		1		
<b>Measured Specific Gravity</b>		2.65		
<b>Liquid Limit</b>		0		
<b>Plastic Limit</b>		0		
<b>Plasticity Index</b>		0		
<b>Failure Sketch</b>				

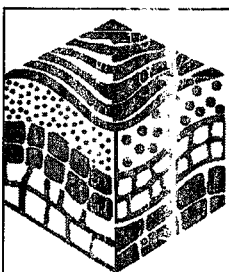
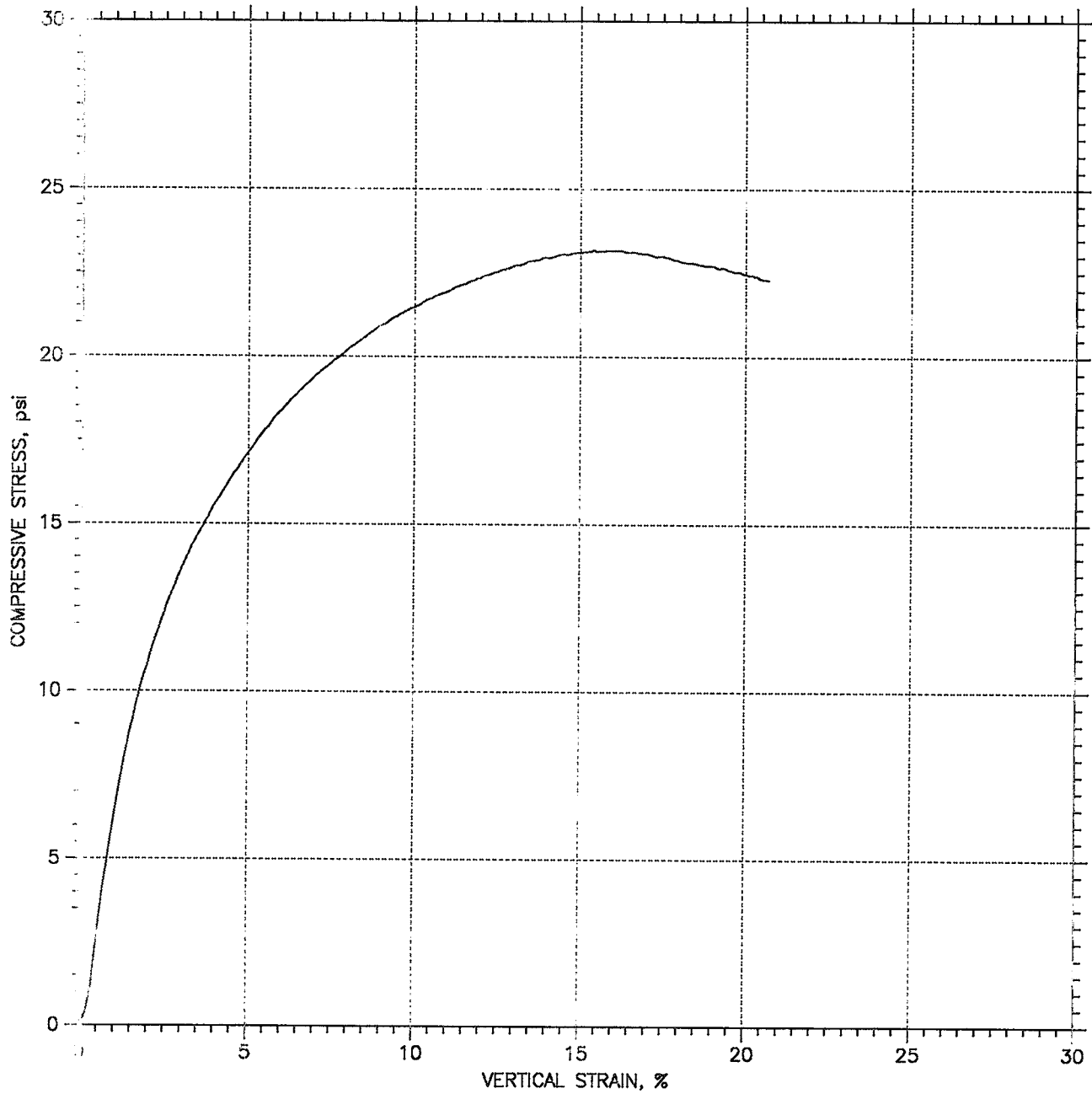


<b>Project:</b> IM-2-094(143)260
<b>Location:</b>
<b>Project No.:</b>
<b>Boring No.:</b> 1
<b>Sample Type:</b> Undisturbed
<b>Description:</b> T. L. 17" Brn Clay with I.O. deposits, rock up to 1 3/4"
<b>Remarks:</b>



SS-345-16

# UNCONFINED COMPRESSION TEST REPORT



Project: IM-2-094(143)260	Location:	Project No.:
Boring No.: 1	Tested By: DT	Checked By: MD
Sample No.: SS-345-16	Test Date: 5/26/2016	Depth: 19.9-20.4
Test No.: UC-19-16	Sample Type: Undisturbed	Elevation:
Description: T. L. 17" Brn Clay with I.O. deposits, rock up to 1 3/4"		
Remarks:		

UNCONFINED COMPRESSION TEST

SS-345-16

Project: IM-2-094(143)260  
 Boring No.: 1  
 Sample No.: SS-345-16  
 Test No.: UC-19-16

Location:  
 Tested By: DT  
 Test Date: 5/26/2016  
 Sample Type: Undisturbed

Project No.:  
 Checked By: MD  
 Depth: 19.9-20.4  
 Elevation:

Soil Description: 1. L. 17" Brn Clay with I.O. deposits, rock up to 1 3/4"  
 Remarks:

Specimen Height: 5.71 in  
 Specimen Area: 6.37 in<sup>2</sup>  
 Specimen Volume: 586.19 cc

Liquid Limit: 0  
 Plastic Limit: 0  
 Measured Specific Gravity: 2.65

Cap Mass: 0 gm

Water Content Information

Container ID	s13
Wt. Container, gm	17.04
Wt. Container + Wet Soil, gm	64.96
Wt. Container + Dry Soil, gm	56.46
Wt. Dry Soil, gm	39.42
Water Content, %	21.56
Void Ratio	0.55
Degree of Saturation, %	103.03
Wet Unit Weight, pcf	129.36
Dry Unit Weight, pcf	106.42

**UNCONFINED COMPRESSION**

North Dakota Department of Transportation, Materials & Research  
SFN 50460 (5-2016)

Project Number <i>IM-2-094(143)260</i>
Boring Number <i>1</i>

Field Sample Number <i>SS-345-16</i>		Lab Number <i>UC-19-16</i>		Depth <i>19.9-20.4</i>	
Weight of Sample <i>1235.41</i>		Test Number <i>1 of 1</i>		AASHTO T-208 Tested by:	
Diameter	1. <i>2.835</i>	4. <i>2.830</i>	Height	1. <i>5.718</i>	Moisture Can Number <i>S13</i>
	2. <i>2.855</i>	5. <i>2.849</i>		2. <i>5.713</i>	Wet Wt + Can <i>64.96</i>
	3. <i>2.866</i>	6. <i>2.852</i>		3. <i>5.713</i>	Dry Wt + Can <i>56.46</i>
	Average <i>2.847</i>			Average <i>5.715</i>	

Total Length: *19.0* | *17" Brn, clay with I.O. deposits rock upto 1 3/4"* | *21.0*  
*13/4" Rock* | *20.0 sample*

Field Sample Number		Lab Number		Depth	
Weight of Sample		Test Number of		AASHTO T-208 Tested by:	
Diameter	1.	4.	Height	1.	Moisture Can Number
	2.	5.		2.	Wet Wt + Can
	3.	6.		3.	Dry Wt + Can
	Average			Average	

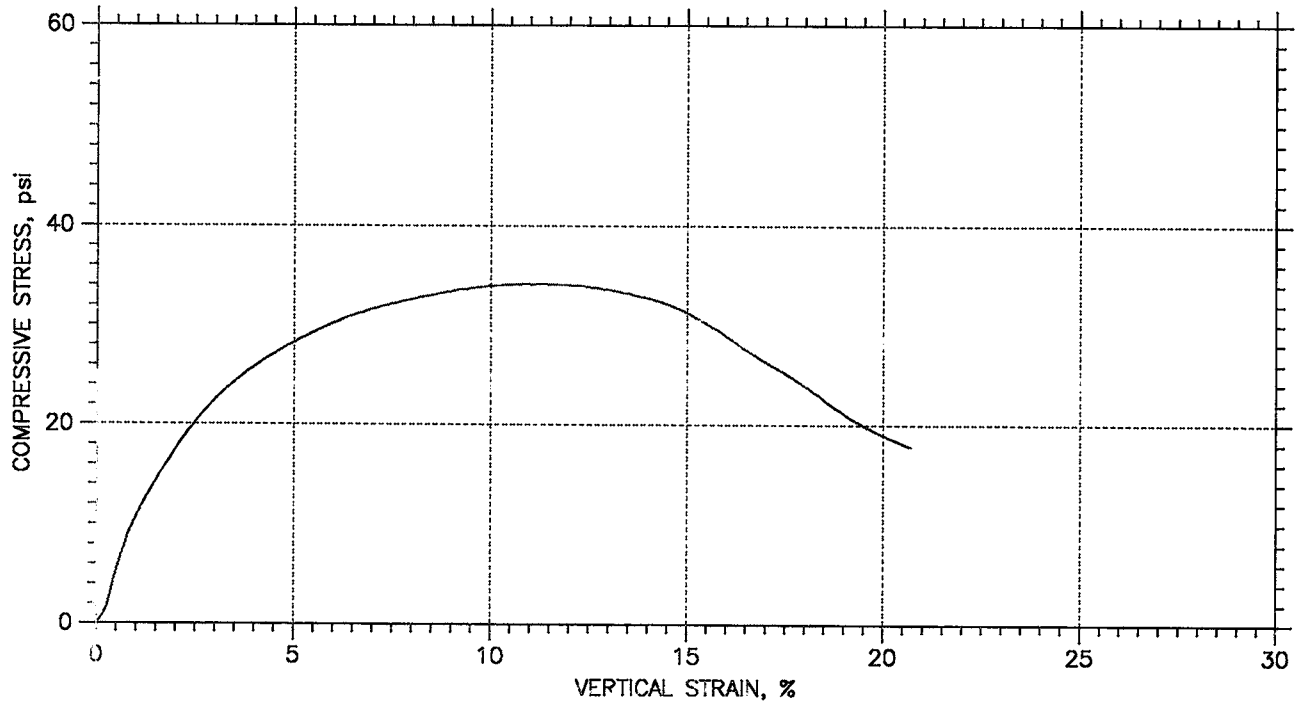
Total Length:

Field Sample Number		Lab Number		Depth	
Weight of Sample		Test Number of		AASHTO T-208 Tested by:	
Diameter	1.	4.	Height	1.	Moisture Can Number
	2.	5.		2.	Wet Wt + Can
	3.	6.		3.	Dry Wt + Can
	Average			Average	

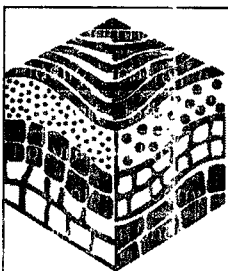
Total Length:

55-339-16

### UNCONFINED COMPRESSION TEST REPORT



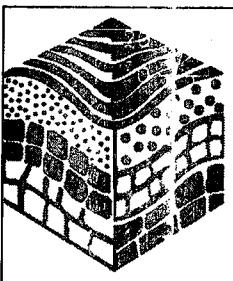
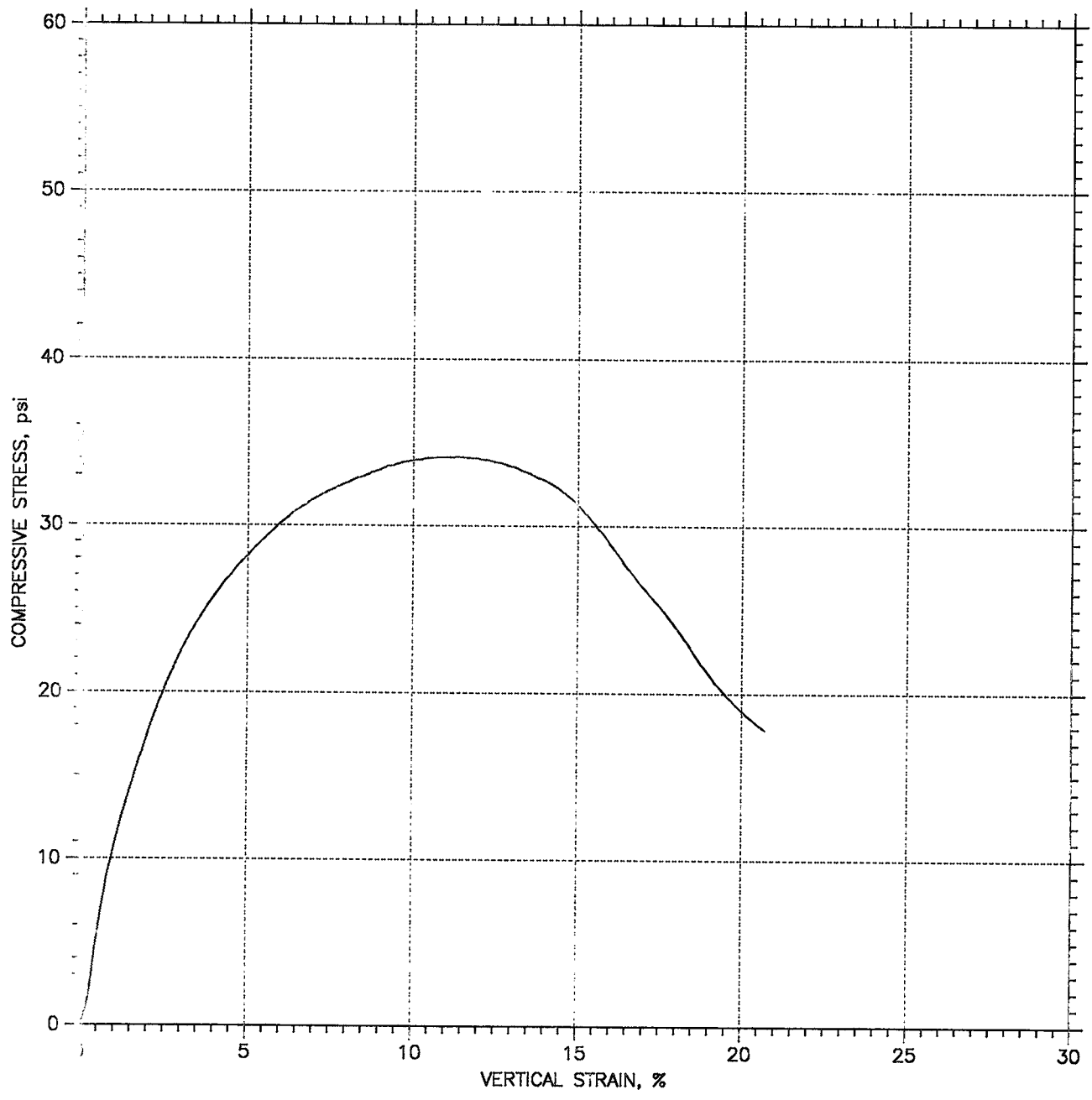
Symbol				
Test No.		UC-18-16		
Initial	Diameter, in	2.857		
	Height, in	5.739		
	Water Content, %	19.72		
	Dry Density, pcf	107.9		
	Saturation, %	97.93		
	Void Ratio	0.534		
Unconfined Compressive Strength, psi		34.11		
Undrained Shear Strength, psi		17.06		
Time to Failure, min		10.917		
Strain Rate, %/min		1		
Measured Specific Gravity		2.65		
Liquid Limit		0		
Plastic Limit		0		
Plasticity Index		0		
Failure Sketch				



Project: IM-2-094(143)260
Location:
Project No.:
Boring No.: 1
Sample Type: Undisturbed
Description: T. L. 14" Brn Clay with I.O. Deposits, rock up to 1/2"
Remarks:

SS-339-16

# UNCONFINED COMPRESSION TEST REPORT



Project: IM-2-094(143)260	Location:	Project No.:
Boring No.: 1	Tested By: DT	Checked By: MD
Sample No.: SS-339-16	Test Date: 5/26/2016	Depth: 4.4-4.9
Test No.: UC-18-16	Sample Type: Undisturbed	Elevation:
Description: T. L. 14" Brn Clay with I.O. Deposits, rock up to 1/2"		
Remarks:		

## UNCONFINED COMPRESSION TEST

SS-339-16

Project: IM-2-094(143)260  
 Boring No.: 1  
 Sample No.: SS-339-16  
 Test No.: UC-18-16

Location:  
 Tested By: DT  
 Test Date: 5/26/2016  
 Sample Type: Undisturbed

Project No.:  
 Checked By: MD  
 Depth: 4.4-4.9  
 Elevation:

Soil Description: T. L. 14" Brn Clay with I.O. Deposits, rock up to 1/2"  
 Remarks:

Specimen Height: 5.74 in  
 Specimen Area: 6.41 in<sup>2</sup>  
 Specimen Volume: 602.90 cc

Liquid Limit: 0  
 Plastic Limit: 0  
 Measured Specific Gravity: 2.65

Cap Mass: 0 gm

Water Content Information  
 -----

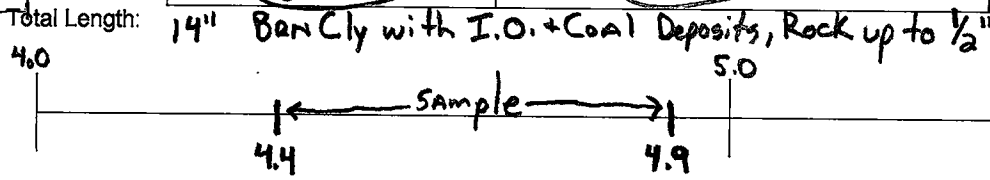
Container ID	s39
Wt. Container, gm	16.89
Wt. Container + Wet Soil, gm	68.06
Wt. Container + Dry Soil, gm	59.63
Wt. Dry Soil, gm	42.74
Water Content, %	19.72
Void Ratio	0.53
Degree of Saturation, %	97.93
Wet Unit weight, pcf	129.14
Dry Unit weight, pcf	107.86

**UNCONFINED COMPRESSION**

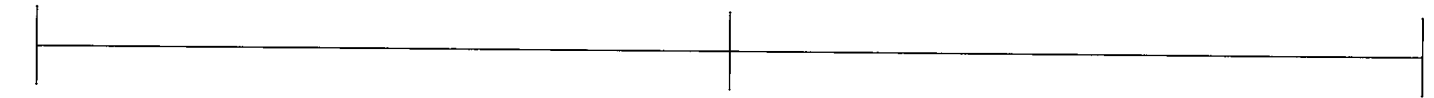
North Dakota Department of Transportation, Materials & Research  
 SFN 50460 (5-2016)

Project Number	IM-2-094(143)260
Boring Number	1
Depth	4.4 to 4.9
AASHTO T-208 Tested by:	DT/MD
Moisture Can Number	539
Wet Wt + Can	68.06
Dry Wt + Can	59.63
Wt of Can	16.89

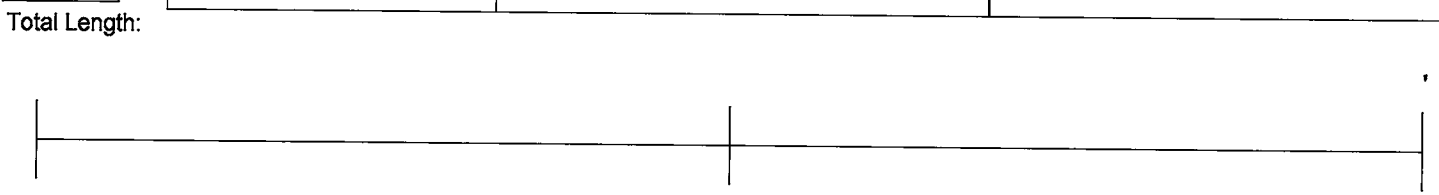
Field Sample Number	SS-339-16		Lab Number	UC-18-16	
Weight of Sample	1247.16		Test Number	1 of 1	
Diameter	1. 2.853	4. 2.857	Height	1. 5.740	
	2. 2.852	5. 2.863		2. 5.738	
	3. 2.855	6. 2.862		3. 5.741	
Average	2.857		Average	5.739	



Field Sample Number			Lab Number		
Weight of Sample			Test Number	of	
Diameter	1.	4.	Height	1.	
	2.	5.		2.	
	3.	6.		3.	
Average			Average		

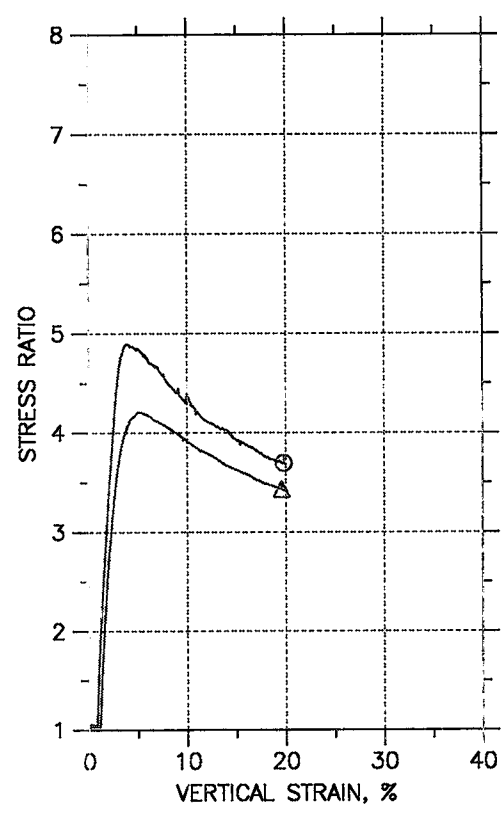
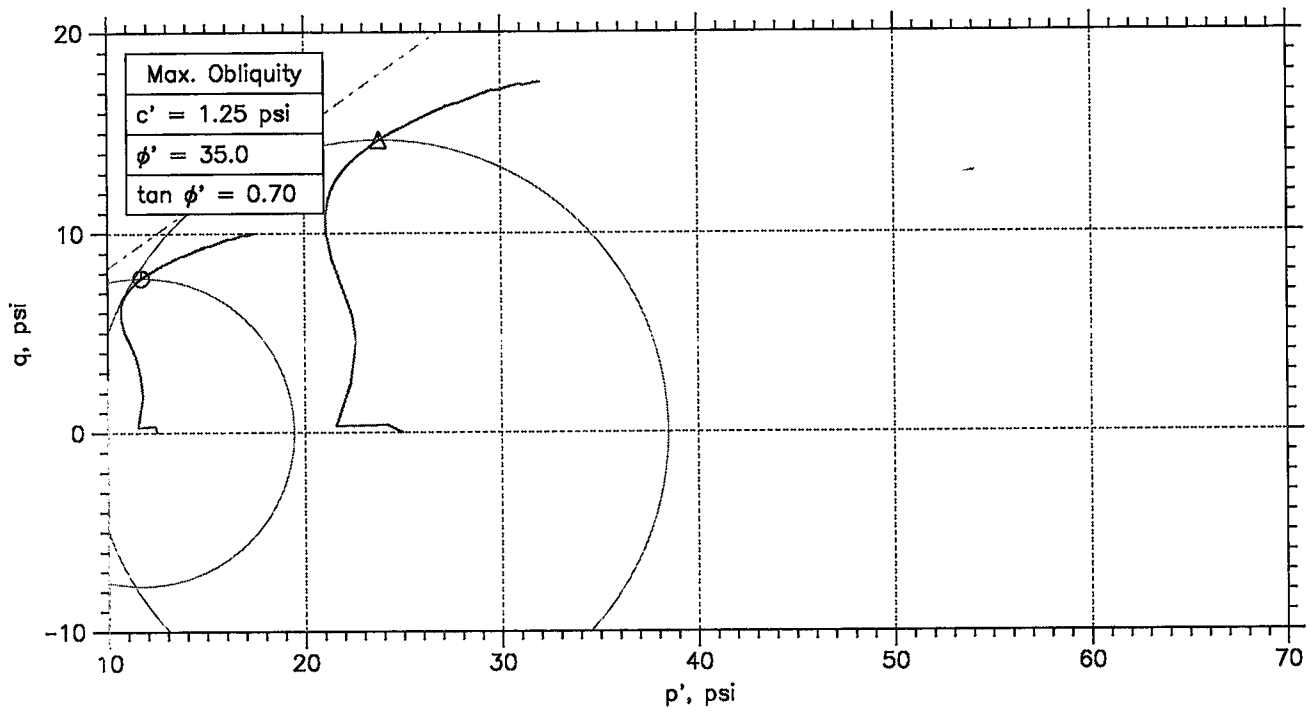


Field Sample Number			Lab Number		
Weight of Sample			Test Number	of	
Diameter	1.	4.	Height	1.	
	2.	5.		2.	
	3.	6.		3.	
Average			Average		



SS-343-16

### CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767



Symbol	⊙	△	
Sample No.	SS-343-16	SS-343-16	
Test No.	CU-6-16	CU-7-16	
Depth	14.1-14.6	14.6-15.1	
Initial	Diameter, in	2.833	2.85
	Height, in	5.739	5.73
	Water Content, %	22.0	23.0
	Dry Density, pcf	104.6	105.3
	Saturation, %	100.2	106.5
Before Shear	Void Ratio	0.582	0.571
	Water Content, %	23.5	20.6
	Dry Density, pcf	101.9	107.
	Saturation*, %	100.0	100.0
	Void Ratio	0.623	0.545
	Back Press., psi	60.92	61.02
	Ver. Eff. Cons. Stress, psi	12.57	24.96
	Shear Strength, psi	9.989	17.52
	Strain at Failure, %	19.8	19.6
	Strain Rate, %/min	0.075	0.075
	B-Value	0.95	0.95
	Estimated Specific Gravity	2.65	2.65
	Liquid Limit	---	---
	Plastic Limit	---	---

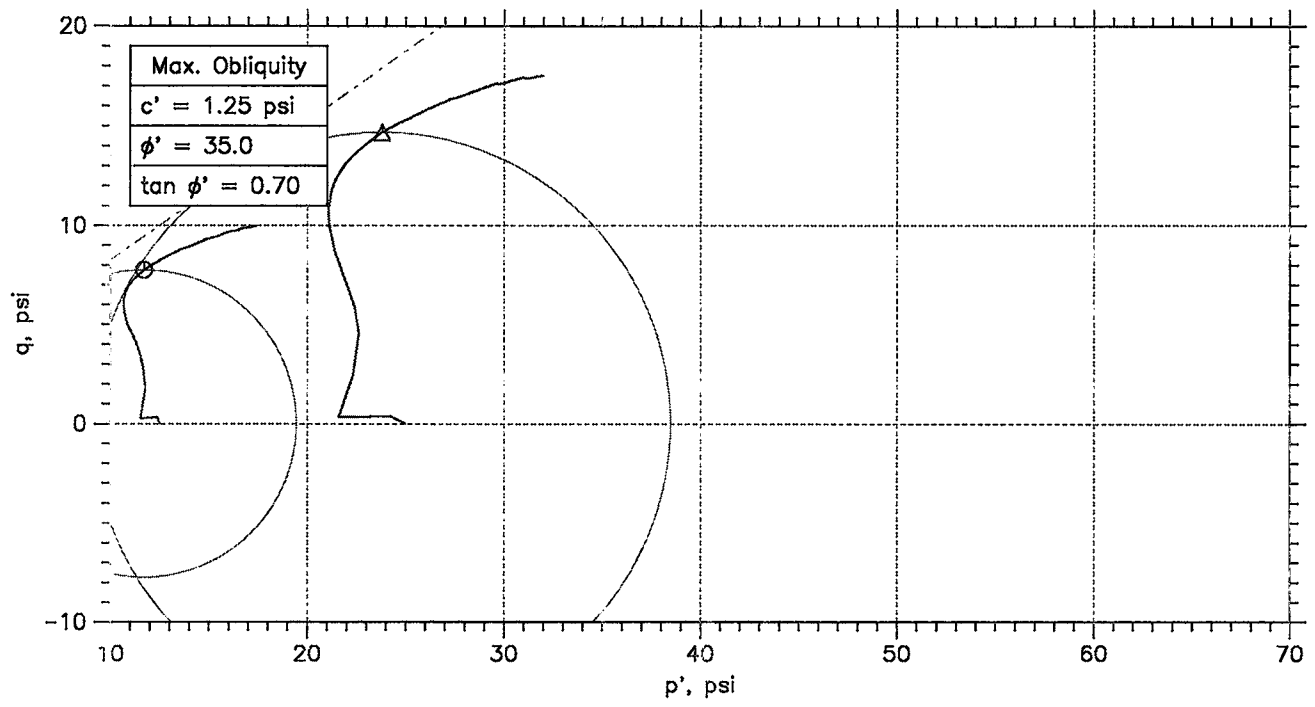
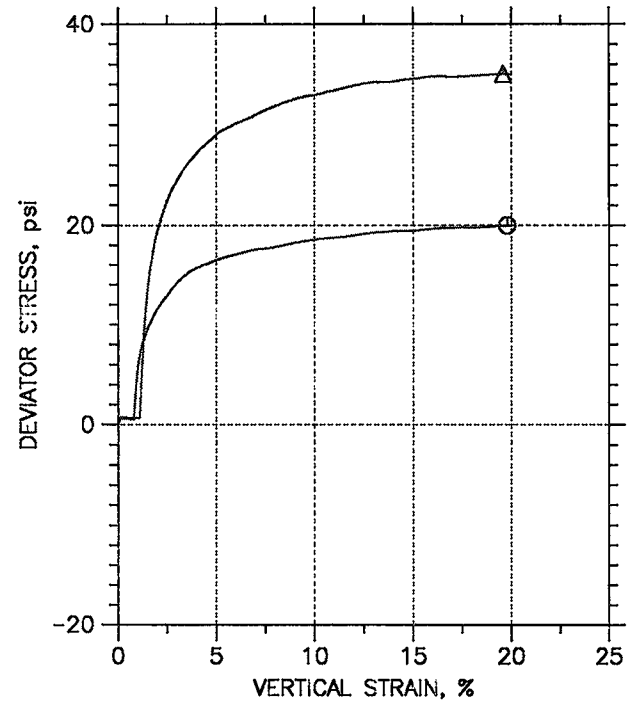
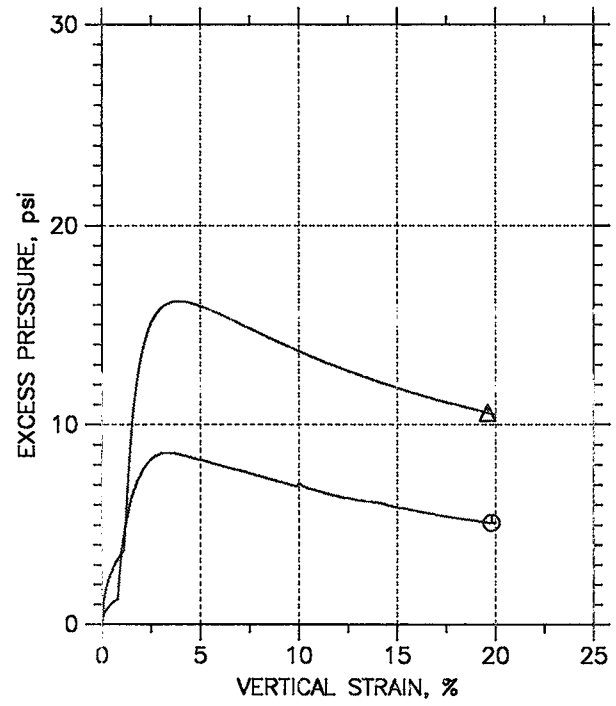
	Project: IM-2-094(143)260	
	Location:	
	Project No.:	
	Boring No.: 1	
	Sample Type:	
	Description: T. L. 18" Brn Silty Clay with I.O. Coal Deposits, Rock upto 3/4"	
Remarks:		

Phase calculations based on start and end of test.  
 \* Saturation is set to 100% for phase calculations.



SS-343-16

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767

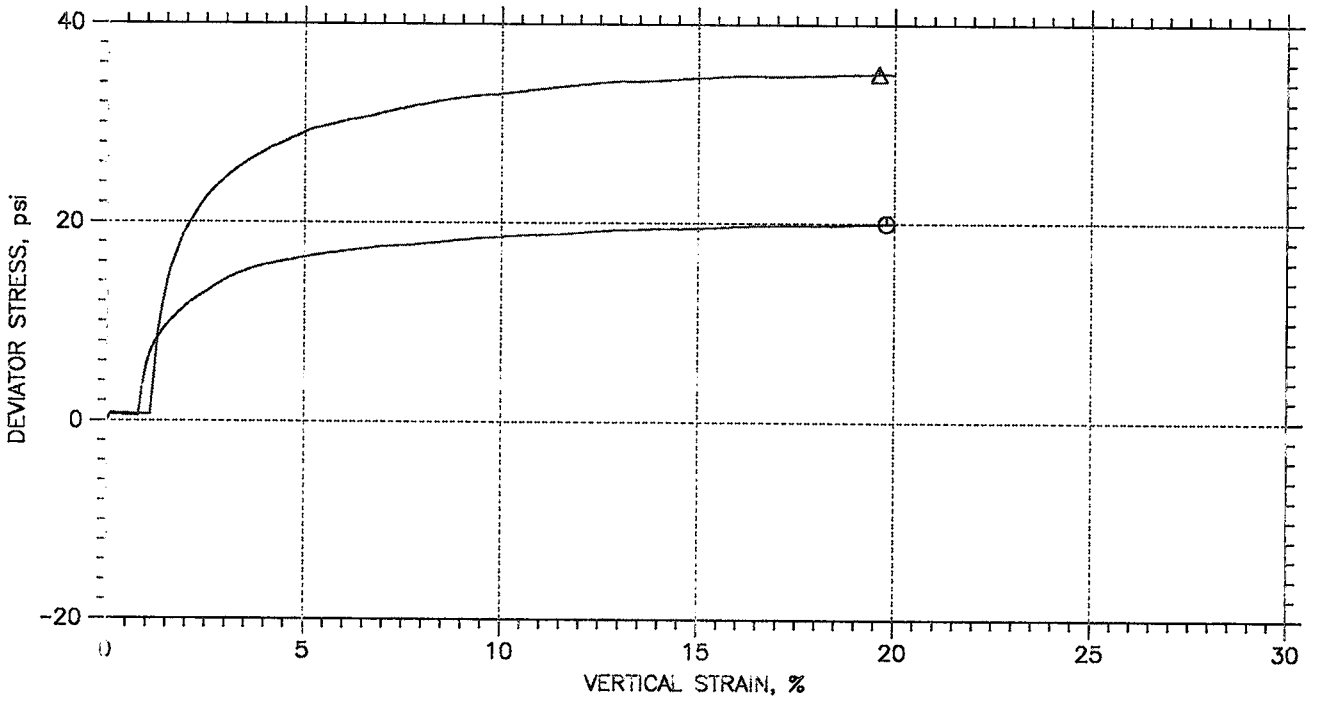
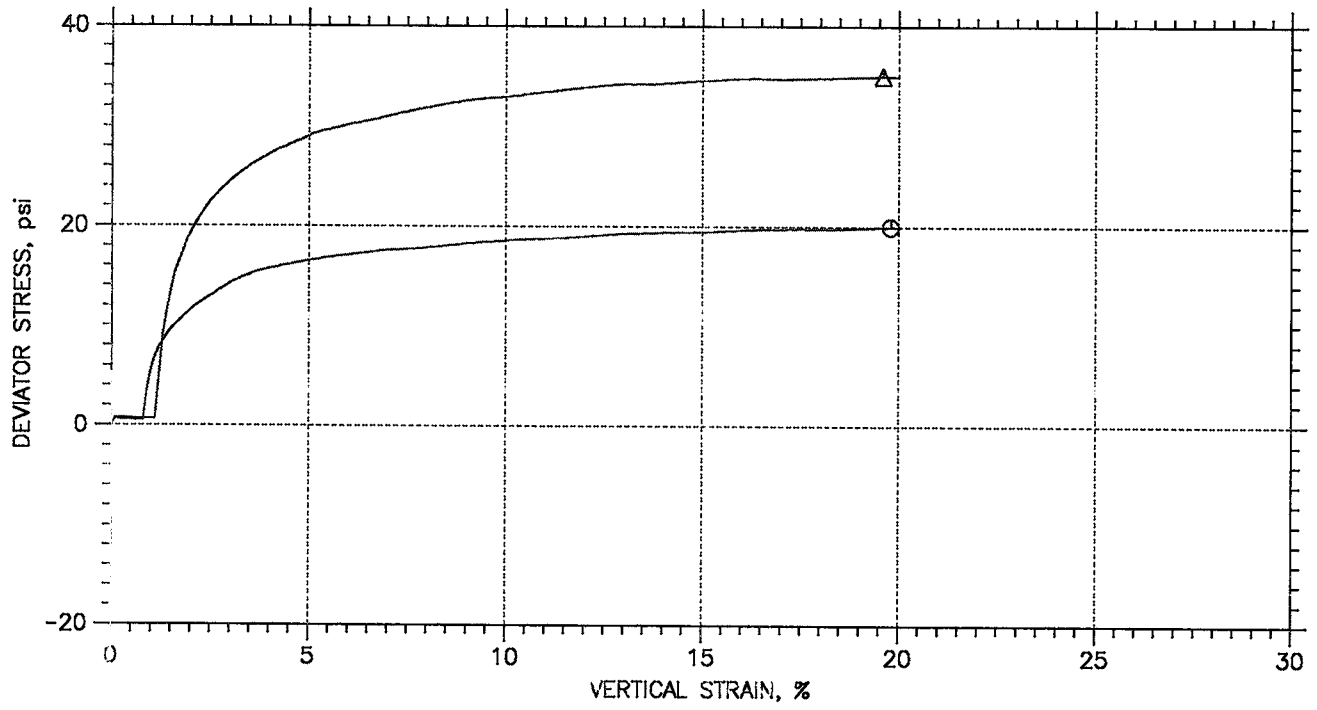


Sample No.	Test No.	Depth	Tested By	Test Date	Checked By	Check Date	Test File
○	SS-343-16	CU-6-16	Dan	5/23/16	Matt		CU-6-16.dat
△	SS-343-16	CU-7-16	Dan	5/25/16	Matt		CU-7-16.dat

	Project: IM-2-094(143)260		Location:		Project No.:	
	Boring No.: 1		Sample Type:			
	Description: T. L. 18" Brn Silty Clay with I.O. Coal Deposits, Rock upto 3/4"					
	Remarks:					

SS-343-16

### CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767



Symbol	Sample No.	Test No.	Depth	Tested By	Test Date	Checked By	Check Date	Test File
○	SS-343-16	CU-6-16	14.1-14.6	Dan	5/23/16	Matt		CU-6-16.dat
△	SS-343-16	CU-7-16	14.6-15.1	Dan	5/25/16	Matt		CU-7-16.dat

	Project: IM-2-094(143)260		Location:		Project No.:	
	Boring No.: 1		Sample Type:			
	Description: T. L. 18" Brn Silty Clay with I.O. Coal Deposits, Rock upto 3/4"					
	Remarks:					

TRIAXIAL TEST

SS-343-16

Project: IM-2-094(143)260  
 Boring No.: 1  
 Sample No.: SS-343-16  
 Test No.: CU-6-16

Location:  
 Tested By: Dan  
 Test Date: 5/23/16  
 Sample Type:

Project No.:  
 Checked By: Matt  
 Depth: 14.1-14.6  
 Elevation:

Soil Description: T. L. 18" Brn Silty Clay with I.O. Coal Deposits, Rock upto 3/4"  
 Remarks:

Specimen Height: 5.74 in  
 Specimen Area: 6.30 in<sup>2</sup>  
 Specimen Volume: 592.82 cc

Piston Area: 0.16 in<sup>2</sup>  
 Piston Friction: 0.00 lb  
 Piston Weight: 0.00 lb

Filter Strip Correction: 0.00 psi  
 Membrane Correction: 4.20 lb/in  
 Correction Type: Uniform

Liquid Limit: ---

Plastic Limit: ---

Estimated Specific Gravity: 2.65

	Before Test Trimmings	Before Test Specimen	After Test Specimen	After Test Trimmings
Container ID	s61	---		s6
Wt. Container + Wet Soil, gm	73.37	---	---	164.81
Wt. Container + Dry Soil, gm	62.89	---	---	139.07
Wt. Container, gm	15.3	---	---	29.66
Wt. Wet Soil, gm	58.07	1211.6	1226.5	135.15
Wt. Dry Soil, gm	47.59	992.94	992.94	109.41
Wt. Water, gm	10.48	218.66	233.6	25.74
Water Content, %	22.02	22.02	23.53	23.53
Void Ratio	---	0.58	0.62	---
Degree of Saturation, %	---	100.25	100.00	---
Dry Unit Weight, pcf	---	104.56	101.9	---

Initial

Height: 5.739 in  
 Area: 6.3035 in<sup>2</sup>  
 Volume: 592.82 cc

Moisture: 22.02 %  
 Void Ratio: 0.58  
 Dry Unit Weight: 104.56 pcf  
 Saturation: 100.25 %

End of Initialization

Time: 4.2388 min  
 Total Vertical Stress: 2.9907 psi  
 Total Horizontal Stress: 2.995 psi  
 Pore Pressure: 0.99564 psi  
 Effective Vertical Stress: 1.995 psi  
 Effective Horizontal Stress: 1.9994 psi

Height Change: 0.0017564 in  
 Area Change: 0 in<sup>2</sup>  
 Volume Change: 0.5443 cc  
 Water Change: 0.36683 cc  
 Correction: 0.71495 cc

Height: 5.7372 in  
 Area: 6.3035 in<sup>2</sup>  
 Volume: 592.27 cc  
 Moisture: 21.91 %  
 Void Ratio: 0.58  
 Dry Unit Weight: 104.66 pcf  
 Saturation: 100.00 %

End of Consolidation/A

Time: 4.2388 min  
 Total Vertical Stress: 2.9907 psi  
 Total Horizontal Stress: 2.995 psi  
 Pore Pressure: 0.99564 psi  
 Effective Vertical Stress: 1.995 psi  
 Effective Horizontal Stress: 1.9994 psi

Height Change: 0.0017564 in  
 Area Change: 0 in<sup>2</sup>  
 Volume Change: 0.5443 cc  
 Water Change: 0.36683 cc  
 Correction: 0.71495 cc

Height: 5.7372 in  
 Area: 6.3035 in<sup>2</sup>  
 Volume: 592.27 cc  
 Moisture: 21.91 %  
 Void Ratio: 0.58  
 Dry Unit Weight: 104.66 pcf  
 Saturation: 100.00 %

End of Saturation

Time: 701.54 min  
 Total Vertical Stress: 62.975 psi  
 Total Horizontal Stress: 62.992 psi  
 Pore Pressure: 60.993 psi  
 Effective Vertical Stress: 1.9823 psi  
 Effective Horizontal Stress: 1.9993 psi

Height Change: -0.0019876 in  
 Area Change: 0 in<sup>2</sup>  
 Volume Change: -0.61592 cc  
 Water Change: -10.204 cc  
 Correction: 10.125 cc

Height: 5.741 in  
 Area: 6.3035 in<sup>2</sup>  
 Volume: 593.43 cc  
 Moisture: 22.03 %  
 Void Ratio: 0.58  
 Dry Unit Weight: 104.46 pcf  
 Saturation: 100.00 %

End of Consolidation/B

Time: 1021.6 min  
 Total Vertical Stress: 73.493 psi  
 Total Horizontal Stress: 73.495 psi  
 Pore Pressure: 60.92 psi  
 Effective Vertical Stress: 12.573 psi  
 Effective Horizontal Stress: 12.575 psi

Height Change: -0.0023111 in  
 Area Change: -0.16198 in<sup>2</sup>  
 Volume Change: -15.479 cc  
 Water Change: -1.1085 cc  
 Correction: -13.832 cc

Height: 5.7413 in  
 Area: 6.4655 in<sup>2</sup>  
 Volume: 608.3 cc  
 Moisture: 23.53 %  
 Void Ratio: 0.62  
 Dry Unit Weight: 101.9 pcf  
 Saturation: 100.00 %

End of Shear

Time: 1292.1 min  
 Total Vertical Stress: 92.998 psi  
 Total Horizontal Stress: 73.503 psi  
 Pore Pressure: 66.06 psi  
 Effective Vertical Stress: 26.938 psi  
 Effective Horizontal Stress: 7.4429 psi

Height Change: 1.146 in  
 Area Change: -1.7784 in<sup>2</sup>  
 Volume Change: -15.479 cc  
 Water Change: -1.1091 cc  
 Correction: -13.832 cc

Height: 4.593 in  
 Area: 8.082 in<sup>2</sup>  
 Volume: 608.3 cc  
 Moisture: 23.53 %  
 Void Ratio: 0.62  
 Dry Unit Weight: 101.9 pcf  
 Saturation: 100.00 %

At Failure

Time: 1289.4 min  
 Total Vertical Stress: 93.472 psi  
 Total Horizontal Stress: 73.495 psi  
 Pore Pressure: 66.076 psi  
 Effective Vertical Stress: 27.396 psi  
 Effective Horizontal Stress: 7.4186 psi

Height Change: 1.1345 in  
 Area Change: -1.5678 in<sup>2</sup>  
 Volume Change: -15.478 cc  
 Water Change: -1.1091 cc  
 Correction: 0 cc

Height: 4.6045 in  
 Area: 7.8713 in<sup>2</sup>  
 Volume: 608.29 cc  
 Moisture: 23.53 %  
 Void Ratio: 0.62  
 Dry Unit Weight: 101.9 pcf  
 Saturation: 100.00 %

TRIAXIAL TEST

55-343-16

Project: IM-2-094(143)260  
 Boring No.: 1  
 Sample No.: SS-343-16  
 Test No.: CU-7-16

Location:  
 Tested By: Dan  
 Test Date: 5/25/16  
 Sample Type:

Project No.:  
 Checked By: Matt  
 Depth: 14.6-15.1  
 Elevation:

Soil Description: T. L. 18" Brn Silty Clay with I.O. Coal Deposits, Rock upto 3/4"  
 Remarks:

Specimen Height: 5.73 in  
 Specimen Area: 6.38 in<sup>2</sup>  
 Specimen Volume: 599.01 cc

Piston Area: 0.16 in<sup>2</sup>  
 Piston Friction: 0.00 lb  
 Piston Weight: 0.00 lb

Filter Strip Correction: 0.00 psi  
 Membrane Correction: 4.20 lb/in  
 Correction Type: Uniform

Liquid Limit: ---

Plastic Limit: ---

Estimated Specific Gravity: 2.65

	Before Test Trimmings	Before Test Specimen	After Test Specimen	After Test Trimmings
Container ID	S 39	---	---	s12
Wt. Container + Wet Soil, gm	67.92	---	---	188.9
Wt. Container + Dry Soil, gm	58.39	---	---	161.7
Wt. Container, gm	16.89	---	---	29.56
Wt. Wet Soil, gm	51.03	1242.3	1218.3	159.34
Wt. Dry Soil, gm	41.5	1010.3	1010.3	132.14
Wt. Water, gm	9.53	232.01	207.97	27.2
Water Content, %	22.96	22.96	20.58	20.58
Void Ratio	---	0.57	0.55	---
Degree of Saturation, %	---	106.55	100.00	---
Dry Unit Weight, pcf	---	105.3	107.04	---

Initial

Height: 5.73 in  
 Area: 6.3794 in<sup>2</sup>  
 Volume: 599.01 cc

Moisture: 22.96 %  
 Void Ratio: 0.57  
 Dry Unit Weight: 105.3 pcf  
 Saturation: 106.55 %

End of Initialization

Time: 15.638 min  
 Total Vertical Stress: 2.9803 psi  
 Total Horizontal Stress: 2.995 psi  
 Pore Pressure: 0.99564 psi  
 Effective Vertical Stress: 1.9847 psi  
 Effective Horizontal Stress: 1.9994 psi

Height Change: 9.2444e-005 in  
 Area Change: 0 in<sup>2</sup>  
 Volume Change: 0.028992 cc  
 Water Change: 0.81142 cc  
 Correction: 13.478 cc

Height: 5.7299 in  
 Area: 6.3794 in<sup>2</sup>  
 Volume: 598.98 cc  
 Moisture: 21.55 %  
 Void Ratio: 0.57  
 Dry Unit Weight: 105.3 pcf  
 Saturation: 100.00 %

End of Consolidation/A

Time: 15.638 min  
 Total Vertical Stress: 2.9803 psi  
 Total Horizontal Stress: 2.995 psi  
 Pore Pressure: 0.99564 psi  
 Effective Vertical Stress: 1.9847 psi  
 Effective Horizontal Stress: 1.9994 psi

Height Change: 9.2444e-005 in  
 Area Change: 0 in<sup>2</sup>  
 Volume Change: 0.028992 cc  
 Water Change: 0.81142 cc  
 Correction: 13.478 cc

Height: 5.7299 in  
 Area: 6.3794 in<sup>2</sup>  
 Volume: 598.98 cc  
 Moisture: 21.55 %  
 Void Ratio: 0.57  
 Dry Unit Weight: 105.3 pcf  
 Saturation: 100.00 %

End of Saturation

Time: 386.29 min  
 Total Vertical Stress: 62.99 psi  
 Total Horizontal Stress: 62.992 psi  
 Pore Pressure: 60.993 psi  
 Effective Vertical Stress: 1.997 psi  
 Effective Horizontal Stress: 1.9993 psi

Height Change: -0.0042987 in  
 Area Change: 0 in<sup>2</sup>  
 Volume Change: -1.3481 cc  
 Water Change: -3.9466 cc  
 Correction: 16.859 cc

Height: 5.7343 in  
 Area: 6.3794 in<sup>2</sup>  
 Volume: 600.36 cc  
 Moisture: 21.69 %  
 Void Ratio: 0.57  
 Dry Unit Weight: 105.06 pcf  
 Saturation: 100.00 %

End of Consolidation/B

Time: 534.83 min  
 Total Vertical Stress: 85.978 psi  
 Total Horizontal Stress: 86 psi  
 Pore Pressure: 61.017 psi  
 Effective Vertical Stress: 24.961 psi  
 Effective Horizontal Stress: 24.983 psi

Height Change: -0.0055929 in  
 Area Change: 0.1103 in<sup>2</sup>  
 Volume Change: 9.7821 cc  
 Water Change: 13.475 cc  
 Correction: 10.568 cc

Height: 5.7356 in  
 Area: 6.2691 in<sup>2</sup>  
 Volume: 589.23 cc  
 Moisture: 20.58 %  
 Void Ratio: 0.55  
 Dry Unit Weight: 107.04 pcf  
 Saturation: 100.00 %

End of Shear

Time: 805.73 min  
 Total Vertical Stress: 120.77 psi  
 Total Horizontal Stress: 86 psi  
 Pore Pressure: 71.492 psi  
 Effective Vertical Stress: 49.28 psi  
 Effective Horizontal Stress: 14.508 psi

Height Change: 1.1416 in  
 Area Change: -1.457 in<sup>2</sup>  
 Volume Change: 9.7816 cc  
 Water Change: 13.474 cc  
 Correction: 10.568 cc

Height: 4.5884 in  
 Area: 7.8364 in<sup>2</sup>  
 Volume: 589.23 cc  
 Moisture: 20.58 %  
 Void Ratio: 0.55  
 Dry Unit Weight: 107.04 pcf  
 Saturation: 100.00 %

At Failure

Time: 800.31 min  
 Total Vertical Stress: 121.04 psi  
 Total Horizontal Stress: 86.008 psi  
 Pore Pressure: 71.589 psi  
 Effective Vertical Stress: 49.454 psi  
 Effective Horizontal Stress: 14.419 psi

Height Change: 1.1187 in  
 Area Change: -1.3693 in<sup>2</sup>  
 Volume Change: 9.7827 cc  
 Water Change: 13.474 cc  
 Correction: 0 cc

Height: 4.6113 in  
 Area: 7.7487 in<sup>2</sup>  
 Volume: 589.23 cc  
 Moisture: 20.58 %  
 Void Ratio: 0.55  
 Dry Unit Weight: 107.04 pcf  
 Saturation: 100.00 %

**TRIAxIAL UU(Q) CCU(R) CD(S)**  
 Department of Transportation, Materials & Research Division  
 SFN 50459 (Rev. 10-2015)

Field Sample Number 55-343-16  
 Test Number 1 of 2

Project Number IM-2-094(143)260  
 Boring Number 1  
 Lab Number CU-6-16  
 Depth 14.1 to 14.6  
 Confining Pressure 12.5

Diameter	<u>2.812</u>	<u>2.850</u>
	<u>2.821</u>	<u>2.851</u>
	<u>2.811</u>	<u>2.851</u>
	Avg <u>2.833</u>	

Wt of Sample 1211.60

Height	<u>5.740</u>	L.L.
	<u>5.741</u>	L.L.
	<u>5.737</u>	
	Avg <u>5.739</u>	

Specific Gravity @ 20°C

**Trimming**

Moisture Can Number 561  
 Wet Wt + Can 73.37  
 Dry Wt + Can 62.89  
 Wt of Can 15.30

After Test = 1211.88  
56 29.66 wet 164.81  
Dry 139.07

DESCRIPTION OF SAMPLE Total Length 18"  
Ben Silty Clay with I.O. + Coal Deposits, Rock up to 3/4"  
 REMARKS

Test Number 2 of 2

Lab Number CU-7-16  
 Depth 14.6 to 15.1

Confining Pressure

Diameter	<u>2.856</u>	<u>2.842</u>
	<u>2.855</u>	<u>2.850</u>
	<u>2.846</u>	<u>2.850</u>
	Avg <u>2.850</u>	

Wt of Sample 1242.35

Height	<u>5.727</u>	L.L.
	<u>5.734</u>	L.L.
	<u>5.728</u>	
	Avg <u>5.730</u>	

Specific Gravity @ 20°C

**Trimming**

Moisture Can Number 539  
 Wet Wt + Can 67.92  
 Dry Wt + Can 58.39  
 Wt of Can 16.89

After test 1237.66  
512W 188.90  
D 161.70  
TARE WT. 29.56

DESCRIPTION OF SAMPLE  
 REMARKS

Test Number of

Lab Number  
 Depth

Confining Pressure

Diameter		
	Avg	

Wt of Sample

Height		L.L.
		L.L.
	Avg	

Specific Gravity @ 20°C

**Trimming**

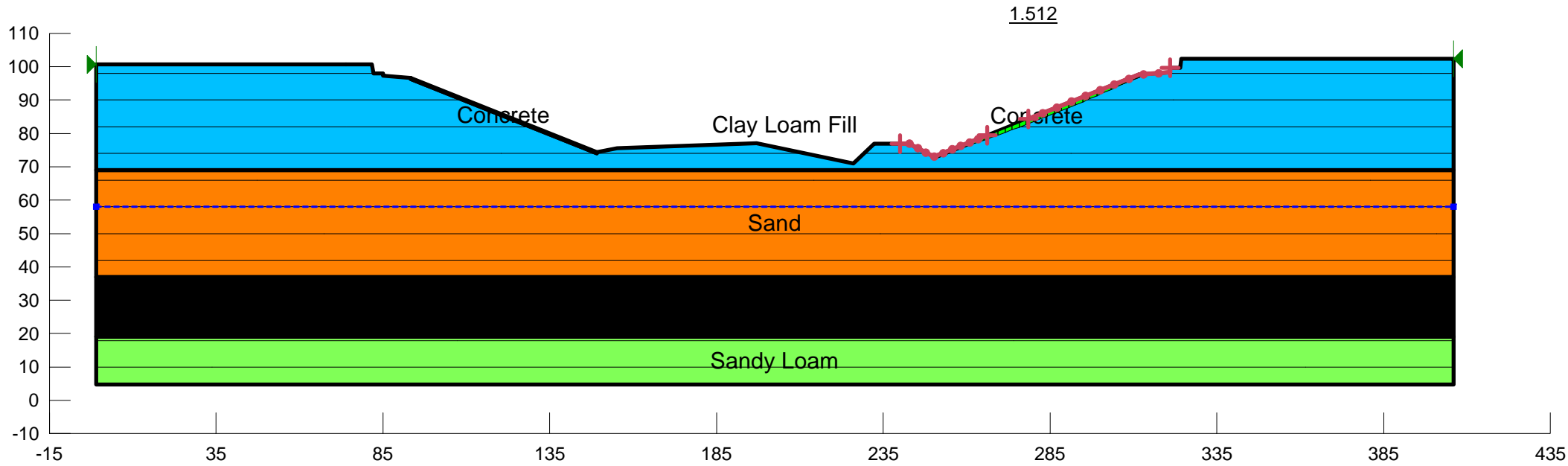
Moisture Can Number  
 Wet Wt + Can  
 Dry Wt + Can  
 Wt of Can

DESCRIPTION OF SAMPLE  
 REMARKS

## APPENDIX C

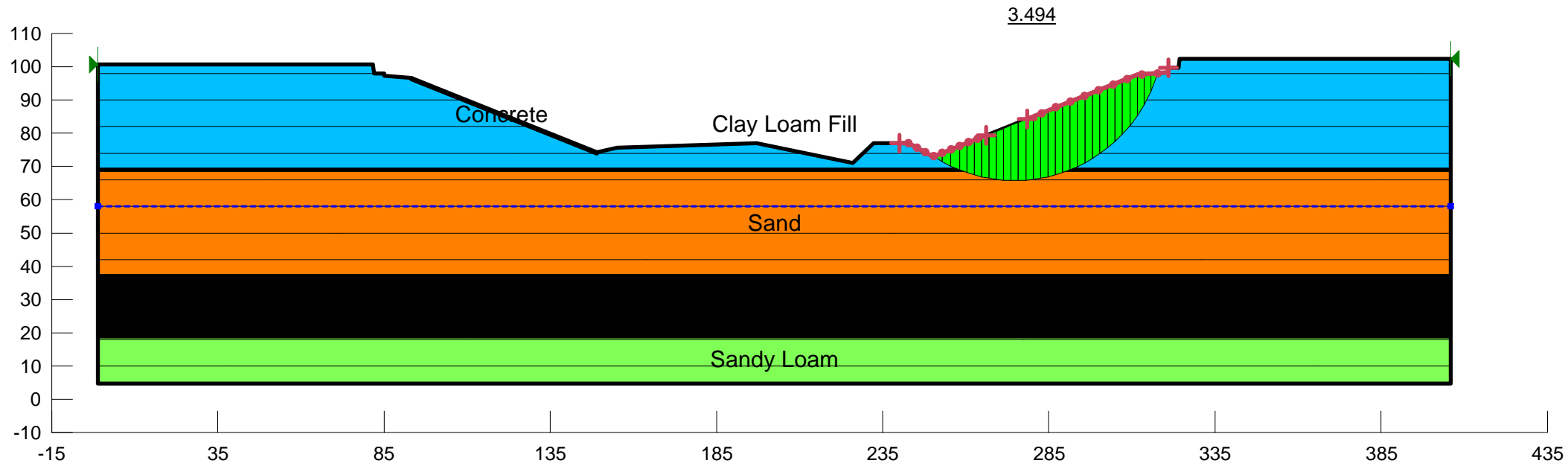
### Stability Analysis Outputs

# East ESA



Name: Clay Loam Fill    Unit Weight: 130 pcf    Cohesion': 0 psf    Phi': 30 °  
 Name: Sand    Unit Weight: 130 pcf    Cohesion': 0 psf    Phi': 33 °  
 Name: Coal    Unit Weight: 130 pcf    Cohesion': 0 psf    Phi': 34 °  
 Name: Sandy Loam    Unit Weight: 130 pcf    Cohesion': 0 psf    Phi': 34 °  
 Name: Concrete    Unit Weight: 140 pcf    Cohesion': 50 psf    Phi': 0 °

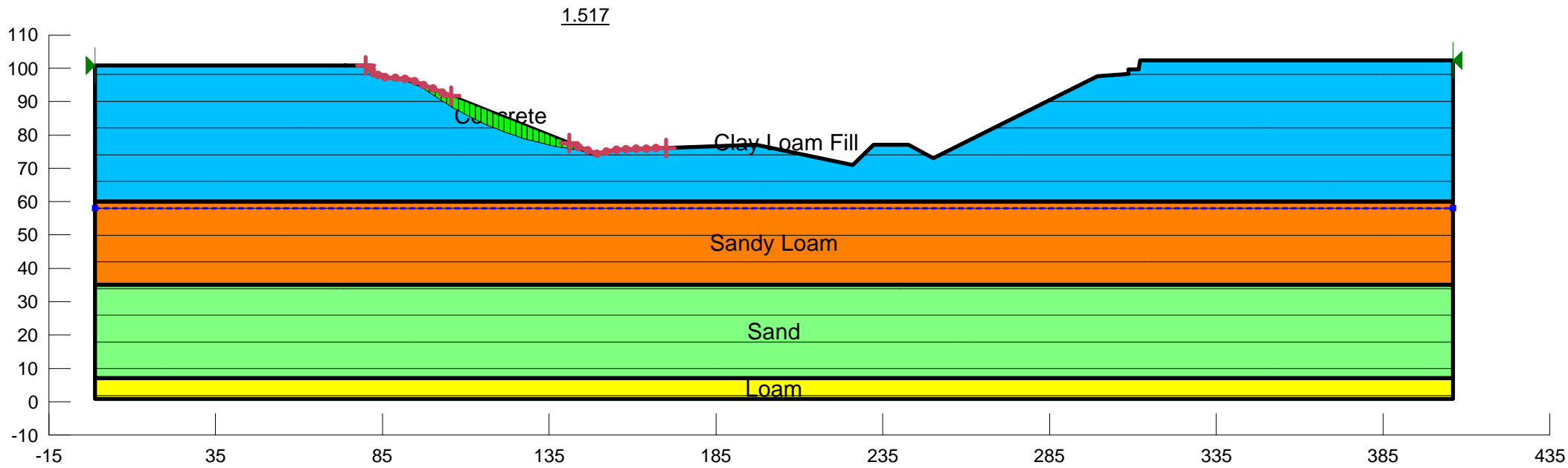
# East TSA



Name: Clay Loam Fill Unit Weight: 130 pcf Cohesion': 1,500 psf Phi': 0 °  
 Name: Sand Unit Weight: 130 pcf Cohesion': 0 psf Phi': 33 °  
 Name: Coal Unit Weight: 130 pcf Cohesion': 0 psf Phi': 34 °  
 Name: Sandy Loam Unit Weight: 130 pcf Cohesion': 0 psf Phi': 34 °  
 Name: Concrete Unit Weight: 140 pcf Cohesion': 1,000 psf Phi': 0 °

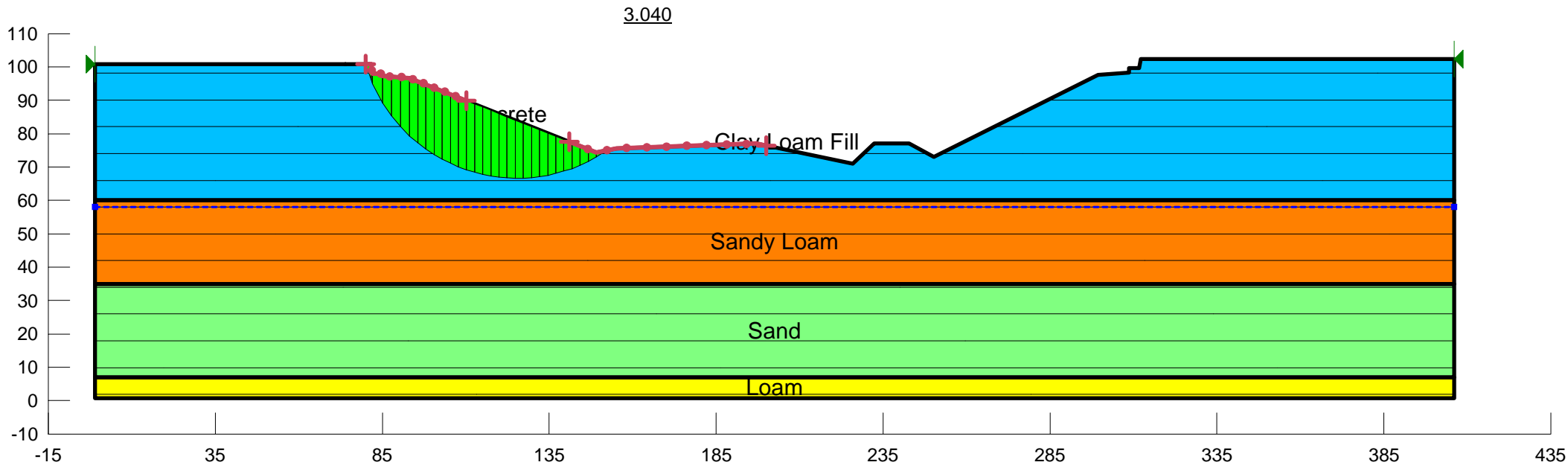


# West ESA



Name: Clay Loam Fill    Unit Weight: 130 pcf    Cohesion': 0 psf    Phi': 30 °  
Name: Sandy Loam    Unit Weight: 130 pcf    Cohesion': 0 psf    Phi': 32 °  
Name: Sand    Unit Weight: 130 pcf    Cohesion': 0 psf    Phi': 37 °  
Name: Loam    Unit Weight: 130 pcf    Cohesion': 0 psf    Phi': 20 °  
Name: Concrete    Unit Weight: 140 pcf    Cohesion': 50 psf    Phi': 0 °

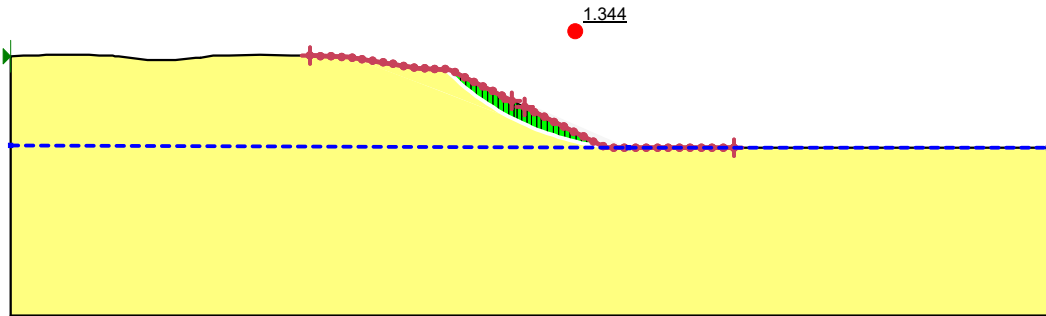
# West TSA



Name: Clay Loam Fill Unit Weight: 130 pcf Cohesion': 1,200 psf Phi': 0 °  
Name: Sandy Loam Unit Weight: 130 pcf Cohesion': 0 psf Phi': 32 °  
Name: Sand Unit Weight: 130 pcf Cohesion': 0 psf Phi': 37 °  
Name: Loam Unit Weight: 130 pcf Cohesion': 4,625 psf Phi': 0 °  
Name: Concrete Unit Weight: 140 pcf Cohesion': 1,000 psf Phi': 0 °

2:1 slope

Color	Name	Slope Stability Material Model	Unit Weight (pcf)	Effective Cohesion (psf)	Effective Friction Angle (°)	Phi-B (°)	Piezometric Line
■	New Material	Mohr-Coulomb	120	0	32.3	0	1



2.5:1 slope

Color	Name	Slope Stability Material Model	Unit Weight (pcf)	Effective Cohesion (psf)	Effective Friction Angle (°)	Phi-B (°)	Piezometric Line
■	New Material	Mohr-Coulomb	120	0	32.3	0	1

